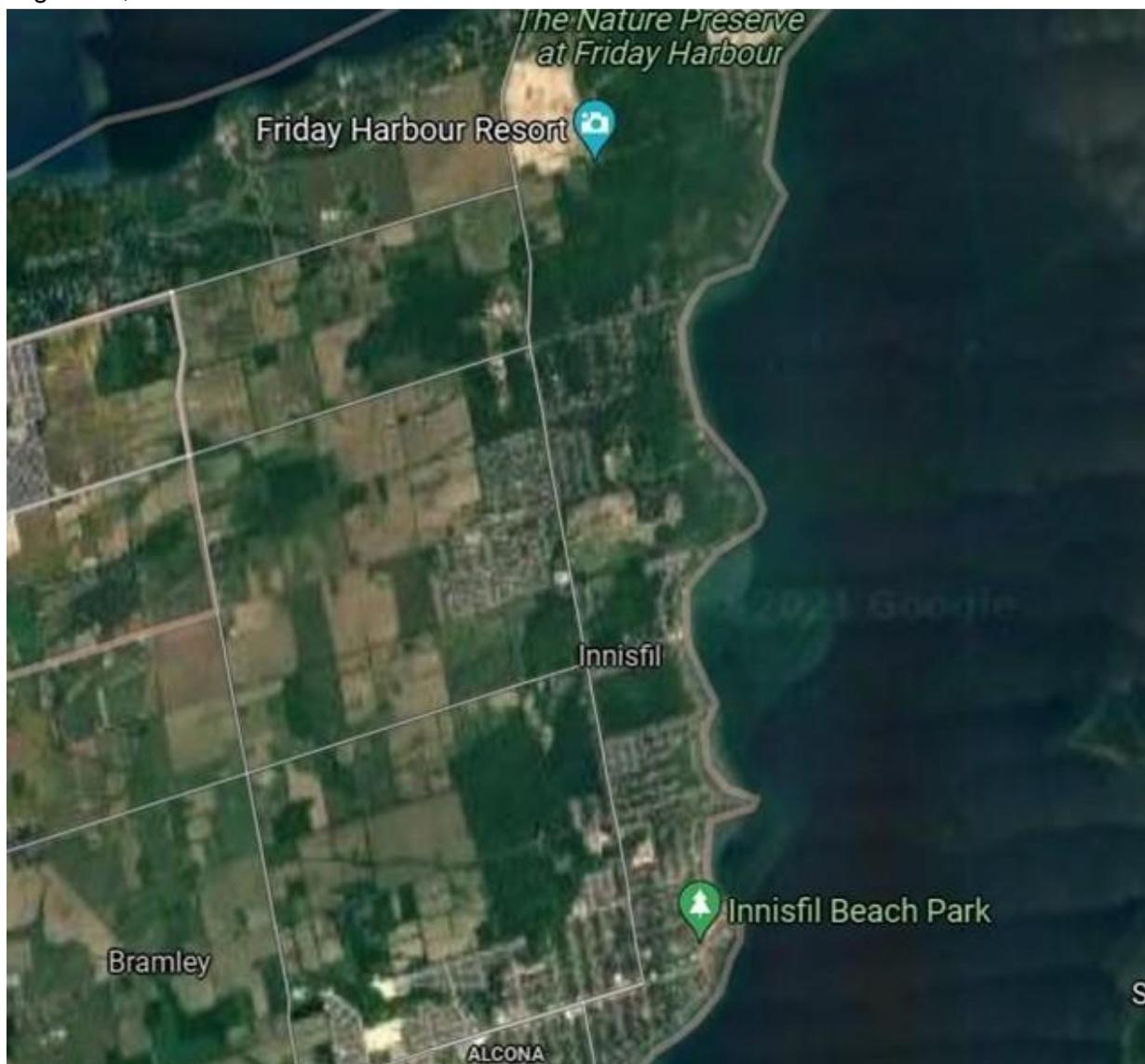


Town of Innisfil

Preliminary Design & Reconstruction Plan of 25th Sideroad, Town of Innisfil Stormwater Management Report

August 04, 2022

FINAL





Preliminary Design & Reconstruction Plan of 25th Sideroad, Town of Innisfil

Stormwater Management Report

Town of Innisfil

FINAL

Project No.: 211-06027-00

Date: August 04, 2022

WSP

100 Commerce Valley Drive West

Thornhill, ON

Canada L3T 0A1

T: +1 905 882-1100

F: +1 905 882-0055

wsp.com

Revision History

FIRST ISSUE

September 14, 2021	Draft for comment / review
Prepared by	Reviewed / Approved by
Ellie Fazeli, Designer	Tahmineh Sarabian, Project Engineer

REVISION 1

October 21, 2021	Address Client comments
Prepared by	Reviewed by
Ellie Fazeli, Designer	Tahmineh Sarabian, Project Engineer

REVISION 2

April 18, 2022	Address Client comments
Prepared by	Reviewed by
Ellie Fazeli, Designer	Tahmineh Sarabian, Project Engineer

FINAL

July 21, 2022	Preliminary Report
Prepared by	Reviewed by
Ellie Fazeli, Designer	Steven van Haren, Manager

Responding Comments

August 4, 2022	Preliminary Report
Prepared by	Reviewed by
Ellie Fazeli, Designer	Steven van Haren, Manager

Signatures

Prepared by



Ellie Fazeli, B.Sc.,
Designer, Water Resources

August 4, 2022

Date

Approved¹ by (must be reviewed for technical accuracy prior to approval)

Steven van Haren, P.Eng., P.E.
Manager, Water Resources

August 4, 2022

Date

WSP Canada Inc. (WSP) prepared this report solely for the use of the intended recipient, Town of Innisfil, in accordance with the professional services agreement. The intended recipient is solely responsible for the disclosure of any information contained in this report. The content and opinions contained in the present report are based on the observations and/or information available to WSP at the time of preparation. If a third party makes use of, relies on, or makes decisions in accordance with this report, said third party is solely responsible for such use, reliance or decisions. WSP does not accept responsibility for damages, if any, suffered by any third party as a result of decisions made or actions taken by said third party based on this report. This limitations statement is considered an integral part of this report.

The original of this digital file will be conserved by WSP for a period of not less than 10 years. As the digital file transmitted to the intended recipient is no longer under the control of WPS, its integrity cannot be assured. As such, WSP does not guarantee any modifications made to this digital file subsequent to its transmission to the intended recipient.

¹ Approval of this document is an administrative function indicating readiness for release and does not impart legal liability on to the Approver for any technical content contained herein. Technical accuracy and fit-for-purpose of this content is obtained through the review process. The Approver shall ensure the applicable review process has occurred prior to signing the document.

Contributors

Client

Town of Innisfil

WSP

Designer, Water Resources

Ellie Fazeli

Manager, Water Resources

Steven van Haren

Senior Project Manager, Active Transportation

Shawn Smith

Project Engineer, Water Resources

Tahmineh Sarabian

Proof (non-technical) / Format

Melinda Nowak



TABLE OF CONTENTS

1	INTRODUCTION.....	1
1.1	Scope	1
1.2	Site Location.....	5
1.3	Design Criteria.....	7
1.3.1	Stormwater Drainage System Policies and Design Guidelines.....	7
1.4	Rainfall Information.....	10
1.4.1	Storm Distribution	11
1.5	Geotechnical Investigations.....	12
1.5.1	Geotechnical Investigation 25 th Side Road	12
1.5.2	Geotechnical Investigation (Proposed Roundabout)	12
1.5.3	Groundwater and Hydraulic Conductivity.....	13
2	EXISTING CONDITIONS	14
2.1	General	14
2.1.1	Existing 25 th Side Road Culvert Capacity Analysis	14
2.1.2	Existing Road Drainage Areas.....	21
3	POST DEVELOPMENT CONDITIONS.....	36
3.1	Bioswales and Underground Infiltration Chambers	45
3.1.1	Bioswales.....	45
3.1.2	Underground Infiltration Chambers.....	49
3.1.3	Post-Development Conditions Hydrologic Model.....	57
3.2	Volume Control.....	57
3.3	Quality Control.....	60
3.4	Water Balance	64
3.5	Quality Control.....	65
4	CONCLUSION	69

Tables

Table 1-1:	Design Flow for Minor system and Major System	8
Table 1-2:	Design Flow Return Period for Bridges and Culverts – Standard Road Classification	9
Table 1-3:	Freeboard for Culverts	9
Table 1-4:	Flow Depths at Culverts	9
Table 1-5:	Rainfall Parameters.....	11
Table 1-6:	Total Rainfall Depths	12
Table 2-1:	Visual OTTHYMO Parameters for Existing Conditions.....	19
Table 2-2:	Full Capacity of the Existing Culverts	20
Table 2-3:	Road Drainage Area Runoffs	22
Table 3-1:	Post-Development Conditions Road Drainage Areas	36
Table 3-2:	Full Capacity of the Existing Culverts (9 th Line Road).....	57
Table 3-3:	Volume to be Retained on Site.....	58
Table 3-4:	Storage Requirements for Volume Control.....	58
Table 3-5:	Required Storage Volume to meet the 80% TTS Removal	61
Table 3-6:	Comparison of Water Balance Under Pre, Post and Post-Development with Mitigation Measures	64
Table 3-7:	Quantity Control Required Storage Volumes (12-hr SCS II)	65
Table 3-8:	Quantity Control Required Storage Volumes (4-hr Chicago Storm).....	67

Figures

Figure 1-1:	Site Location.....	6
Figure 2-1:	Existing Sub-catchment Drainage Areas.....	17
Figure 2-2:	Existing Sub-catchment Drainage Areas within the Study Area	18
Figure 2-3:	Existing Road Drainage Areas	24
Figure 2-4:	Existing Road Drainage Areas	25
Figure 2-5:	Existing Road Drainage Areas	26
Figure 2-6:	Existing Road Drainage Areas	27
Figure 2-7:	Existing Road Drainage Areas	28
Figure 2-8:	Existing Road Drainage Areas	29
Figure 2-9:	Existing Road Drainage Areas	30
Figure 2-10:	Existing Road Drainage Areas	31
Figure 2-11:	Existing Road Drainage Areas	32
Figure 2-12:	Existing Road Drainage Areas	33
Figure 2-13:	Existing Road Drainage Areas	34
Figure 2-14:	Existing Road Drainage Areas	35
Figure 3-1:	Post-Development Drainage Areas ...	39
Figure 3-2:	Post-Development Drainage Areas ...	40
Figure 3-3:	Post-Development Drainage Areas ...	41
Figure 3-4:	Post-Development Drainage Areas ...	42
Figure 3-5:	Post-Development Drainage Areas ...	43
Figure 3-6:	Post-Development Road Drainage Areas Plan.....	44

Appendices

A	Background Information
B	Stormwater Management Calculations
C	Visual OTTHYMO Results
D	CulvertMaster Results

1 INTRODUCTION

1.1 Scope

WSP has been retained by Town of Innisfil to provide a preliminary stormwater management plan for implementing Low Impact Development (LID) systems within the proposed reconstruction of the 25th Side Road (approximately 7 kilometres from Big Bay Point Road / 13th Line to Innisfil Beach Road) and a proposed roundabout at the intersection of 25th Side Road and 9th Line. The Town also seeks to implement roadway improvements to accommodate growth while prioritizing active transportation along the length of the project. The objective of this report is to:

- Provide preliminary direction for planning
- Design a range of LID systems appropriate for the subject site
- Analyze the capacity of the 25th Side Road existing crossing culverts
- Review the condition of existing culverts and recommend repair or replacement of the existing culverts

This report will illustrate how each requirement will be addressed as informed by the goals of the Town of Innisfil Engineering Design Standards and Specifications Manual, Technical Guidelines for Stormwater Management Submission LSRCA (2016), and MOE (now MECP) Stormwater Management Planning and Design Manual (Ontario Ministry of Environment, 2003).

The study area is located within the Innisfil Creeks subwatershed under the jurisdiction of the Lake Simcoe Conservation Authority (LSRCA).

The existing 25th Side Road is a two-lane roadway and can be classified as a collector roadway with rural cross-sections along the corridor. The following images present the existing conditions of the road. Please refer to **Figures 1-A, 1-B, 5 and 9** for the existing road cross-sections in **Appendix A**.



Image 1-1: Existing Conditions of 25th Sideroad © 2022 Goggle Earth



Image 1-2: Existing Conditions of 25th Side Road © 2022 Goggle Earth



Image 1-3: Existing Conditions of 25th Side Road © 2022 Goggle Earth

Under post-development conditions, WSP proposes the following contexts to illustrate the varying roadway character:

Context 1 (Low-density Residential):

Includes the community of Alcona from 80 m north of Innisfil Beach Road to Rose Lane (2.0 km) and through Sandy Cove Acres from Lockhart Road to 300 m south of Mapleview Drive (1.1 km).

Context 2 (Green Space):

Includes the segment north of Mapleview Drive (1.4 km) which is adjacent to rural and key natural heritage feature areas. Context 2 includes rural cross-sections and will have limited driveways.

Context 3 (Downtown Commercial):

Includes the first 80 m north of Innisfil Beach Road, the Innis Village development in Sandy Cove Acres (~0.7 km) and the future urban area south of Mapleview Drive inclusive of the intersection approaches (0.3 km). Context 3 includes urbanized

cross-sections. This portion of the road will be designed for on-street parking, streetscaping, placemaking, walkable development, and lower vehicle travel speeds.

The location of the proposed Contexts 1 to 3 is shown in the map below.

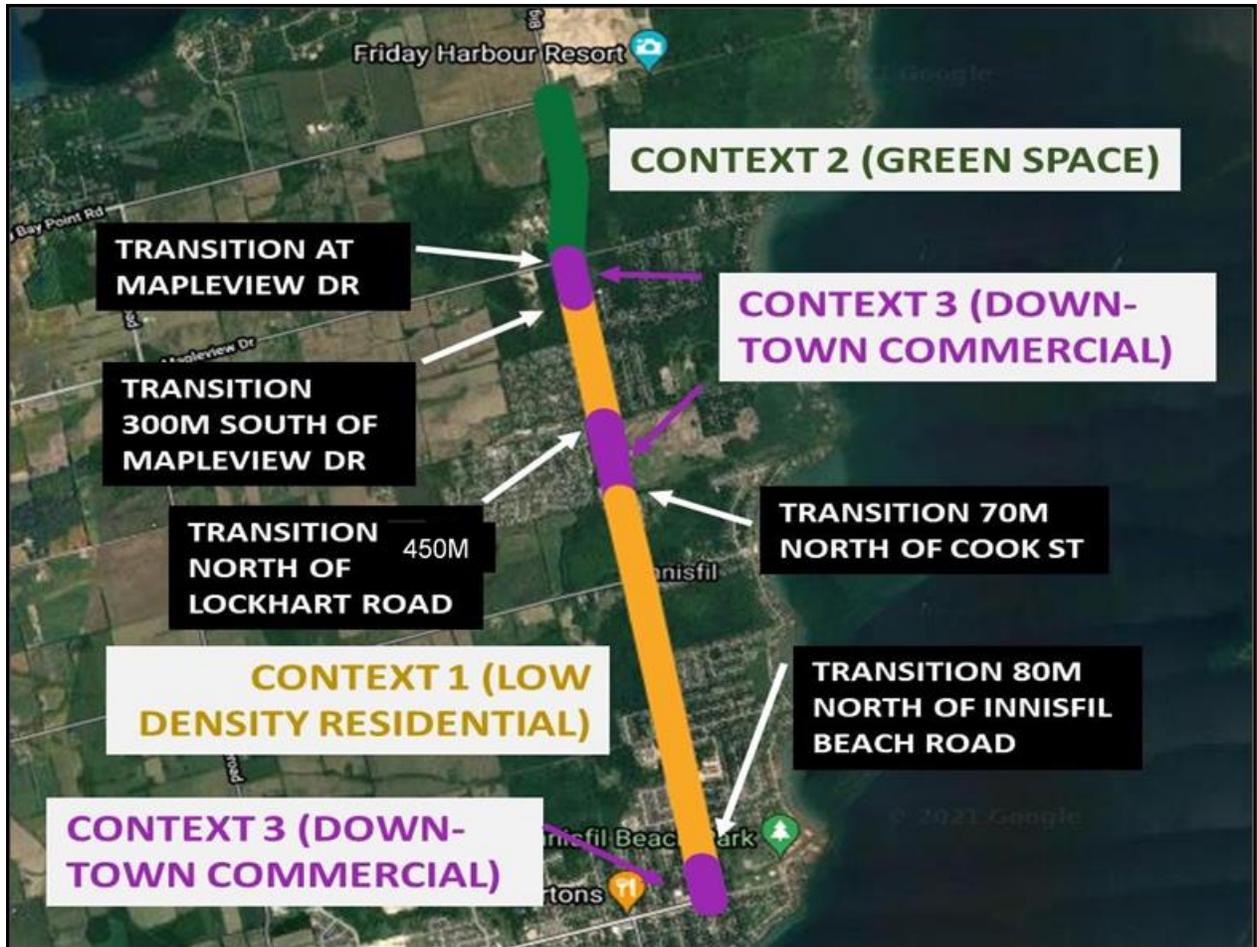


Image 1-4: Map of the Study Area including the Three Corridor Contexts

1.2 Site Location

This section of the 25th Side Road is bounded by Innisfil Beach Road to the south, Lake Simcoe to the east, Big Bay Point Road to the north and residential and agricultural lands to the west. Please refer to **Figure 1-1** for the location of the site.

FIGURE 1.1 - 25th SR Innisfil.dwg - Site Location C:\Users\bailey\OneDrive\WSP_Canada\Projects\211-06027-00 P-20-24 Innisfil 25 Sideroad\SWMCAD\FIGURES\ Sep 13, 2021 - 11:31 am



@2021 Google - Map data @2021 Tele Atlas



CLIENT	TOWN OF INNISFIL
TITLE	25th SIDE ROAD, INNISFIL
SITE LOCATION	

	
Checked S.v.H.	Drawn AutoCAD/B.K.B.
Date SEPTEMBER 2021	Proj. No. 211-06027-00
Scale AS SHOWN	Figure No. 1.1

1.3 Design Criteria

1.3.1 Stormwater Drainage System Policies and Design Guidelines

The following documents were referenced in the assessment of the drainage system:

- Town of Innisfil Engineering Design Standards and Specifications Manual (Revision # 6 May 2020)
- Technical Guidelines for Stormwater Management Submission LSRCA (2016)
- Low Impact Development Stormwater Management Planning and Design Guide, 2010, CVC and TRCA
- Ministry of Environment (MOE, now MECP) Stormwater Management Planning and Design Manual, March 2003
- MTO Drainage Management Manual, 1997
- Comprehensive Stormwater Management Master Plan (CSMMP) Town of Innisfil prepared by C.C. Tatham & Associated Ltd, dated October 2016
- Alcona North Secondary Plan Town of Innisfil prepared by C.C. Tatham & Associated Ltd, dated August 2011
- City of Toronto, Wet Weather Flow Management Guidelines, November 2006

The MTO Highway Drainage Design Standards (HDDS, 2008) prescribe standards for designing Surface Drainage Systems (SD) and Water Crossings (WC). The standards that are relevant to this study are as follows:

- **Road Crossing and Entrance Culverts:**
 - The minimum sizes for entrance and road crossing culverts are 400 mm and 500 mm, respectively.
 - The conveyance capacity of the culvert must be sufficient to ensure that the maximum depth of ponding as indicated in the table below is not exceeded.

LOCATION	STORM RETURN FREQUENCY (YEARS)		
	5	25	100/Hazel
Local Roads (2 Lane)	No ponding	0.05m above crown	0.15m above crown
Collector (4 lane+), Local and Industrial Roads (extra wide lanes)	1.0 m wide in gutter or 0.10m deep at low point catchbasins	up to crown "contradictions" - discuss	0.10 m above crown maximum depth of flooding of 0.3 m

Source: WC-7; MTO Highway Drainage and Design Standards. January 2008.

- The minimum depth of cover for all culverts is 300 mm unless otherwise indicated by the Town.

– **SD-1 Design Flows for Surface Drainage Systems:**

This standard identifies the minimum design flows that shall be used for the sizing of road surface drainage systems. The selected design flow shall be applied to size the minor and major drainage systems (piped and surface flow) for various MTO road types as shown in **Table 1-1**.

Table 1-1: Design Flow for Minor system and Major System

Functional Road Classification	Drainage System Type	Design Flow
Freeway	Minor System	10-Year
Arterial (Urban)	Major System	100-Year
Arterial (Rural)	Minor System	10-Year
Collector (Urban and Rural)	Major System	100-Year
Local Road (Urban and Rural)	Minor System	5-Year
	Major System	-

Source: SD-1: MTO Highway Drainage Design Standards. January 2008.

Road grading must direct flows from the right-of-way to a safe outlet at specified low points. Roads may be used for major system overland flow conveyance during the Regulatory Event (i.e., the larger of the 100-yr storm or Hurricane Hazel) storm subject to a water depth limitation of 0.10 m above crown of road for collector roads.

– **SD-9 Roadside Ditches (Conveyance Only):**

This standard identifies the minimum design requirement of roadside ditches for the conveyance of flow. It does not address the design of roadside ditches for improvement of water quality. Roadside ditches shall be designed to convey both the Minor System Design Flow and the Major System Design Flow, as defined in Standard SD-1, Design Flows for Surface Drainage.

– **WC-1 Design Flows (Bridges and Culverts):**

This standard identifies the minimum design flows for the sizing of bridges and culverts for flow conveyance on Regulated and Non-regulated watercourses as shown in **Table 1-2**. It also identifies the requirement for accommodating the regulatory flow on regulated watercourses, and for determining the maximum allowable increase in flood elevations upstream of a bridge or culvert.

Table 1-2: Design Flow Return Period for Bridges and Culverts – Standard Road Classification

Functional Road Classification	Return Period of Design Flows (Years) 1, 2, 3		Check Flow for Scour
	Total Span less than or equal to 6.0 m	Total Span greater than 6.0 m	
Freeway, Urban Arterial	50	100	130% of 100 year
Rural Arterial, Collector Road	25	50	115% of 100 year
Local Road	10	25	100% of 100 year

Source: WC-1; MTO Highway Drainage Design Standards. January 2008.

– **WC-7 Culvert Crossings on a Watercourse:**

This standard identifies the minimum Freeboard (**Table 1-3**), the minimum Clearance, and the maximum Flood Depth at culvert crossings (**Table 1-4**).

Table 1-3: Freeboard for Culverts

Road Type	Freeboard
Freeways, Arterials, Collectors	≥ 1.0 m
Highway Ramps	≥ 1.0 m
Local Roads including Private Entrances	≥ 0.3 m

Table 1-4 depicts the design standard for Flood Depth at the upstream face of a culvert. It is expressed as a ratio of the Flood Depth at the upstream face of the culvert to the diameter or rise of the culvert (HW/D).

Table 1-4: Flow Depths at Culverts

Functional Road Classification	Design Flow	HW/D Ratio
Freeways, Arterials, Collectors	See Standard WC-1	Culverts with diameter or rise < 3.0 m HW/D ≤ 1.5
Highway Ramps, Other Roads, and Private Entrances	See Standard SD-13	Culverts with diameter or rise 3.0 to 4.5 m HW ≤ 4.5 m Culverts with diameter or rise > 4.5 m HW/D ≤ 1.0

1.3.1.1 Stormwater Management Policies and Design Guidelines

– Quantity Control:

The post-development peak flow rates are not to exceed the corresponding predevelopment peak flow rates for the 1 in 2 year, 1 in 5 year, 1 in 10 year, 1 in 25 year, 1 in 50 year and the 1 in 100 year design storm events.

– Volume Control:

Linear redevelopment projects on sites without restrictions that create 0.5 or greater hectares of new and/or fully reconstructed impervious surfaces, shall capture and retain / treat on site, the larger of the following:

- 1 The runoff from a 12.5 mm event from the fully reconstructed impervious surface and newly constructed impervious area.
- 2 The runoff from a 25 mm event from the net increase in impervious area on the site.

– Water Quality:

– Suspended Solids

The required suspended solids removal treatment is MOECC (now MECP) Enhanced Protection Level (Level 1). This corresponds to a long-term average removal of 80% of suspended solids.

– Phosphorus

LSRCA initiated the Lake Simcoe Phosphorus Offsetting Policy (LSPOP) in 2017, which requires that all new development must control 100% of the phosphorus from leaving their property. This is referred to as the Zero Export Target. Under this policy, any remaining phosphorus load that cannot be controlled would trigger the need for an offset to achieve a net zero target.

– Water Balance:

Maintain pre-development infiltration rates under proposed development conditions.

1.4 Rainfall Information

Rainfall Intensity-Duration-Frequency (IDF) parameters for various return period design storms were obtained based on the IDF tables developed by Environment Canada for Barrie Water Pollution Control Centre (WPCC) including a 15% increase in rainfall intensity data to account for climate change. The parameters, 4-hour, 6-hour and 12-hour rainfall depths are summarized in **Table 1-5**.

Table 1-5: Rainfall Parameters

Return Period (Years)	2	5	10	25	50	100
A	678.085	853.608	975.865	1146.275	1236.152	1426.408
B	4.699	4.699	4.699	4.922	4.699	5.273
C	0.781	0.766	0.760	0.757	0.751	0.759
Equation for Rainfall Intensity	$I = A/(t + B)^C$					
Rainfall Depth, mm (4 hour)	37.0	50.5	59.7	71.3	79.5	87.6
Rainfall Depth, mm (6 hour)	42.3	59.5	70.8	85.2	95.9	106.5
Rainfall Depth, mm (12 hour)	46.7	64.3	76.0	90.7	101.7	112.5

1.4.1 Storm Distribution

A hydrologic study was completed by C.C. Tatham Associates Ltd. within the Comprehensive Stormwater Management Master Plan (CSMMP) Town of Innisfil Report dated October 2016 for subwatersheds within the study area (i.e., Bon Secours, Leonard, Mooselanka, and Sandy Cove Creeks). The hydrologic study was completed using Visual OTTHYMO (VO2) hydrologic model software and applied the 4-hour Chicago and 12-hour SCS Type II design storms. For the purpose of this study, the CSMMP model data and catchment delineations have been used where possible. For the SCS storms, a 12-hour SCS Type II mass storm file has been used, using total precipitation depths for the 12-hour storm event from the Barrie WPCC data. The 4-hour Chicago storm event has also been modelled using the Barrie WPCC rainfall data to determine the peak flows under existing and future conditions. In order to model a climate change scenario using only the 12-hour SCS storms, we have selected the Barrie WPCC data as our baseline and applied a 15% increase in rainfall intensity. The total rainfall depths for the 12-hour SCS Type II storms are summarized in **Table 1-6**.

Table 1-6: Total Rainfall Depths

Storm Event	Depth (mm)	
	12-hr SCS (no climate change)	12-hr SCS (climate change)
2-year	40.8	46.8
5-year	56.4	64.8
10-year	66.0	75.6
25-year	79.2	91.2
50-year	88.8	102.0
100-year	98.4	112.8
Regional (Hazel)	212	

1.5 Geotechnical Investigations

1.5.1 Geotechnical Investigation 25th Side Road

A preliminary Geotechnical investigation report was provided by WSP Canada Inc. (WSP), dated April 2022 for the subject site. Fourteen boreholes ranging between 1.5 m to 2.1 m depth below ground surface (bgs) and seven coreholes were installed through the existing road surface to determine the type and thickness of the pavement structure. Six hand-dug holes were implemented to confirm topsoil thickness for the proposed new active transportation facilities (cycle tracks, multi-use paths, and sidewalks) within the study limits. Based on the Geotechnical Investigation Report a flexible pavement structure (asphalt over granular fill material) was encountered. The existing pavement structure consists of:

- Asphalt Thickness: 125 mm
- Granular Base Thickness: 200 mm
- Granular Subbase Thickness: 290 mm
- Total Pavement Structure Thickness: 615 mm

Subgrade soil material consists mostly of sand, except in areas of BH1 and BH5 where silty clay was observed.

1.5.2 Geotechnical Investigation (Proposed Roundabout)

Peto MacCallum Ltd. (PML) conducted a geotechnical investigation in 2018 on 9th Line between Ralph Street and Lake Simcoe. A total of 12 boreholes were drilled to a depth of 2 m.

For the preliminary design of the roundabout, the recommended pavement design will be based on the findings of BH17 from the PML report (18BF060), which was drilled

approximately 30 meters east of the intersection. A flexible pavement structure (asphalt over granular fill material) was encountered within BH17. The existing pavement structure consists of:

- Asphalt Thickness: 70 mm
- Granular Base Thickness: 150 mm
- Granular Subbase Thickness: 280 mm
- Total Pavement Structure Thickness: 500 mm

A deposit of sand with a trace of gravel and silt was encountered below the granular subbase in BH17 at a depth of 0.5 m and was present up to borehole termination depth (2 m).

1.5.3 Groundwater and Hydraulic Conductivity

Groundwater was not encountered during the WSP investigation, and all of the boreholes remained open and dry upon completion. It should be noted that groundwater levels can vary and are prone to seasonal fluctuations in response to major weather events. The lowest permeability (i.e., hydraulic conductivity) of the subgrade has been estimated in Boreholes BH1 and BH5 (i.e., between 1×10^{-8} to 1×10^{-10}). The medium permeability (i.e., 1×10^{-4}) has been estimated in Borehole BH3.

The Geotechnical Investigation Report was provided in a separate package.

2 EXISTING CONDITIONS

2.1 General

The existing road is a two-lane, two-direction roadway classified as a collector roadway. Within the study limits, the existing road is a flexible pavement with a rural cross-section (i.e., surface water drains towards ditches on either side of the road crown except for the first 80 m north of Innisfil Beach Road, which drains to the existing catchbasins along the road). Please refer to **Appendix A** for existing cross-sections of the road.

The 25th Side Road study area is located within the Innisfil Creek subwatershed (see **Appendix A / Figure 2-1**). Some portions of the four named streams (i.e., Bon Secours Creek, Leonard's Creek, Moosenlanka Creek and Sandy Cove Creek) were identified within the study boundary, all of the subwatershed's streams have headwaters in agricultural areas and then flow downstream toward the existing 25th Side Road crossing culverts prior to being discharged into Lake Simcoe. Refer to **Figure 2-1** for the location of Bon Secours, Moosenlanka, Sandy Cove, and Leonard Creeks. **Figure 2-2** illustrates the subwatershed boundaries within the study area.

Although the majority of the study area includes agricultural lands, some settlement areas (i.e., Alcona, Sandy Cove and Big Bay Point) are identified within the study area as well.

The Alcona settlement area consists of a total of 1569.1 ha of land and a total of 21 existing SWM facilities. The Sandy Cove settlement area is 501.5 ha of land in size and hosts two existing SWM facilities. The Big Bay Point settlement area consists of a total of 239.1 ha of land with no existing SWM facility identified. Please refer to **Appendix A, Figures 9A and 9F** for the location of the existing SWM facilities within the Alcona and Sandy Cove settlement areas. Some portions of Alcona, Sandy Cove and Big Bay Point settlement areas are located on the west side of the subject site (**Figure 2-2**).

2.1.1 Existing 25th Side Road Culvert Capacity Analysis

To estimate the capacity of the existing 25th Side Road crossing culverts, the study area was delineated into 12 catchments (i.e., Catchments 101, 101-1, 102 to 111). A Visual OTTHYMO (VO6.1) model was simulated to estimate the existing peak flow rates for Catchments 101,101-1 to 111. Peak flow rates for the 4-hour Chicago and 12-hour SCS Type II design storms were determined for the 25- and 100-year return periods (**Table 1-2 / Section 1.3**).

The model has been developed using the existing land use data provided within the Comprehensive Stormwater Management Master Plan (CSMMP) Town of Innisfil Report.

As mentioned previously, there are some SWM facilities within the study area which control runoff generated from the settlement areas prior to being discharged into the 25th Side Road existing crossing culverts. The impact of the existing SWM facilities has been considered for the capacity analysis of the existing 25th Side Road crossing culverts.

The hydrologic model within the CSMMP report includes catchment delineations for Bon Secours, Leonard, Moosenlanka and Sandy Cove Creeks. Although the details of the CSMMP's Visual OTTHYMMO model outputs and SWM facilities release rates are not available, the CSMMP's Visual OTTHYMMO model data and catchment delineation within the settlement areas were used to estimate flow rates at the location of the 25th Side Road existing culverts. Refer to **Figures 9-A** and **9-F** in **Appendix A** for further details.

Under existing conditions, to estimate the capacity of the existing 25th Side Road crossing culvert (i.e., Bon Secours Creek crossing culvert), Catchment 101 was divided into two sub-catchments. The immediate catchment on the west side of 25th Side Road (i.e., Catchment 101-1) was modelled in this report. Since there are some existing SWM facilities within the remaining areas of Catchment 101, the CSMMP's Visual OTTHYMO modelling results for Catchments AC100 to AC107 were used for the remaining areas of Catchment 101.

Figure 9-A in **Appendix A** shows the location of Catchments AC100 to AC107.

Figure 1 in **Appendix A** shows a sketch of the CSMMP existing Visual OTTHYMO model for Catchments AC100 to AC 107. No SWM facility exists within Catchment 102, therefore, WSP modelled Catchment 102.

Runoff generated from portions of Catchment 103 is controlled by the existing SWM facilities (i.e., 8-3, 8-4, 8-5 and 8-9). Please refer to **Figure 9-A** in **Appendix A** for the location of these SWM facilities. The function of these existing SWM facilities has been modelled within the Alcona North Secondary Master Drainage Plan study prepared by C.C. Tatham & Associated (CCTA) Ltd. dated, August 2011 (see **Figure 9-A** in **Appendix A** for the location of Alcona North Secondary Plan). Although the details of the modelling results are not available, a summary of the peak flow rates expected at the 25th Side Road has been provided in Table 2 of the CCTA report. Based on the results, the performance of the existing SWM facilities in reducing the flow rate within Catchment 103 closely matches existing conditions (i.e., without SWM facilities). Thus,

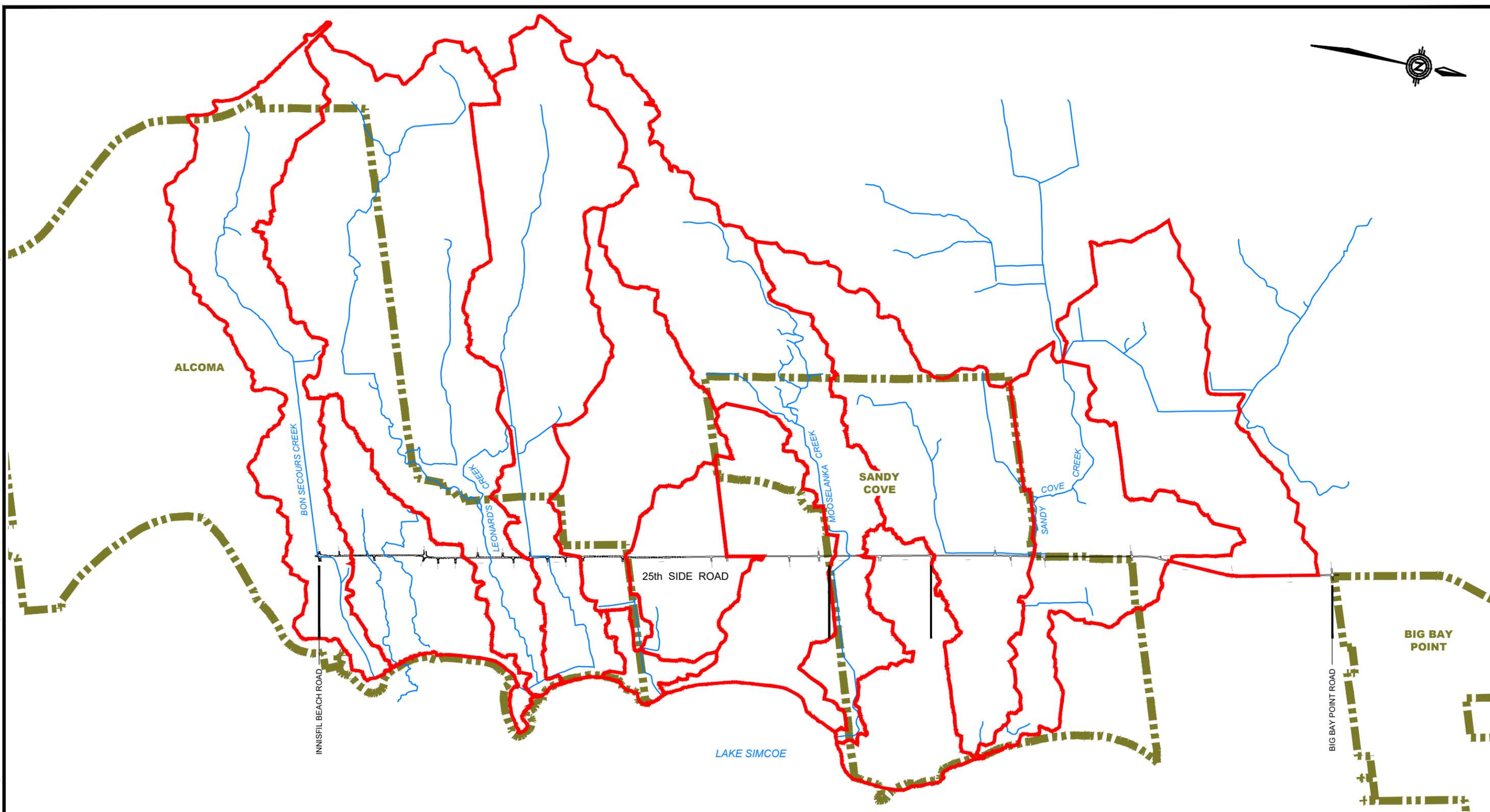
the existing flood prone areas within Catchment 103 downstream of the 25th Side Road existing culverts are not reduced by controlling flow rates within the SWM facilities. The Alcona North Secondary report indicates some stormwater management plans to reduce runoff generated from this portion of the study area, which has not been constructed. Therefore, within the current Visual OTTHYMO model WSP disregarded the existing SWM facilities within Catchment 103.

There is no existing SWM facility within Catchments 104 to 111. As such, Catchments 104 to 111 have been modelled.

Table 2-1 summarizes drainage areas, the assumed CN numbers corresponding to each land use, the TIMP and XIMP values for STANDHYD commands and the time to peak (Tp) values for NASHYD commands within the Visual OTTHYMO model. The time to peak values for the catchment areas was calculated using the Airport Method for runoff coefficients less than 0.4. Please refer to **Appendix B**.

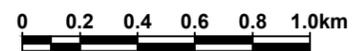
Under existing conditions, the existing culvert capacities were evaluated using CulvertMaster. The physically measured properties of the culverts, including diameter, length, and slope, were applied to a CulvertMaster model from the existing survey plan. Based on the CulvertMaster modelling results, the capacities of the existing culverts are significantly lower than the 25- and 100-year storm events (**Table 2-2**). CulvertMaster was used to estimate the minimum required size of the culverts to pass the 25-year design storms. The results are provided in **Table 2-2** and **Appendix B**. The details of the Visual OTTHYMO and CulvertMaster modelling results are provided in **Appendix C** and **D** respectively.

UPDATED-FIGURE 2.1 - 25th SR, Innisfil.dwg - Ex-Sub-Catch Drainage Areas C:\Users\slouis\m\ACCDocs\WSP Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Sideroad\SWMCAD\FIGURES - Aug 03, 2022 - 2:52pm



LEGEND

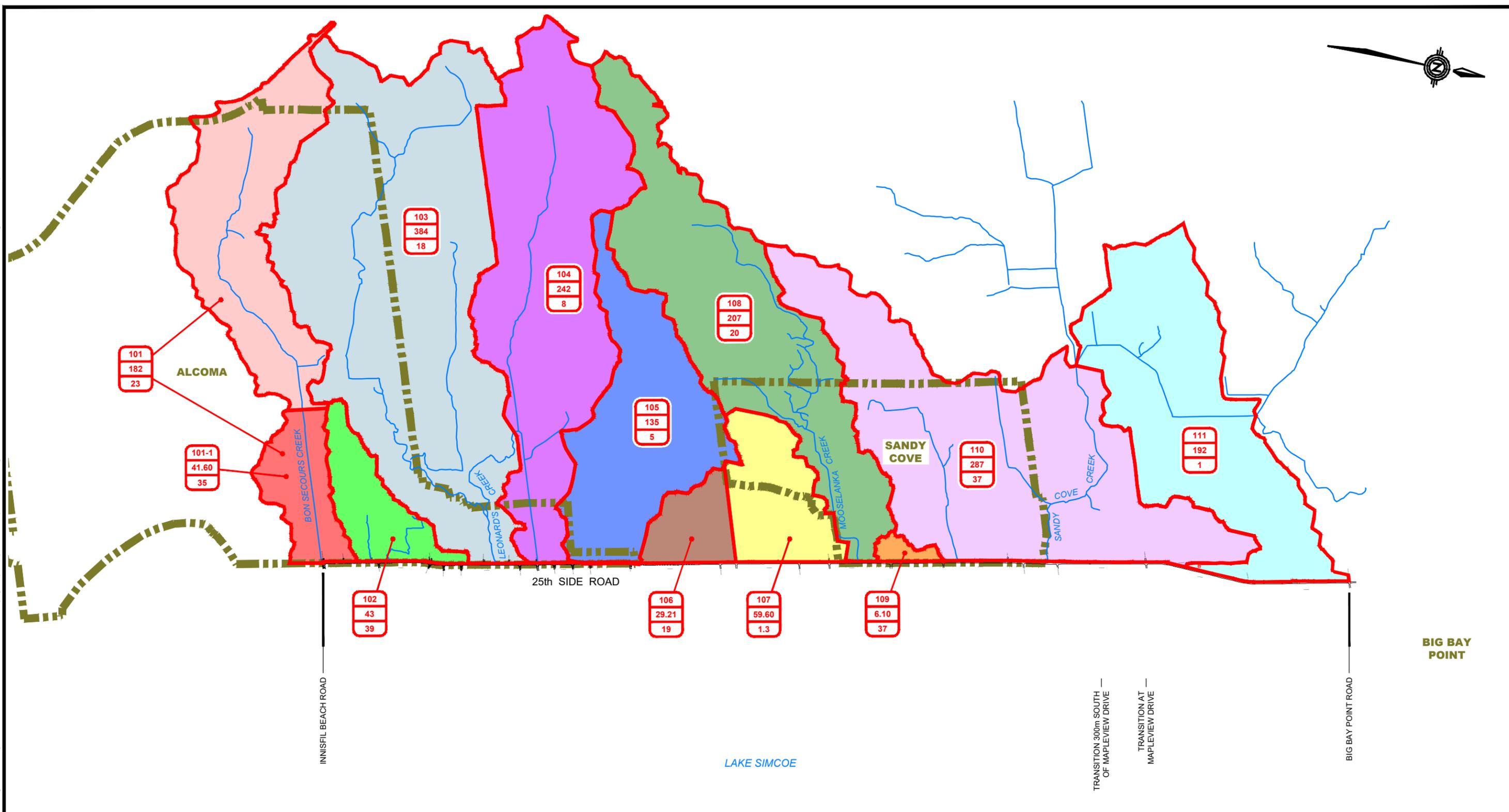
-  CATCHMENT BOUNDARY
-  SETTLEMENT BOUNDARY
-  WATERCOURSE



CLIENT	TOWN OF INNISFIL		
TITLE	25th SIDE ROAD, INNISFIL		
EXISTING SUB-CATCHMENT DRAINAGE AREAS			
Checked	S.v.H.	Drawn	AutoCAD/B.K.B.
Date	AUGUST 2022	Proj. No.	211-06027-00
Scale	AS SHOWN	Figure No.	2.1

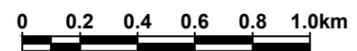


UPDATED-FIGURE 2.2 - 25th SR, Innisfil.dwg - Ex-Sub-Catch Drainage Within Study Area C:\Users\slouis\m\ACCDocs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00-P-20-24_Innisfil 25 Sideroad\SWM\CAD\FIGURES\ Aug 03, 2022 - 2:53pm



LEGEND

- CATCHMENT BOUNDARY
- SETTLEMENT BOUNDARY
- WATERCOURSE
- 101 CATCHMENT ID.
- 182 DRAINAGE AREA (ha)
- 23 RUNOFF COEFFICIENT



CLIENT	TOWN OF INNISFIL	
TITLE	25th SIDE ROAD, INNISFIL	
EXISTING SUB-CATCHMENT DRAINAGE AREAS WITHIN STUDY AREA		

Checked S.v.H.	Drawn AutoCAD/B.K.B.
Date AUGUST 2022	Proj. No. 211-06027-00
Scale AS SHOWN	Figure No. 2.2

Table 2-1: Visual OTTHYMO Parameters for Existing Conditions

Catchment Name	Catchment Area (ha)	TIMP (%)	XIMP (%)	CN	TP (hr)	Existing Culvert Size
101-1	42	35	30	63.57	N/A	1800 mm
101 (includes 101-1)	182	21	0.18	55	N/A	
102	43	39	31	66.18	N/A	500 mm and 600 mm
103	384	18	15	65	1.87	1100 mm x 1650 mm
104	242	8	7	59	1.66	800 mm
105	135	5	3	53.4	0.35	1200 mm
						800 mm
106	29.21	20	18	53.4	0.61	500 mm x 700 mm
107	59.6	1.3	1.2	53.4	0.93	800 mm
108	207	20	18	51.2	1.86	1000 mm x 700 mm and 900 mm
109	6	37	30	72	N/A	450 mm
110	287	37	30	72	N/A	6 m x 3.2 m Bridge and 900 mm
111	192	1	0.8	63.2	2.72	400 mm

Table 2-2: Full Capacity of the Existing Culverts

Catchment Name	Watercourse Name	Ex. Culvert		25- year 12 hr SCS II (m ³ /s)	100- year 12 hr SCS II (m ³ /s)	25- year 4- hr Chicago (m ³ /s)	100- year 4- hr Chicago (m ³ /s)	Culvert Capacity (m ³ /s)	Required Culvert size to pass 25- year flow (m)
		Span / d (m)	Height / d (m)						
101-1	Bon Secours	1.8	1.8	3.9	5	4.26	5.34	4.38	3.6 x 1.5
101 (includes 101-1)				9.41	12.25	9.50	12.01		
102	N/A	0.5	0.5	4.24	5.4	4.70	5.86	0.45	3 x 1.2
		0.6	0.6						
103	Leonard's Creek	1.1	1.65	8.87	12.825	6.69	9.68	3.69	3.6 x 1.2
104		0.8	0.8	2.47	3.63	1.83	2.68	0.66	2.4 x 1.5
105	N/A	1.2	1.2	6.13	9.13	3.90	5.86	1.99	2.1 x 1.2
		0.8	0.8						1.2 x 0.9
106	N/A	0.5	0.7	0.90	1.34	0.60	0.90	0.44	0.5 x 0.7
			0.5 x 0.7						
107	N/A	0.8	0.8	1.35	2.00	0.93	1.39	0.58	1.5 x 0.9
108	Moosenlanka	0.7	1	2.52	3.79	1.85	2.77	1.41	2.1 x 0.9
		0.9	0.9						0.9 x 0.9
109	N/A	0.45	0.45	0.71	0.97	0.75	0.94	0.58	0.6 x 0.6
110	Sandy Cove	6	3.2	22.46	29.43	22.38	28.37	42	Not Required
		0.9	0.9						
111	N/A	0.4	0.4	2.48	3.63	1.88	2.75	0.10	2.1 x 0.9

2.1.2 Existing Road Drainage Areas

Under existing conditions, the roadway runoff is conveyed predominantly by roadside ditches prior to being discharged into the existing 25th Side Road culverts. Minor flows from a small portion of the roadway areas between Raynor Court and Innisfil Beach Road discharge into the existing storm pipes. Road drainage areas are identified in **Figure 2-3** to **Figure 2-14**.

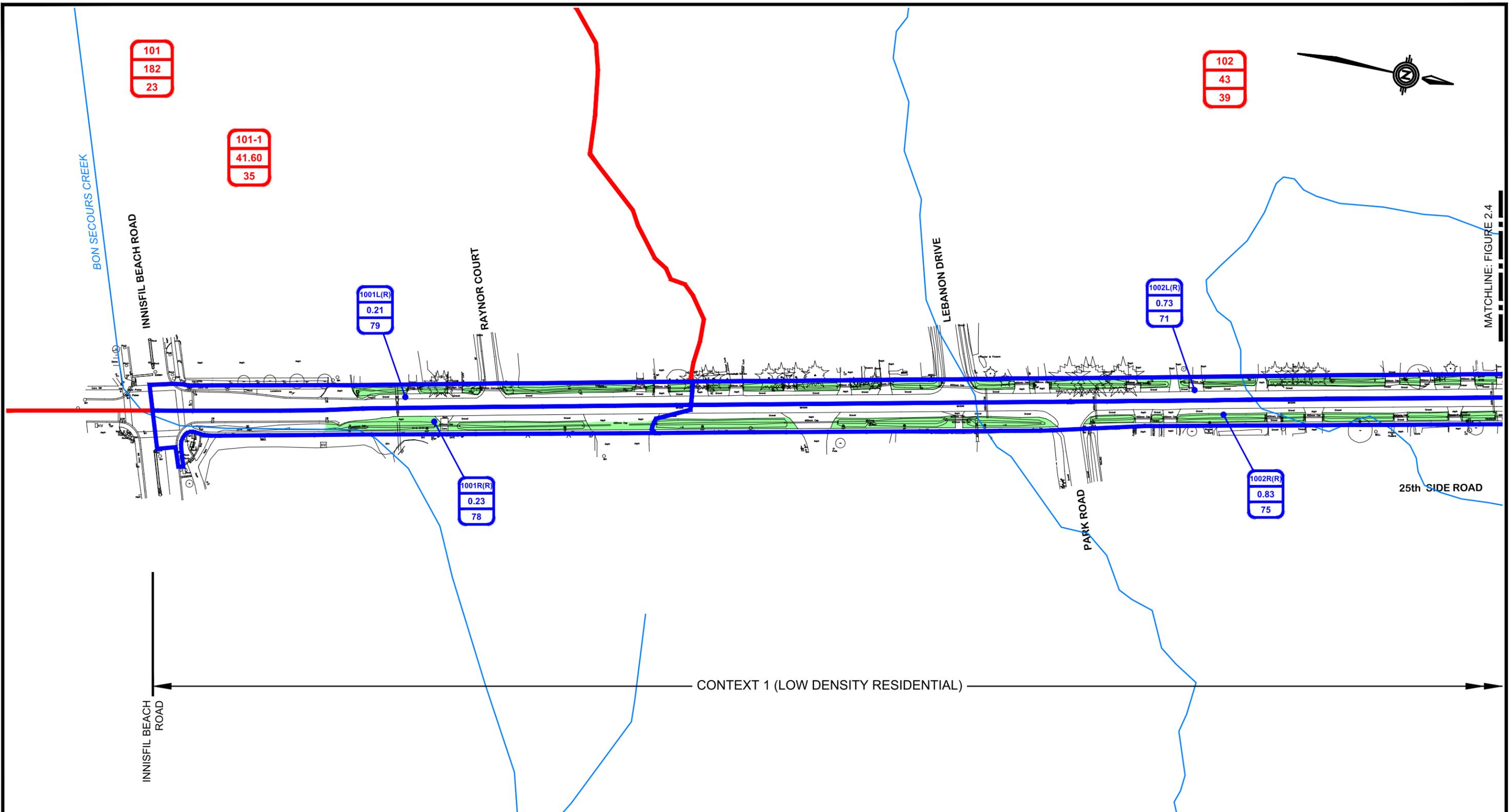
Roadway runoff from the west side of 25th Side Road is discharged into the existing culverts along 25th Side Road prior to being discharged into Lake Simcoe southerly. Under existing conditions, the left and right sides of the road were delineated into 12 catchments as shown in **Table 2-3**. A Visual OTTHYMO (VO) model was simulated to estimate the existing peak flow rates for road drainage areas. Peak flow rates for the 4-hour Chicago and 12-hour SCS Type II design storms were determined for the 25- and 100-year return periods (**Table 2-3**). More details are provided in **Appendix C**.

Table 2-3: Road Drainage Area Runoffs

Catchment Name	Watercourse Name	Area (ha)	Imp (%)	Ex. Culvert		25- year 12 hr SCS II (m ³ /s)	100- year 12 hr SCS II (m ³ /s)	25- year 4- hr Chicago (m ³ /s)	100- year 4- hr Chicago (m ³ /s)
				Span /d (m)	Height /d (m)				
1001L (R)	Bon Secours	0.21	79	1.8	1.8	0.066	0.083	0.077	0.098
1001R (R)		0.23	78			0.072	0.091	0.084	0.107
1002L (R)	N/A	0.73	71	0.5	0.5	0.218	0.282	0.253	0.314
				0.6	0.6				
1002R (R)		0.83	75	0.5	0.5	0.258	0.324	0.296	0.366
				0.6	0.6				
1003L (R)	Leonard's Creek	0.38	66	1.1	1.65	0.112	0.142	0.124	0.159
1003R (R)		0.39	79	1.1	1.65	0.123	0.154	0.143	0.182
1004L (R)	Leonard's Creek	0.35	69	0.8	0.8	0.104	0.135	0.12	0.149
1004R (R)		0.38	74	0.8	0.8	0.118	0.148	0.135	0.167
1005L (R)	N/A	0.37	66	1.2	1.2	0.118	0.138	0.135	0.155
1005R (R)		0.36	62	0.8	0.8	0.104	0.133	0.114	0.142
1006L (R)	N/A	0.65	65	0.5	0.7	0.190	0.241	0.209	0.269
1006R (R)		0.66	62	0.5	0.7	0.191	0.243	0.208	0.260
1007L (R)	N/A	0.72	60	0.8	0.8	0.210	0.263	0.220	0.280
1007R (R)		0.74	71	0.8	0.8	0.220	0.285	0.320	0.319

Catchment Name	Watercourse Name	Area (ha)	Imp (%)	Ex. Culvert		25- year 12 hr SCS II (m ³ /s)	100- year 12 hr SCS II (m ³ /s)	25- year 4- hr Chicago (m ³ /s)	100- year 4- hr Chicago (m ³ /s)
				Span /d (m)	Height /d (m)				
1008L (R)	Moosenlanka	0.21	52	0.7	1	0.059	0.075	0.061	0.078
1008R (R)				0.9	0.9				
1009L (R)	N/A	0.5	74	0.45	0.45	0.155	0.195	0.178	0.220
1009R (R)				0.63	71				
1100L (R)	Sandy Cove	0.91	55	6	3.2	0.260	0.326	0.270	0.340
1100R (R)				0.9	0.9				
1110L (R)	N/A	1.06	59	6	3.2	0.302	0.385	0.324	0.407
1110R (R)				0.9	0.9				
1120L (R)	N/A	1.21	68	0.4	0.4	0.360	0.461	0.410	0.510
1120R (R)				1.22	69				

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.3 C:\Users\bailey\ACD\docs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Side Road(SWM\CAD\FIGURES) Sep 10, 2021 - 9:28am



MATCHLINE: FIGURE 2.4

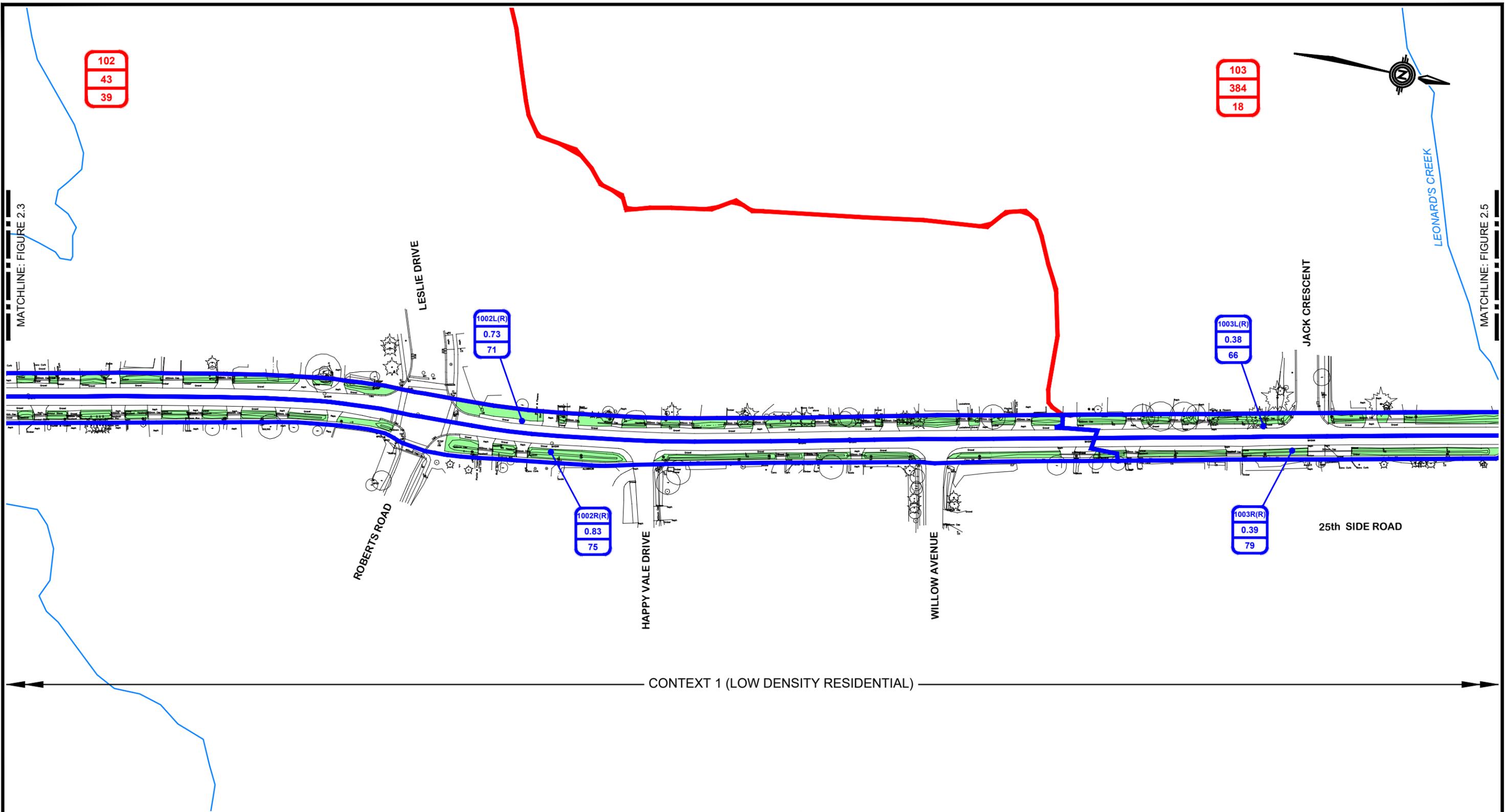
LEGEND

- CATCHMENT BOUNDARY
- ROAD CATCHMENT BOUNDARY
- PERVIOUS LANDSCAPE
- SETTLEMENT BOUNDARY
- WATERCOURSE
- 101 — CATCHMENT ID.
- 182 — DRAINAGE AREA (ha)
- 23 — IMPERVIOUSNESS (%)
- 1001L(R) — CATCHMENT ID.
- 0.21 — DRAINAGE AREA (ha)
- 79 — IMPERVIOUSNESS (%)



<p>CLIENT TOWN OF INNISFIL</p> <p>TITLE 25th SIDE ROAD, INNISFIL</p>		<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 50%;">Checked S.v.H.</td> <td style="width: 50%;">Drawn AutoCAD/B.K.B.</td> </tr> <tr> <td>Date SEPTEMBER 2021</td> <td>Proj. No. 211-06027-00</td> </tr> <tr> <td>Scale AS SHOWN</td> <td>Figure No. 2.3</td> </tr> </table>	Checked S.v.H.	Drawn AutoCAD/B.K.B.	Date SEPTEMBER 2021	Proj. No. 211-06027-00	Scale AS SHOWN	Figure No. 2.3
Checked S.v.H.	Drawn AutoCAD/B.K.B.							
Date SEPTEMBER 2021	Proj. No. 211-06027-00							
Scale AS SHOWN	Figure No. 2.3							
EXISTING ROAD DRAINAGE AREAS								

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.4 C:\Users\bailey\ACDdocs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Sideroad\SWM\CAD\FIGURES\ Sep 10, 2021 - 9:30am



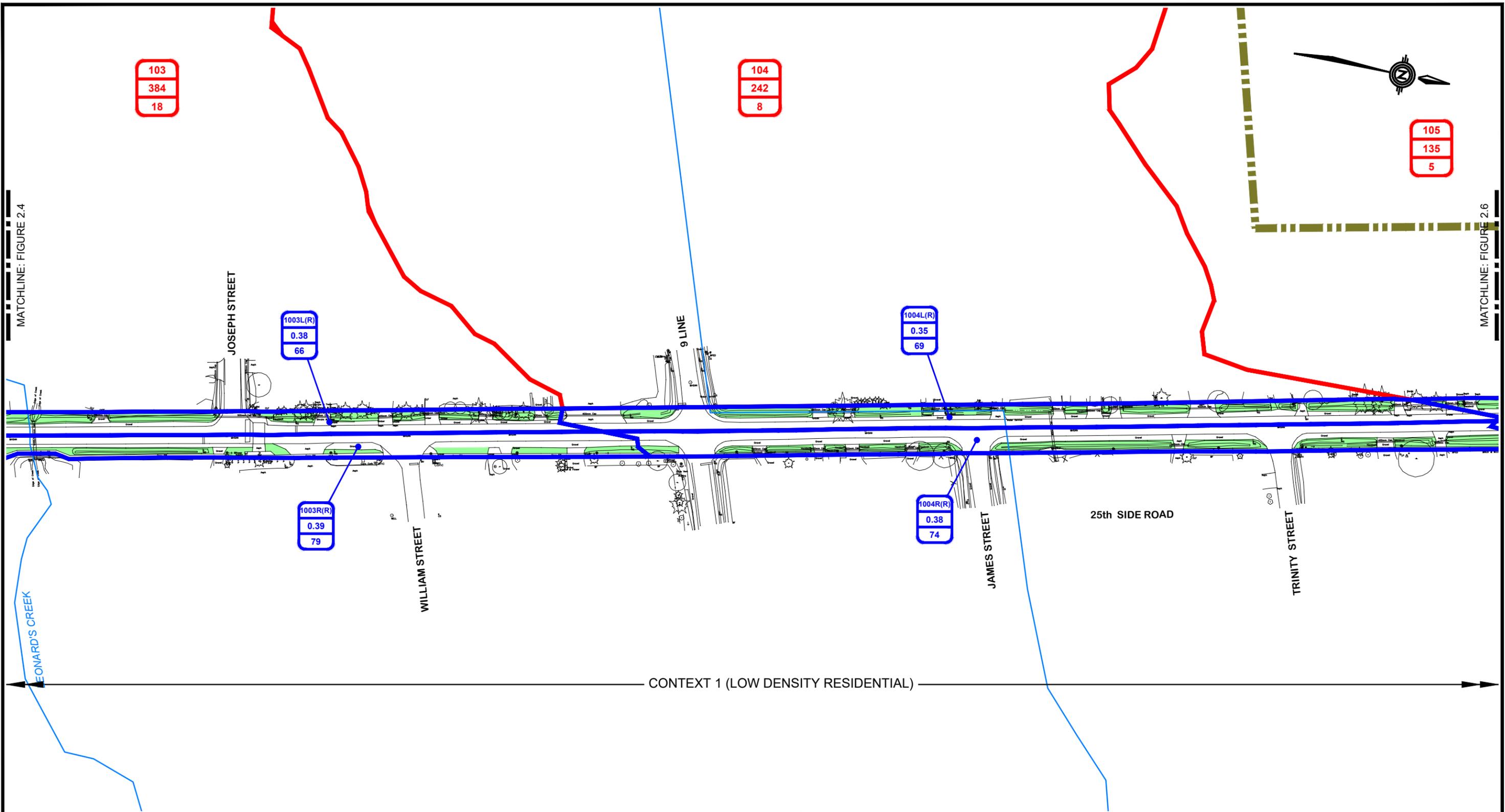
LEGEND

- CATCHMENT BOUNDARY
 - ROAD CATCHMENT BOUNDARY
 - PERVIOUS LANDSCAPE
 - SETTLEMENT BOUNDARY
 - WATERCOURSE
- | | | |
|--|---|--|
| <ul style="list-style-type: none"> 101 — CATCHMENT ID. 182 — DRAINAGE AREA (ha) 23 — IMPERVIOUSNESS (%) | <ul style="list-style-type: none"> 1001L(R) — CATCHMENT ID. 0.21 — DRAINAGE AREA (ha) 79 — IMPERVIOUSNESS (%) | <ul style="list-style-type: none"> 1002L(R) — CATCHMENT ID. 0.73 — DRAINAGE AREA (ha) 71 — IMPERVIOUSNESS (%) |
|--|---|--|



CLIENT	TOWN OF INNISFIL	
TITLE	25th SIDE ROAD, INNISFIL	
EXISTING ROAD DRAINAGE AREAS		
Checked	S.v.H.	Drawn
Date	SEPTEMBER 2021	AutoCAD/B.K.B.
Scale	AS SHOWN	Proj. No. 211-06027-00
		Figure No. 2.4

FIGURE 2.3 - 2.14 - 25th SR Drainage Areas.dwg - Figure 2.5 C:\Users\bailey\ACDdocs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Infill 25 Sideroad\SWM\CAD\FIGURES\ Sep 10, 2021 - 9:31am



LEGEND

- CATCHMENT BOUNDARY
- ROAD CATCHMENT BOUNDARY
- PERVIOUS LANDSCAPE
- SETTLEMENT BOUNDARY
- WATERCOURSE
- 101 CATCHMENT ID.
- 182 DRAINAGE AREA (ha)
- 23 IMPERVIOUSNESS (%)
- 1001L(R) CATCHMENT ID.
- 0.21 DRAINAGE AREA (ha)
- 79 IMPERVIOUSNESS (%)



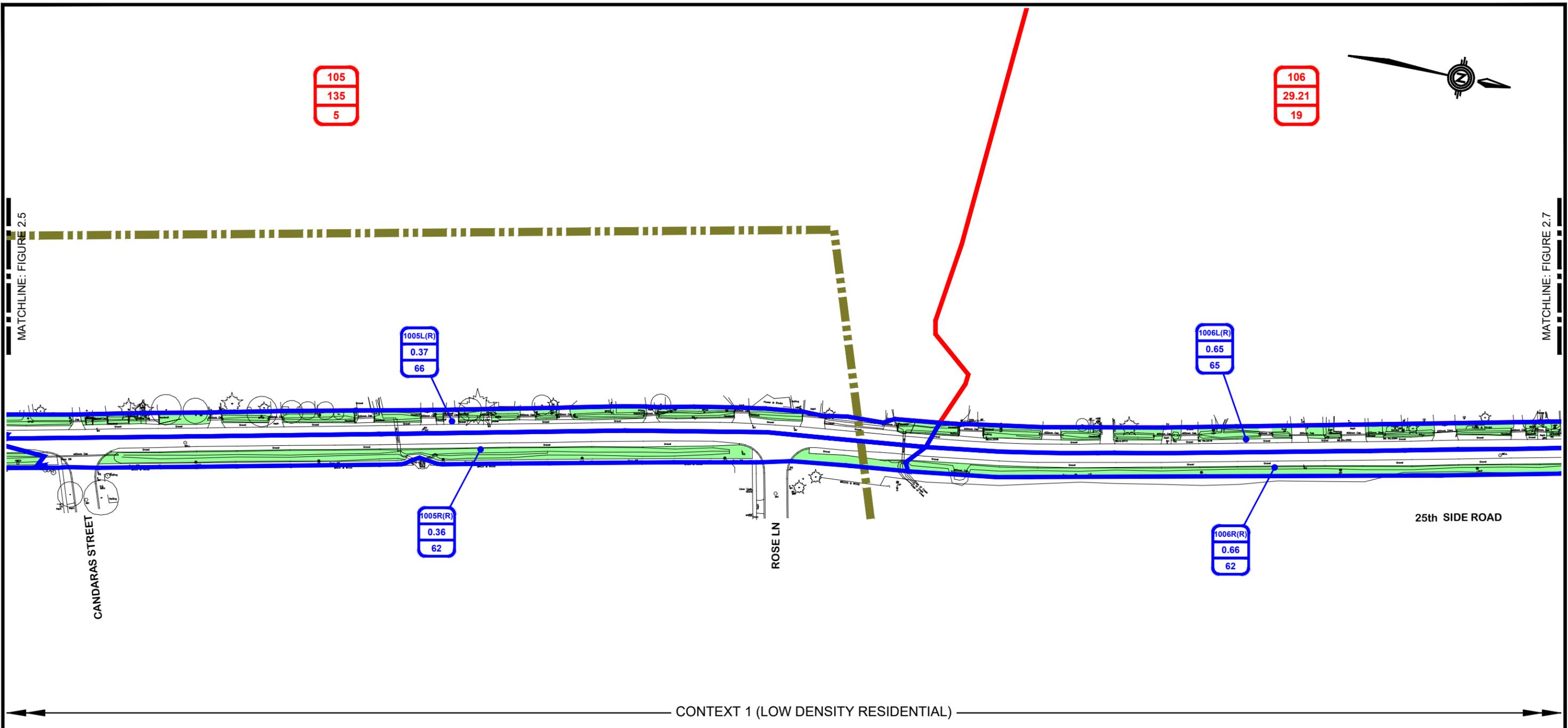
CLIENT	TOWN OF INNISFIL	
TITLE	25th SIDE ROAD, INNISFIL	
EXISTING ROAD DRAINAGE AREAS		
Checked	S.v.H.	Drawn
Date	SEPTEMBER 2021	AutoCAD/B.K.B.
Scale	AS SHOWN	Proj. No. 211-06027-00
		Figure No. 2.5



MATCHLINE: FIGURE 2.4

MATCHLINE: FIGURE 2.6

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.6 C:\Users\bailey\ACCDocs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Side Road\SWM\CAD\FIGURES\ Sep 10, 2021 - 9:32am



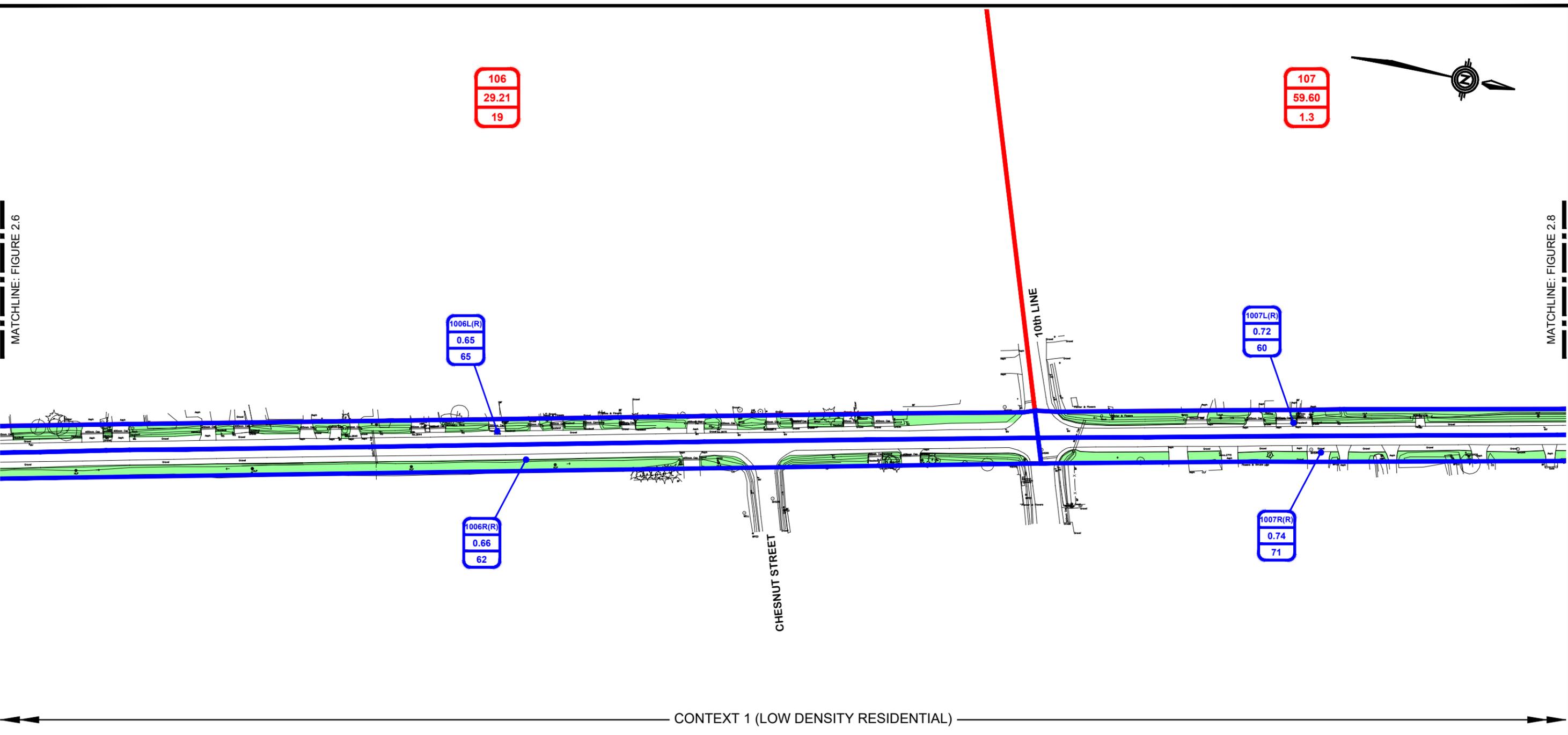
LEGEND

- CATCHMENT BOUNDARY
 - ROAD CATCHMENT BOUNDARY
 - PERVIOUS LANDSCAPE
 - SETTLEMENT BOUNDARY
 - WATERCOURSE
- | | |
|--|--|
| <ul style="list-style-type: none"> 101 — CATCHMENT ID. 182 — DRAINAGE AREA (ha) 23 — IMPERVIOUSNESS (%) | <ul style="list-style-type: none"> 1001L (R) — CATCHMENT ID. 0.21 — DRAINAGE AREA (ha) 79 — IMPERVIOUSNESS (%) |
|--|--|
- 0 10 25 50 75m

CLIENT	TOWN OF INNISFIL	
TITLE	25th SIDE ROAD, INNISFIL	
EXISTING ROAD DRAINAGE AREAS		
Checked	S.v.H.	Drawn
Date	SEPTEMBER 2021	AutoCAD/B.K.B.
Scale	AS SHOWN	Proj. No. 211-06027-00
		Figure No. 2.6



FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.7 C:\Users\bailey\ACDdocs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Inisfil 25 Sideroad\SWM\CAD\FIGURES\ Sep 10, 2021 - 9:32am



CONTEXT 1 (LOW DENSITY RESIDENTIAL)

LEGEND

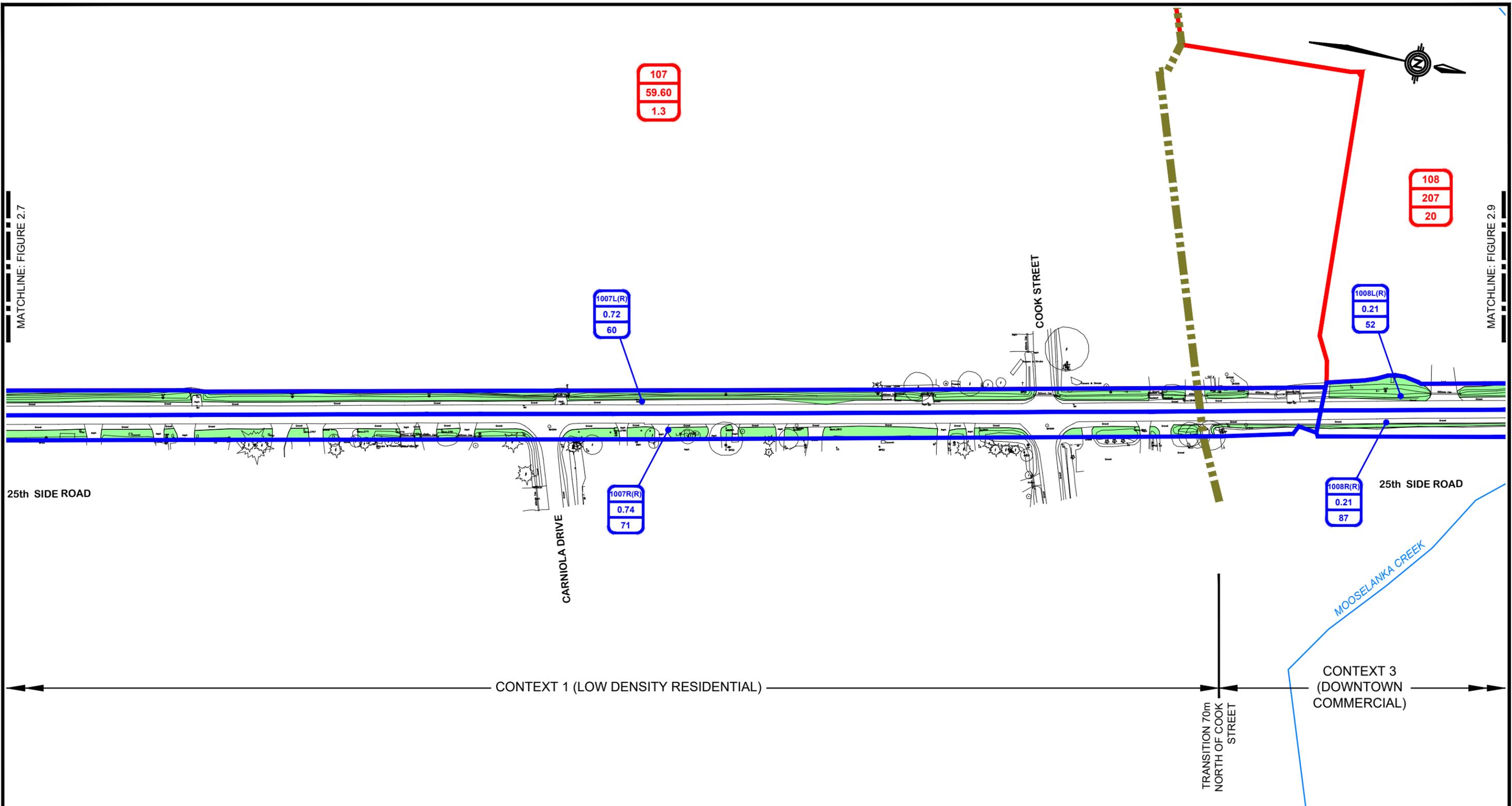
- CATCHMENT BOUNDARY
- ROAD CATCHMENT BOUNDARY
- PERVIOUS LANDSCAPE
- SETTLEMENT BOUNDARY
- WATERCOURSE
- 101 — CATCHMENT ID.
- 182 — DRAINAGE AREA (ha)
- 23 — IMPERVIOUSNESS (%)
- 1001L (R) — CATCHMENT ID.
- 0.21 — DRAINAGE AREA (ha)
- 79 — IMPERVIOUSNESS (%)



CLIENT	TOWN OF INNISFIL
TITLE	25th SIDE ROAD, INNISFIL
EXISTING ROAD DRAINAGE AREAS	

Checked S.v.H.	Drawn AutoCAD/B.K.B.
Date SEPTEMBER 2021	Proj. No. 211-06027-00
Scale AS SHOWN	Figure No. 2.7

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.8 C:\Users\bailey\ACCDocs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Side Road\SWM\CAD\FIGURES\ Sep 10, 2021 - 9:33am



LEGEND

- CATCHMENT BOUNDARY
- ROAD CATCHMENT BOUNDARY
- PERVIOUS LANDSCAPE
- SETTLEMENT BOUNDARY
- WATERCOURSE

Catchment Data Legend:

Box Color	Content
Red	CATCHMENT ID.
Red	DRAINAGE AREA (ha)
Red	IMPERVIOUSNESS (%)
Blue	CATCHMENT ID.
Blue	DRAINAGE AREA (ha)
Blue	IMPERVIOUSNESS (%)

0 10 25 50 75m

CLIENT: TOWN OF INNISFIL

TITLE: 25th SIDE ROAD, INNISFIL

EXISTING ROAD DRAINAGE AREAS

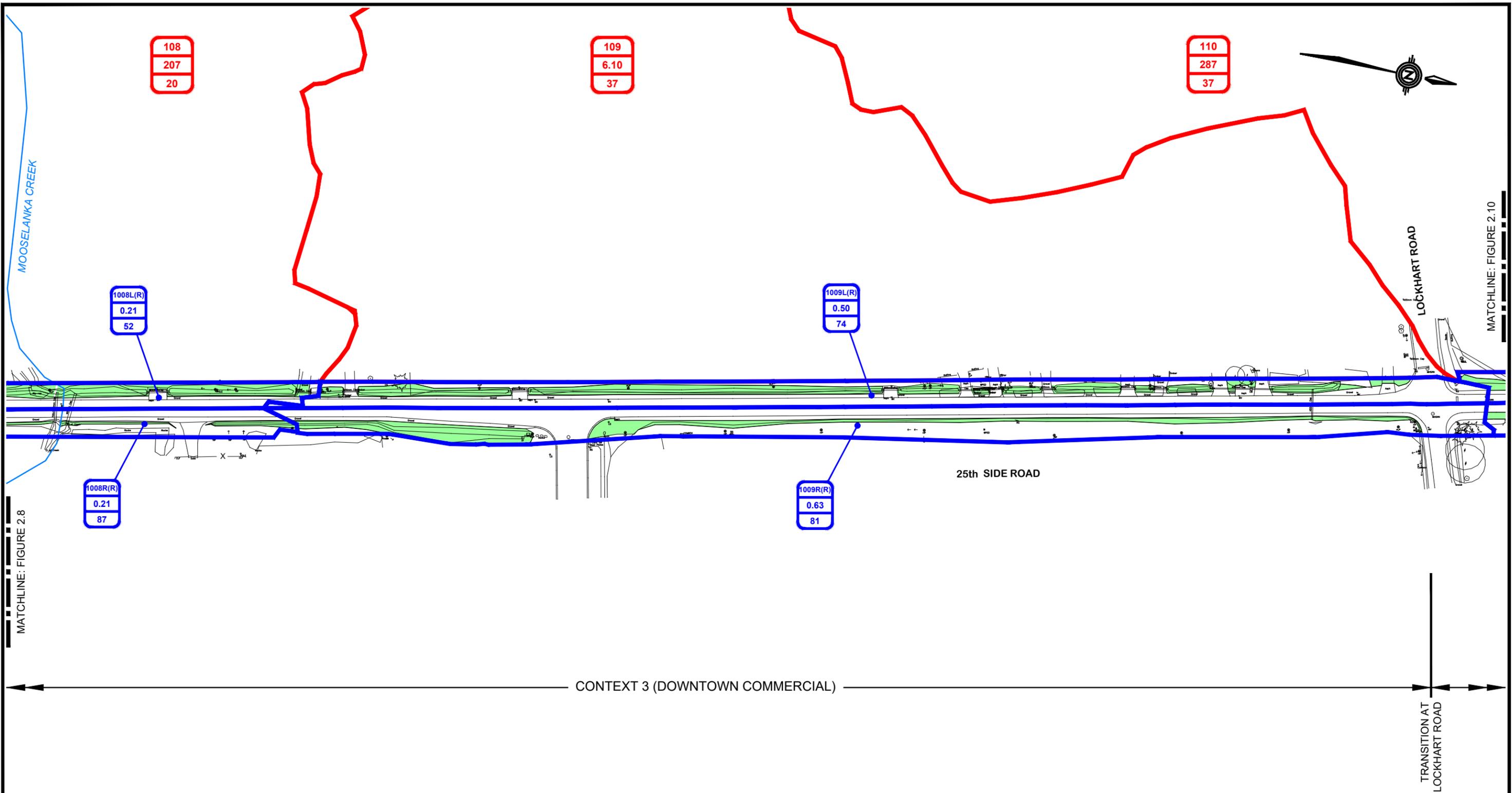
wsp

Checked S.v.H.	Drawn AutoCAD/B.K.B.
Date SEPTEMBER 2021	Proj. No. 211-06027-00
Scale AS SHOWN	Figure No. 2.8

MATCHLINE: FIGURE 2.7

MATCHLINE: FIGURE 2.9

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.9 C:\Users\bailey\ACCDocs\WSP_Canada\projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Side Road\SWM\CAD\FIGURES\ Sep 10, 2021 - 9:34am

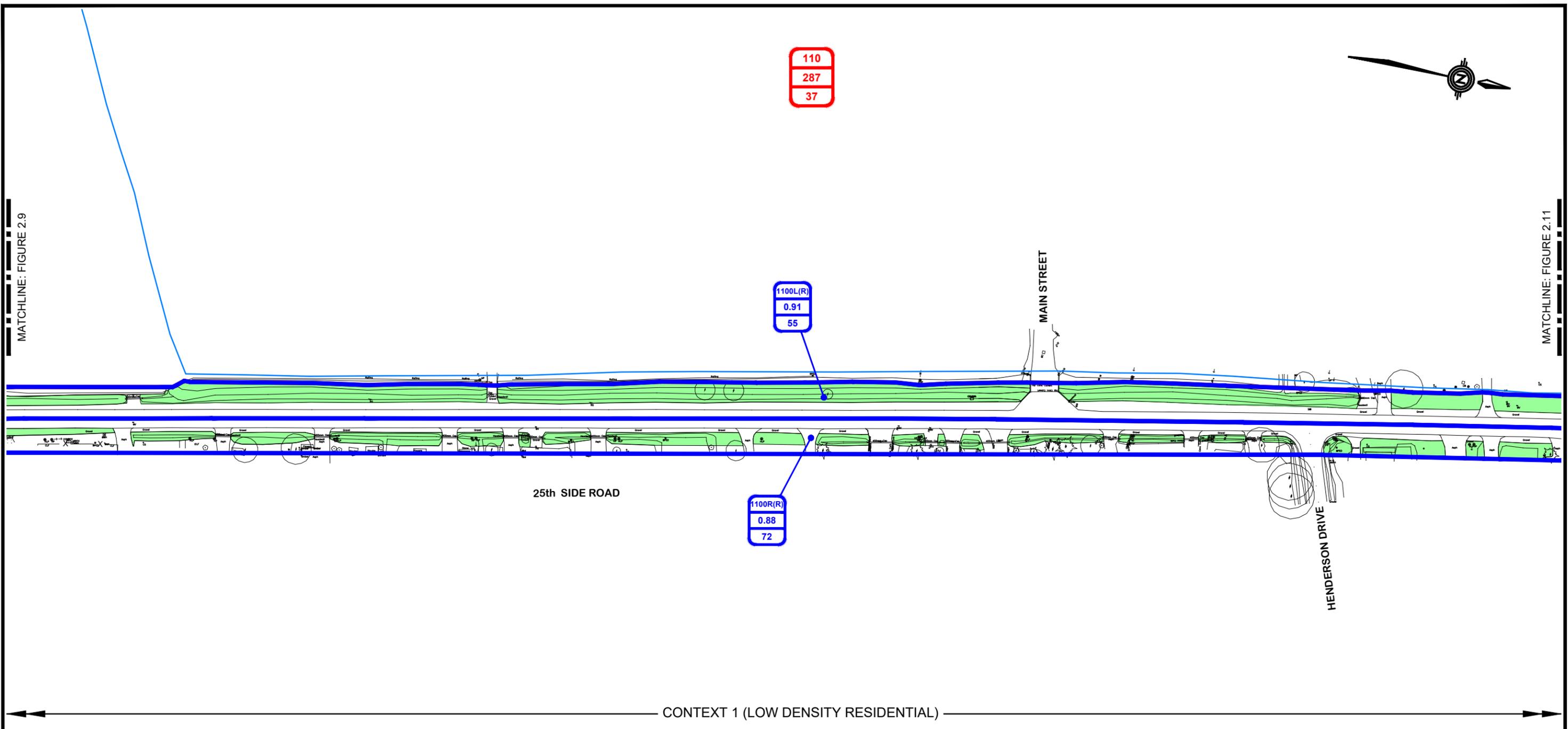


LEGEND

- CATCHMENT BOUNDARY
 - ROAD CATCHMENT BOUNDARY
 - PERVIOUS LANDSCAPE
 - SETTLEMENT BOUNDARY
 - WATERCOURSE
 - CATCHMENT ID.
 - CATCHMENT ID.
 - DRAINAGE AREA (ha)
 - DRAINAGE AREA (ha)
 - IMPERVIOUSNESS (%)
 - IMPERVIOUSNESS (%)
- 0 10 25 50 75m

CLIENT	TOWN OF INNISFIL		
TITLE	25th SIDE ROAD, INNISFIL		
EXISTING ROAD DRAINAGE AREAS			Checked S.v.H. Date SEPTEMBER 2021 Scale AS SHOWN
			Drawn AutoCAD/B.K.B. Proj. No. 211-06027-00 Figure No. 2.9

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.10 C:\Users\bailey\ACC\Docs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00_P-20-24_Innisfil_25 Side Road\SWM\CAD\FIGURES - Sep 10, 2021 - 9:34am



CONTEXT 1 (LOW DENSITY RESIDENTIAL)

LEGEND

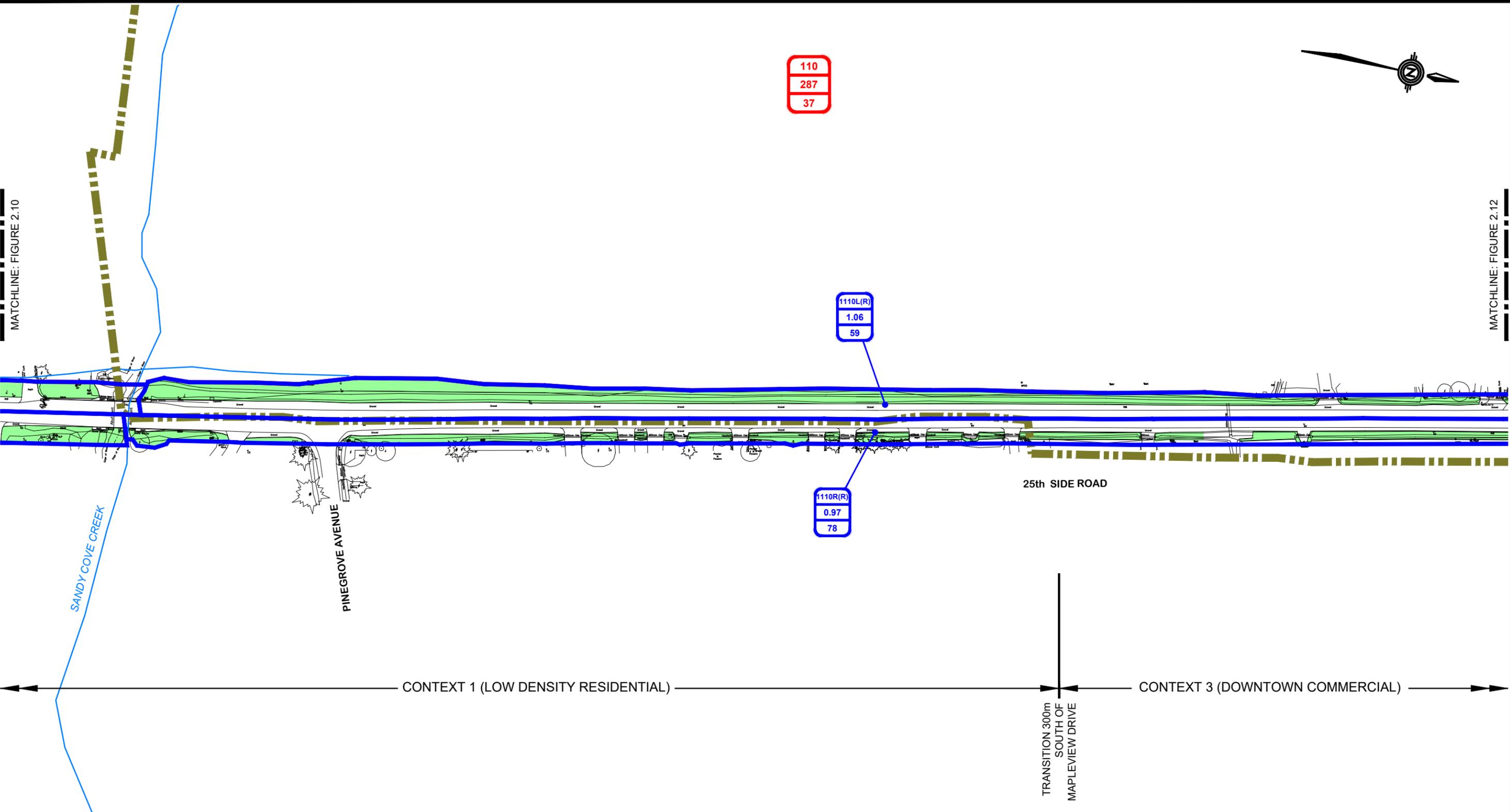
- CATCHMENT BOUNDARY
- ROAD CATCHMENT BOUNDARY
- PERVIOUS LANDSCAPE
- SETTLEMENT BOUNDARY
- WATERCOURSE
- 101 — CATCHMENT ID.
- 1001L (R) — CATCHMENT ID.
- 182 — DRAINAGE AREA (ha)
- 0.21 — DRAINAGE AREA (ha)
- 23 — IMPERVIOUSNESS (%)
- 79 — IMPERVIOUSNESS (%)



CLIENT	TOWN OF INNISFIL
TITLE	25th SIDE ROAD, INNISFIL
EXISTING ROAD DRAINAGE AREAS	

Checked S.v.H.	Drawn AutoCAD/B.K.B.
Date SEPTEMBER 2021	Proj. No. 211-06027-00
Scale AS SHOWN	Figure No. 2.10

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.11 C:\Users\bailey\ACC\Docs\WSP_Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Side Road\SWM\CAD\FIGURES - Sep 10, 2021 - 9:35am



110
287
37

1110L(R)
1.06
59

1110R(R)
0.97
78

25th SIDE ROAD

CONTEXT 1 (LOW DENSITY RESIDENTIAL)

CONTEXT 3 (DOWNTOWN COMMERCIAL)

TRANSITION 300m
SOUTH OF
MAPLEVIEW DRIVE

LEGEND

- CATCHMENT BOUNDARY
- ROAD CATCHMENT BOUNDARY
- PERVIOUS LANDSCAPE
- SETTLEMENT BOUNDARY
- WATERCOURSE
- 101 — CATCHMENT ID.
- 1001L (R) — CATCHMENT ID.
- 182 — DRAINAGE AREA (ha)
- 0.21 — DRAINAGE AREA (ha)
- 23 — IMPERVIOUSNESS (%)
- 79 — IMPERVIOUSNESS (%)

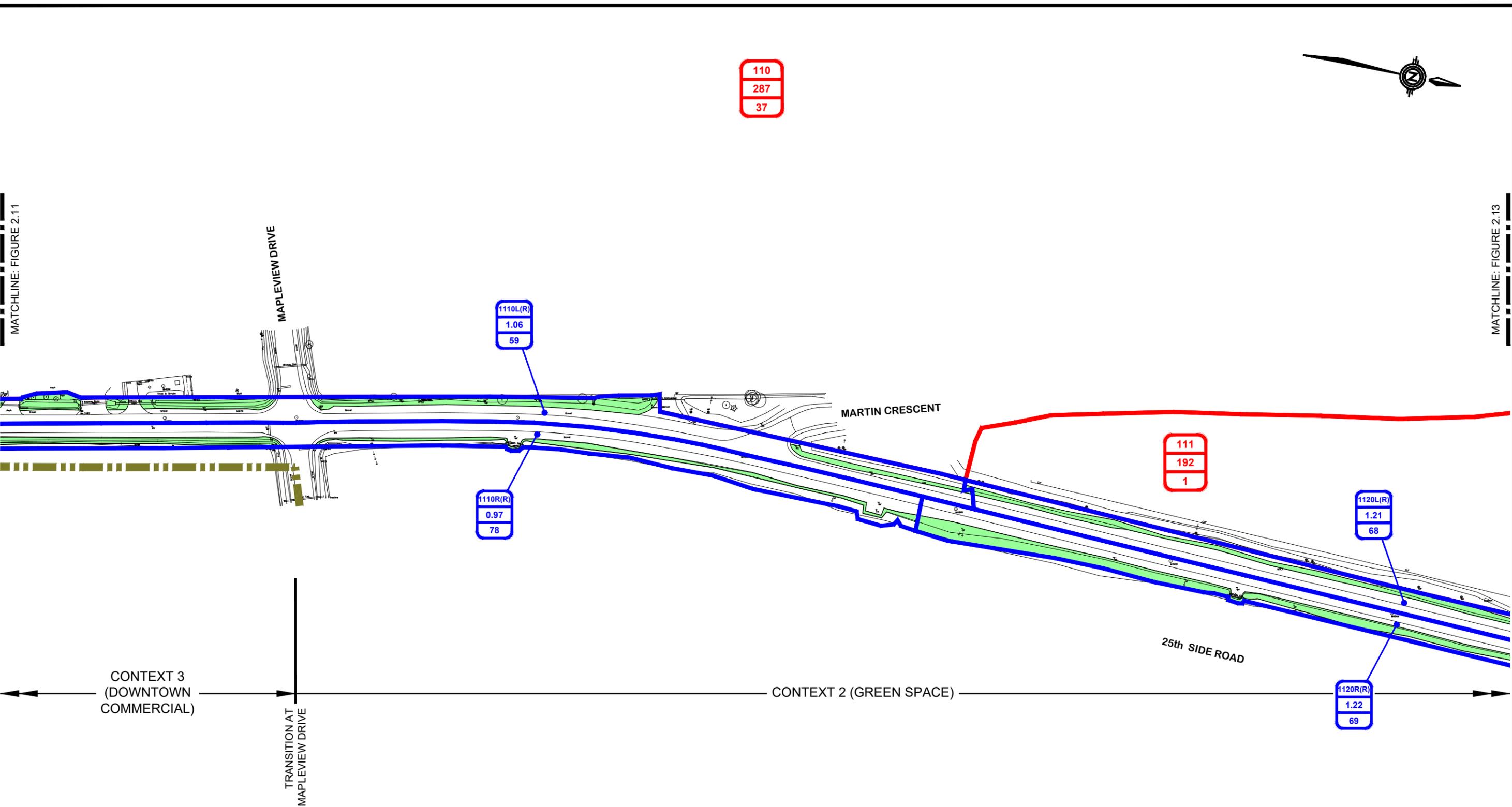


CLIENT	TOWN OF INNISFIL
TITLE	25th SIDE ROAD, INNISFIL
EXISTING ROAD DRAINAGE AREAS	



Checked S.v.H.	Drawn AutoCAD/B.K.B.
Date SEPTEMBER 2021	Proj. No. 211-06027-00
Scale AS SHOWN	Figure No. 2.11

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.12 C:\Users\bailey\ACC\Docs\WSP - Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Sideroad\SWM\CAD\FIGURES - Sep 10, 2021 - 9:36am



LEGEND

	CATCHMENT BOUNDARY		ROAD CATCHMENT BOUNDARY		PERVIOUS LANDSCAPE
	CATCHMENT ID. DRAINAGE AREA (ha) IMPERVIOUSNESS (%)		CATCHMENT ID. DRAINAGE AREA (ha) IMPERVIOUSNESS (%)		SETTLEMENT BOUNDARY
					WATERCOURSE

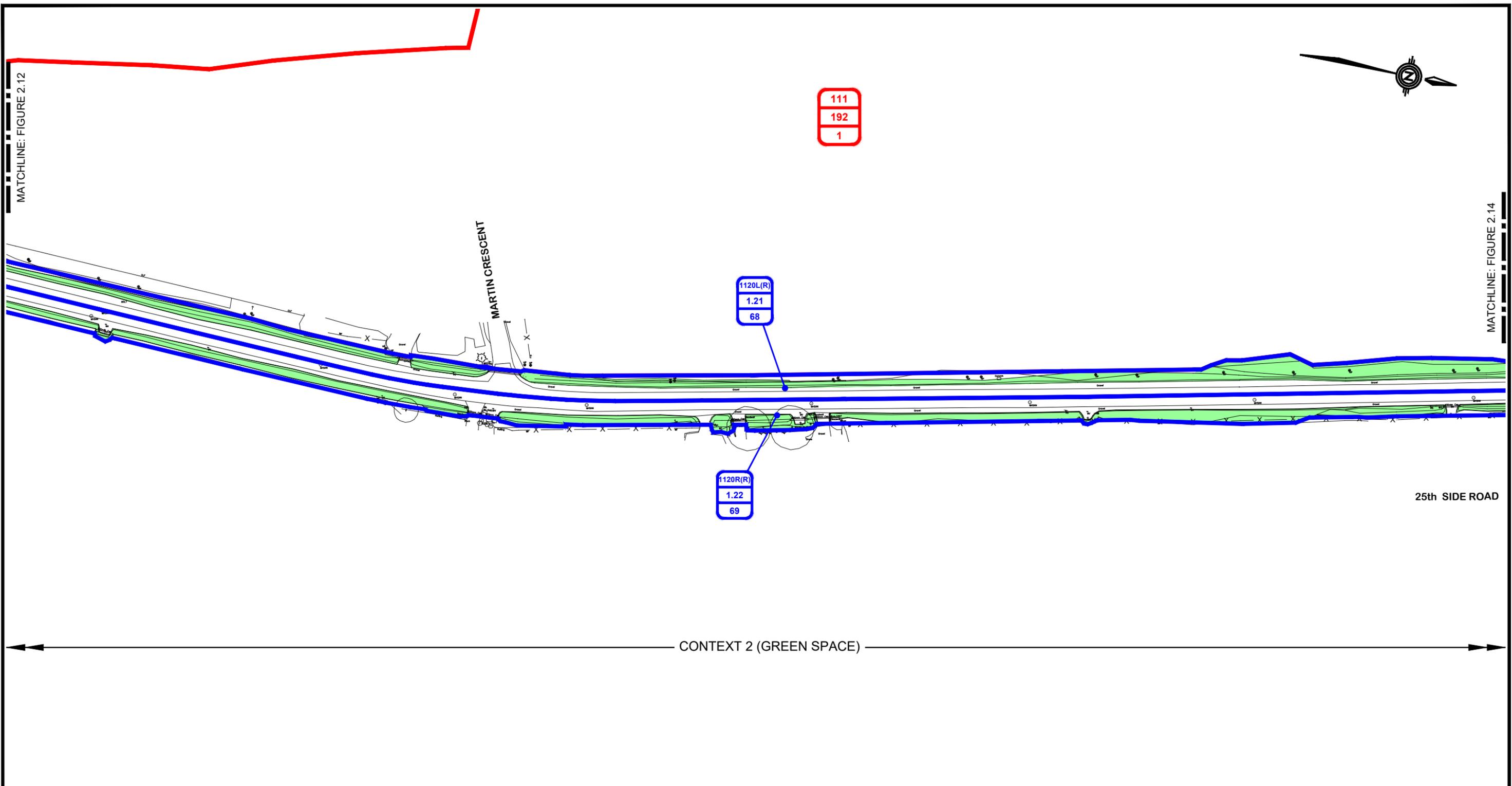
0 10 25 50 75m

CLIENT	TOWN OF INNISFIL	
TITLE	25th SIDE ROAD, INNISFIL	
EXISTING ROAD DRAINAGE AREAS		



Checked	S.v.H.	Drawn	AutoCAD/B.K.B.
Date	SEPTEMBER 2021	Proj. No.	211-06027-00
Scale	AS SHOWN	Figure No.	2.12

FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.13 C:\Users\bailey\ACCDOCS\WSP - Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Sideroad\SWM\CAD\FIGURES - Sep 10, 2021 - 9:37am



LEGEND

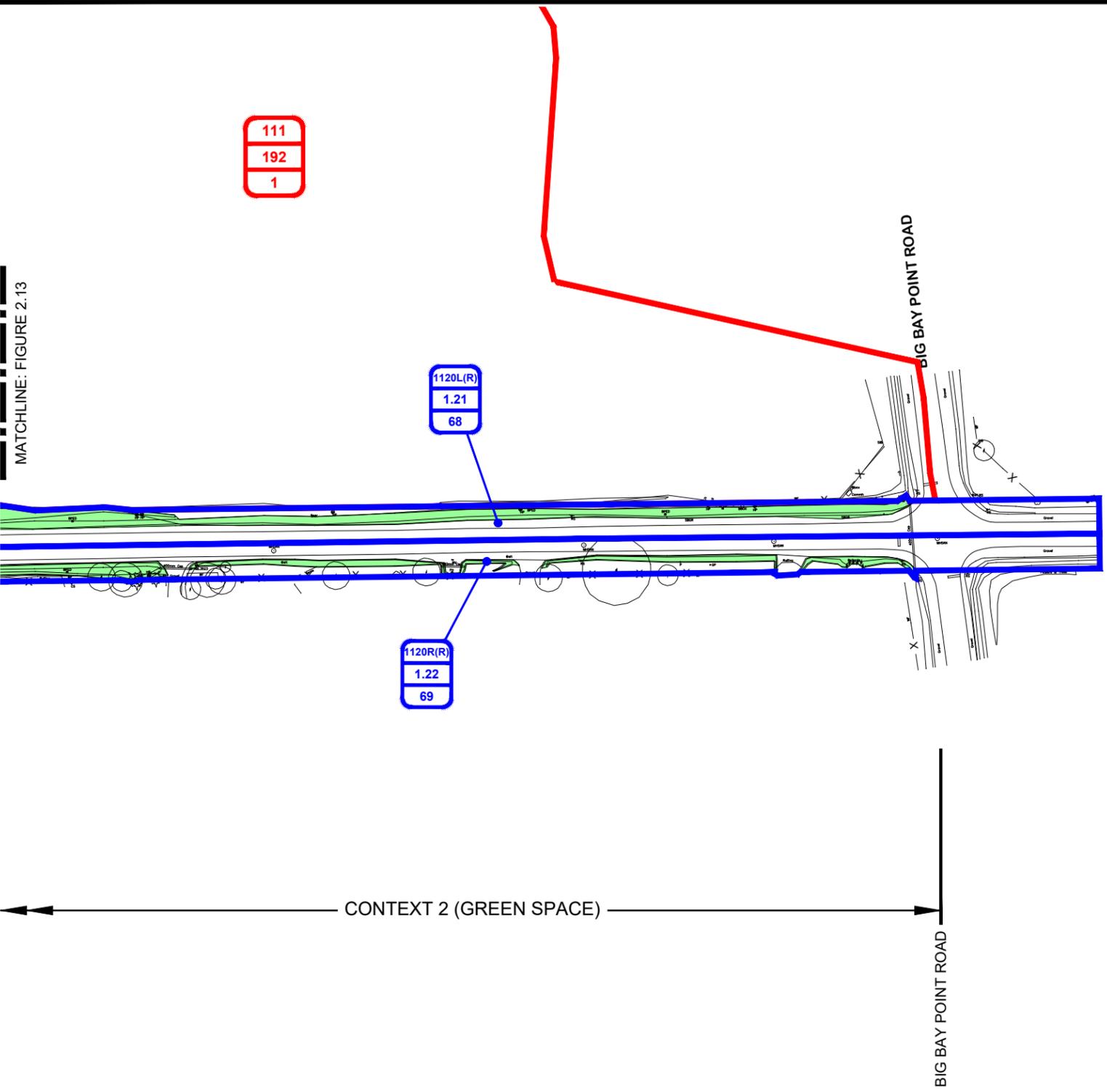
- CATCHMENT BOUNDARY
- ROAD CATCHMENT BOUNDARY
- PERVIOUS LANDSCAPE
- SETTLEMENT BOUNDARY
- WATERCOURSE
- 101 — CATCHMENT ID.
- 1001L (R) — CATCHMENT ID.
- 182 — DRAINAGE AREA (ha)
- 0.21 — DRAINAGE AREA (ha)
- 23 — IMPERVIOUSNESS (%)
- 79 — IMPERVIOUSNESS (%)



CLIENT	TOWN OF INNISFIL	
TITLE	25th SIDE ROAD, INNISFIL	
EXISTING ROAD DRAINAGE AREAS		
Checked	S.v.H.	Drawn
Date	SEPTEMBER 2021	AutoCAD/B.K.B.
Scale	AS SHOWN	Proj. No. 211-06027-00
		Figure No. 2.13



FIGURE 2.3 - 2.14 - 25th SR Innisfil - Ex Rd Drainage Areas.dwg - Figure 2.14 C:\Users\bailey\ACCDOCS\WSP Canada projects (AMER)\Land Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Sideroad\SWM\CAD\FIGURES - Sep 10, 2021 - 9:37am



LEGEND

	CATCHMENT BOUNDARY		ROAD CATCHMENT BOUNDARY		PERVIOUS LANDSCAPE
	CATCHMENT ID. DRAINAGE AREA (ha) IMPERVIOUSNESS (%)		CATCHMENT ID. DRAINAGE AREA (ha) IMPERVIOUSNESS (%)		SETTLEMENT BOUNDARY
					WATERCOURSE

0 10 25 50 75m

CLIENT	TOWN OF INNISFIL	
TITLE	25th SIDE ROAD, INNISFIL	
EXISTING ROAD DRAINAGE AREAS		

Checked S.v.H.	Drawn AutoCAD/B.K.B.
Date SEPTEMBER 2021	Proj. No. 211-06027-00
Scale AS SHOWN	Figure No. 2.14

3 POST DEVELOPMENT CONDITIONS

Under post-development conditions, the drainage area for the west and east sides of 25th Side Road were delineated into 12 catchments (i.e., Catchments 2001, to 2012) as shown in **Table 3-1**. Please refer to **Figures 3-1 to 3-5** for the details of the post-development road drainage areas and **Figures 3-6** for the post-development drainage areas plan.

Table 3-1: Post-Development Conditions Road Drainage Areas

Catchment Name	Catchment Area (ha)	TIMP (%)	XIMP (%)	LID Footprint Area (ha)	Vegetated Area (ha)	Existing Culvert Size
Catchment 2001.1 L Context 3	0.100	90	90	0.006	0.01	Existing Storm Pipe
Catchment 2001.1 R Context 3	0.099	79	79	0.011	0.02	
Catchment 2001.2 L Context 1	0.11	91	91	0.01	0.01	(1800 mm and 900 mm diameter Existing Culverts)
Catchment 2001.2 R Context 1	0.13	86	86	0.02	0.02	
Catchment 2002 L Context 1	0.70	93	93	0.05	0.08	(500 mm diameter Existing Culvert)
Catchment 2002 R Context 1	0.80	88	88	0.09	0.12	
Catchment 2003 L Context 1	0.43	90	90	0.017	0.04	(1100 mm x 1500 mm diameter Existing Culvert)
Catchment 2003 R Context 1	0.47	87	87	0.04	0.06	
Catchment 2004 L Context 1	0.35	89	89	0.015	0.04	800 mm diameter Existing Culvert
Catchment 2004 R Context 1	0.36	85	85	0.05	0.05	

Catchment Name	Catchment Area (ha)	TIMP (%)	XIMP (%)	LID Footprint Area (ha)	Vegetated Area (ha)	Existing Culvert Size
Catchment 2005 L Context 1	0.28	90	90	0.01	0.03	1200 mm and 800 mm diameters Existing Culverts
Catchment 2005 R Context 1	0.29	78	78	0.05	0.06	
Catchment 2006 L Context 1	0.65	88	88	0.06	0.08	500 mm x 700 mm diameter Existing Culverts
Catchment 2006 R Context 1	0.73	80	80	0.14	0.15	
Catchment 2007 L Context 1	0.58	85	85	0.08	0.09	800 mm diameter Existing Culvert
Catchment 2007 R Context 1	0.64	84	84	0.07	0.11	
Catchment 2008-1 L Context 1	0.07	84	84	0.007	0.011	700 mm x 1000 mm diameter Existing Culverts and 900 mm diameter Existing Culvert
Catchment 2008-1 R Context 1	0.07	83	83	0.006	0.012	
Catchment 2008-2 L Context 3	0.38	70	70	0.07	0.117	
Catchment 2008-2 R Context 3	0.39	59	59	0.08	0.160	
Catchment 2009 L Context 3	0.40	77	77	0.06	0.093	450 mm diameter Existing Culvert
Catchment 2009 R Context 3	0.39	77	77	0.06	0.091	
Catchment 2010 L Context 3	0.25	82	82	0.05	0.04	Existing Bridge 6 m x 3.2 m
Catchment 2010 R Context 3	0.25	82	82	0.01	0.04	
Catchment 2010 L Context 1	0.47	74	74	0.06	0.12	

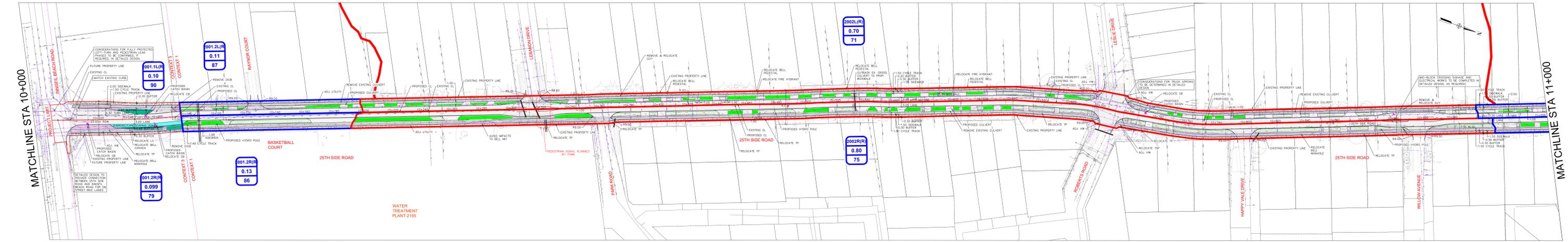
Catchment Name	Catchment Area (ha)	TIMP (%)	XIMP (%)	LID Footprint Area (ha)	Vegetated Area (ha)	Existing Culvert Size
Catchment 2010 R Context 1	0.47	85	85	0.03	0.07	
Catchment 2011 L Contexts 3 & 2	0.87	67	67	0.21	0.28	450 mm diameter Existing Culvert
Catchment 2011 R Contexts 3 & 2	0.90	80	80	0.10	0.18	
Catchment 2012 L Context 2	1.11	56	56	0.49	0.49	400 mm diameter Existing Culvert
Catchment 2012 R Context 2	1.11	81	81	0.21	0.21	

As mentioned previously, it is proposed to provide a preliminary stormwater management plan and a direction for designing a range of LID systems appropriate for the 25th Side Road study area. Typically, LID features are designed to implement the functions of natural drainage systems by attenuating, filtering and infiltrating stormwater runoff as close as possible to where it is generated. With this approach, stormwater runoff can be reduced substantially when coupled with appropriate environmental conditions.

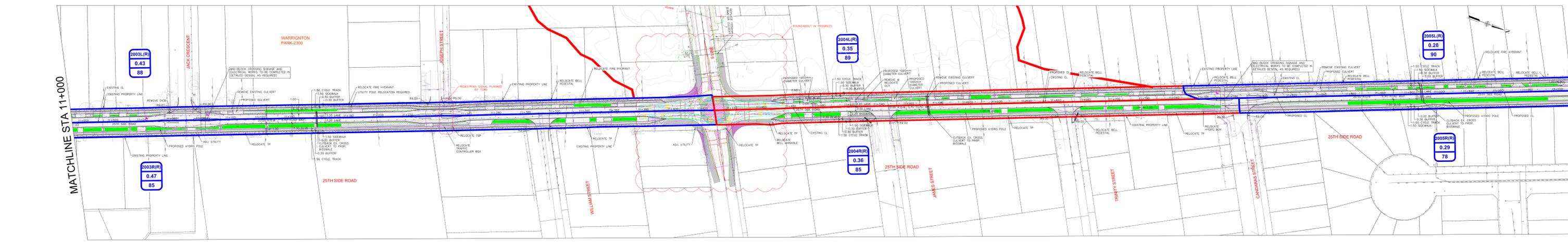
LID systems can be installed in areas where runoff is controlled or uncontrolled to provide additional stormwater quantity and quality control as well as promote infiltration and groundwater recharge. LID opportunities will depend on available space, soil and groundwater conditions. The applicable LID systems within Catchments 2001 to 2012 are:

- Bioswale
- Infiltration Chamber

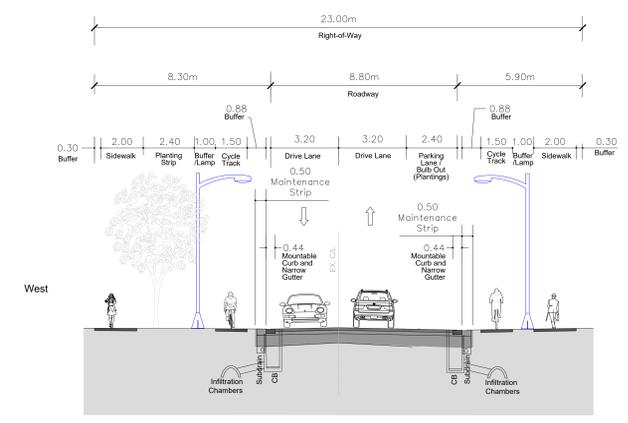
25th Side Road Plan view-1
From Innisfil Beach Road to North of Willow Avenue
Context 1 and Context 3



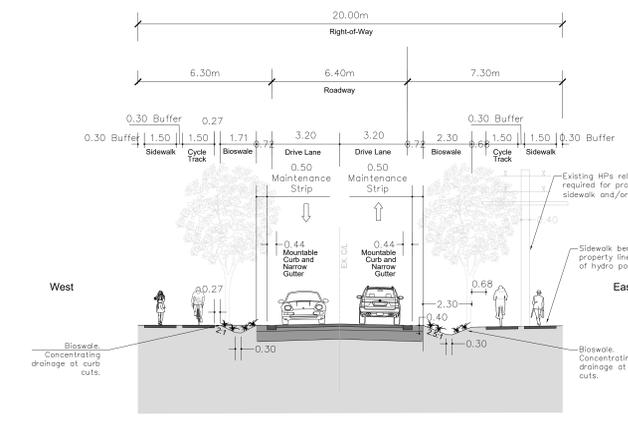
25th Side Road Plan view-2
From North of Willow Avenue to North of Candaras Street
Context 1



TYPICAL CROSS-SECTIONS



Context 3
(Downtown Commercial)
From Innisfil Beach Road to 80m North of Innisfil Beach



Context 1
(Low Density Residential)
From 80m North of Innisfil Beach to 70m North of Cook Street

TOWN OF INNISFIL
25TH SIDE ROAD

MAY, 2022

LEGEND

- PROPOSED ASPHALT ROADWAY (PARTIAL DEPTH)
- 1.5m - 2m PROPOSED CONCRETE SIDEWALK
- 1.5m - 2.0m PROPOSED CYCLE LANE
- PROPOSED HARDSCAPE BUFFER
- 3.0m PROPOSED MULTI USE PATH
- 2.4m PROPOSED BI-DIRECTIONAL CYCLE LANE
- PLANTING STRIP
- PROPOSED HYDRO POLE
- MISC. REMOVALS
- 2.4m PARKING/LANDSCAPING / MAINTENANCE STRIP
- 0.5m ASPHALT BUFFER / MAINTENANCE STRIP
- PROPOSED MOUNTABLE CURB
- 2.15m FURNISHING STRIP/ZONE
- EXISTING PROPERTY LINE
- FUTURE PROPERTY LINE
- 1.5m - 2m PROPOSED CROSS RIDE
- 2.5m PROPOSED CROSSWALK
- 0.3-0.9m PROPOSED SOD
- PROPOSED CULVERT
- REMOVE EXISTING CULVERT AND BEDDING MATERIALS
- PROPOSED CATCH BASIN
- TACTILE PLATE

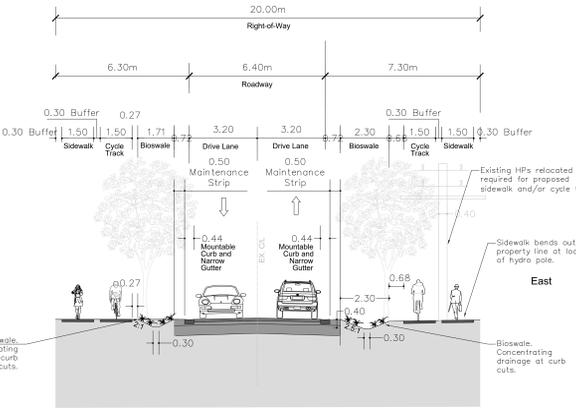
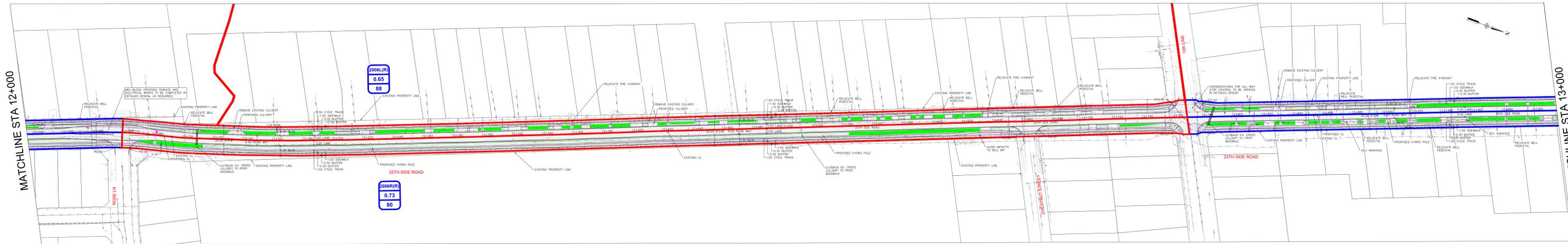
TITLE :
POST-DEVELOPMENT CONDITIONS
FIGURE 3-1

PRELIMINARY DESIGN

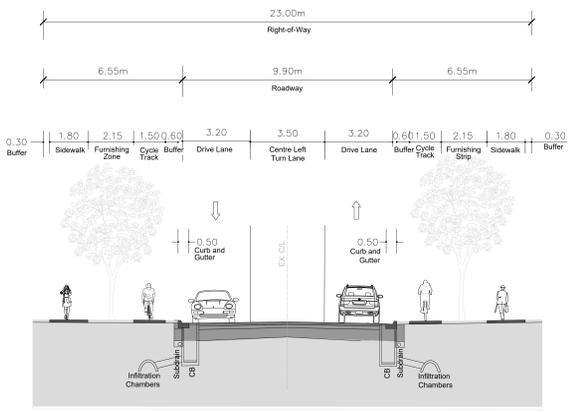
- DRIVEWAY PAVEMENT
- PROPOSED GRAVEL SHOULDER
- EXISTING 25TH SIDE ROAD CROSSING CULVERTS
- SUB-CATCHMENT BOUNDARY
- SUB-CATCHMENT ID, DRAINAGE AREA (ha), IMPERVIOSNESS (%)
- ROAD DRAINAGE AREA
- EXISTING WATER
- EXISTING SANITARY
- EXISTING GAS SERVICE
- EXISTING GAS
- FLOW DIRECTION
- CONTOURS
- UNDER GROUND INFILTRATION CHAMBER
- PROPOSED BIOSWALE

GENERAL NOTES:
SIGNAGE AND PARKING LIMITS TO BE COMPLETED IN DETAILED DESIGN

25th SideRoad Plan view-3
From North of Candaras Street to North of 10th Line
Context 1

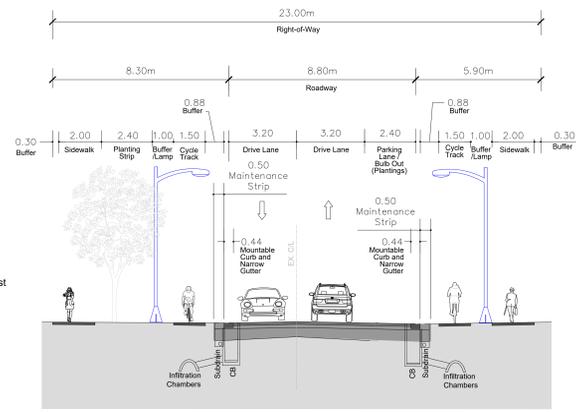
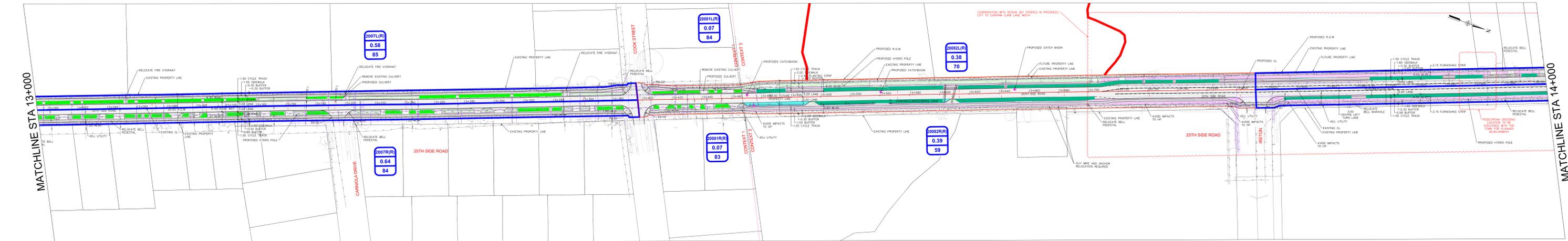


Context 1
(Low Density Residential)
From 80m North of Innisfil Beach to 70m North of Cook Street



Context 3
(Downtown Commercial)
From 450m South of Lockhart Road to 50m North of Lockhart

25th SideRoad Plan view-4
From North of 10th Line to North of Cook Street
Context 1 and Context 3



Context 3
(Downtown Commercial)
From 70m North of Cook Street to 450 m South of Lockhart Road

TOWN OF INNISFIL
25TH SIDE ROAD

MAY, 2022

LEGEND

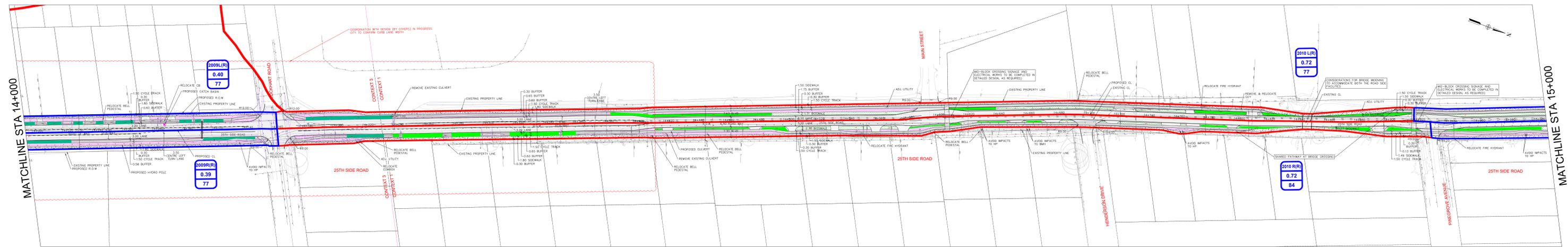
- PROPOSED ASPHALT ROADWAY (PARTIAL DEPTH)
- 1.5m - 2m PROPOSED CONCRETE SIDEWALK
- 1.5m - 2.0m PROPOSED CYCLE LANE
- PROPOSED HARDSCAPE BUFFER
- 3.0m PROPOSED MULTI USE PATH
- 2.4m PROPOSED BI-DIRECTIONAL CYCLE LANE
- PLANTING STRIP
- PROPOSED HYDRO POLE
- MISC. REMOVALS
- 2.4m PARKING/LANDSCAPING
- 0.5m ASPHALT BUFFER / MAINTENANCE STRIP
- PROPOSED MOUNTABLE CURB
- 2.15m FURNISHING STRIP/ZONE
- EXISTING PROPERTY LINE
- FUTURE PROPERTY LINE
- 1.5m - 2m PROPOSED CROSS RIDE
- 2.5m PROPOSED CROSSWALK
- 0.3-0.9m PROPOSED SOD
- PROPOSED CULVERT
- EXISTING WATER AND BEDDING MATERIALS
- PROPOSED CATCH BASIN
- TACTILE PLATE

TITLE :
POST-DEVELOPMENT CONDITIONS
FIGURE 3-2

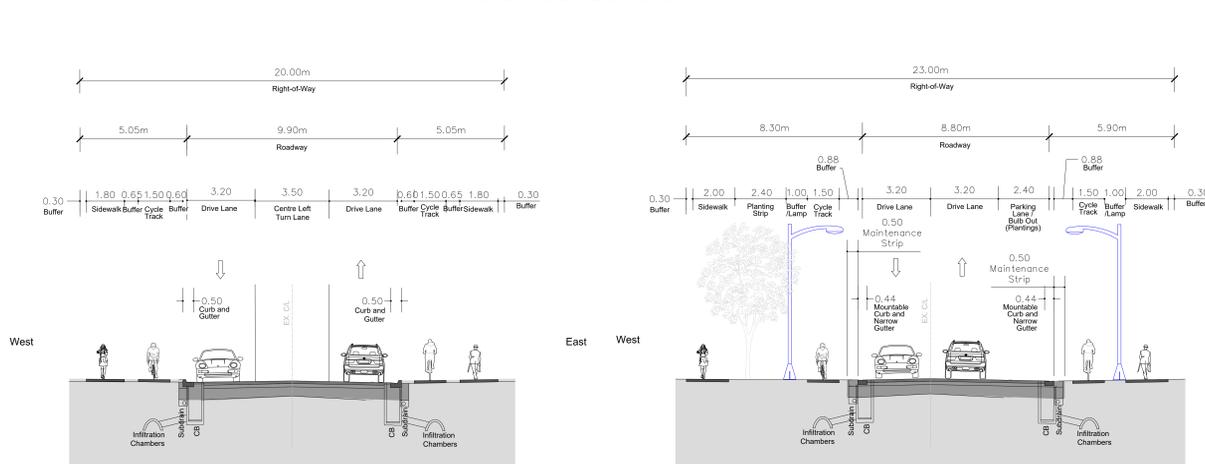
PRELIMINARY DESIGN

- DRIVEWAY PAVEMENT
 - PROPOSED GRAVEL SHOULDER
 - EXISTING 25TH SIDE ROAD CROSSING CULVERTS
 - SUB-CATCHMENT BOUNDARY
 - SUB-CATCHMENT ID, DRAINAGE AREA (ha), IMPERVIOUSNESS (%)
 - ROAD DRAINAGE AREA, ROAD DRAINAGE AREA
 - EXISTING WATER, EXISTING SANITARY, EXISTING GAS SERVICE, EXISTING GAS
 - FLOW DIRECTION
 - CONTOURS
 - UNDER GROUND INFILTRATION CHAMBER, PROPOSED BIOSWALE
- GENERAL NOTES:
SIGNAGE AND PARKING LIMITS TO BE COMPLETED IN DETAILED DESIGN

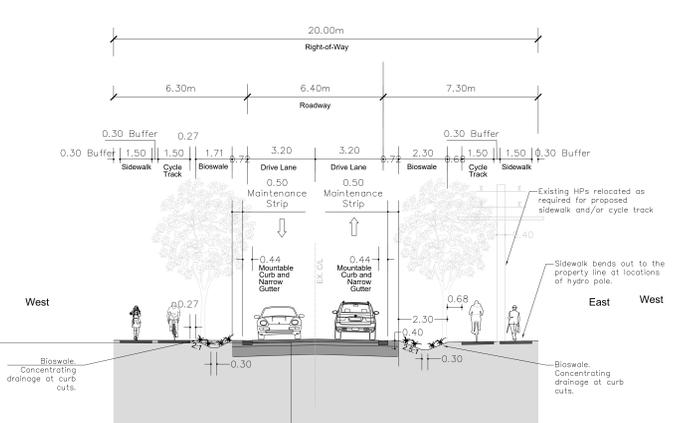
25th Side Road Plan view-5
From South of Lockhard St. to North of Pinegroove Ave.
Context 3 and Context 1



TYPICAL CROSS-SECTIONS

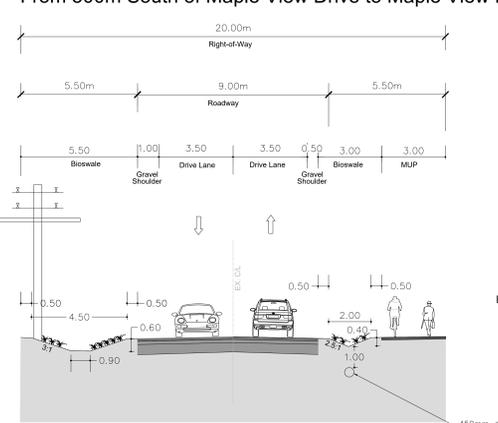


Context 1
(Low Density Residential)
From 50m North of Lockhart to 200m North of Lockhart



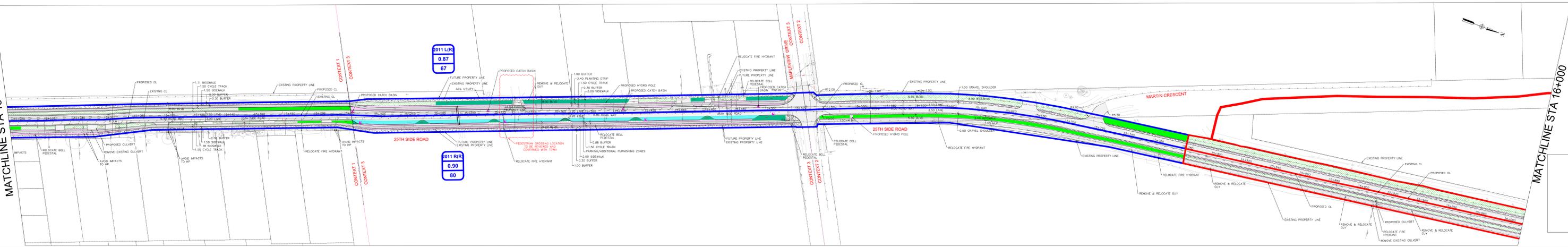
Context 1
(Low Density Residential)
From 200m North of Lockhart to 300m South of Maple View Drive

Context 3
(Downtown Commercial)
From 300m South of Maple View Drive to Maple View Drive



Context 2
(Green Space)
From Maple View Drive to Bay Point Road

25th Side Road Plan view-6
From North of Pinegroove Ave. to North of Martin Crescent
Context 1 Context 3 and Context 2



TOWN OF INNISFIL
25TH SIDE ROAD

MAY, 2022

LEGEND

- PROPOSED ASPHALT ROADWAY (PARTIAL DEPTH)
- 1.5m - 2m PROPOSED CONCRETE SIDEWALK
- 1.5m - 2.0m PROPOSED CYCLE LANE
- PROPOSED HARDSCAPE BUFFER
- 3.0m PROPOSED MULTI USE PATH
- 2.4m PROPOSED BI-DIRECTIONAL CYCLE LANE
- PLANTING STRIP
- PROPOSED HYDRO POLE
- MISC. REMOVALS
- 2.4m PARKING/LANDSCAPING / MAINTENANCE STRIP
- 0.5m ASPHALT BUFFER / MAINTENANCE STRIP
- PROPOSED MOUNTABLE CURB
- 2.15m FURNISHING STRIP/ZONE
- EXISTING PROPERTY LINE
- FUTURE PROPERTY LINE
- 1.5m - 2m PROPOSED CROSS RIDE
- 2.5m PROPOSED CROSSWALK
- 0.3-0.9m PROPOSED SOD
- PROPOSED CULVERT
- REMOVE EXISTING CULVERT AND BEDDING MATERIALS
- PROPOSED CATCH BASIN
- TACTILE PLATE

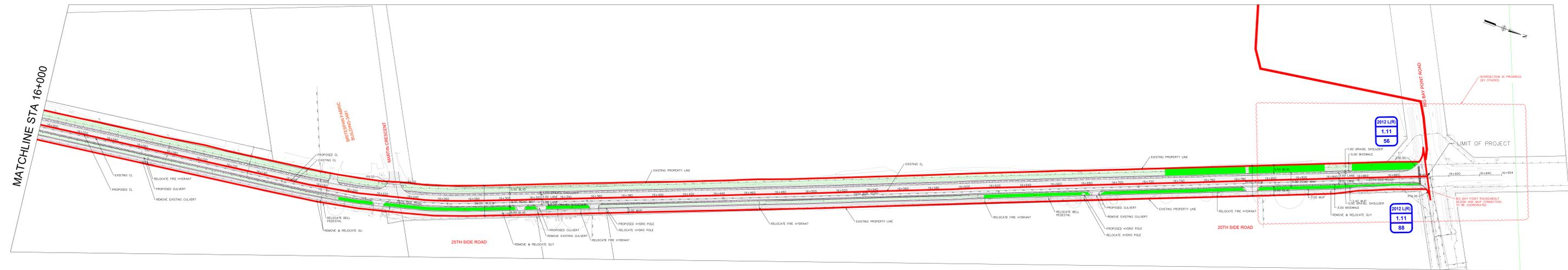
TITLE :
POST-DEVELOPMENT CONDITIONS
FIGURE 3-3

PRELIMINARY DESIGN

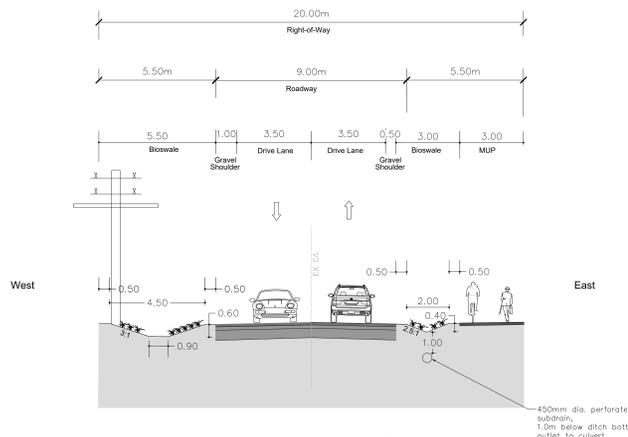
- DRIVEWAY PAVEMENT
- PROPOSED GRAVEL SHOULDER
- EXISTING 25TH SIDE ROAD CROSSING CULVERTS
- SUB-WATERSHED BOUNDARY
- SUB-CATCHMENT ID, DRAINAGE AREA (ha), IMPERVIOUSNESS (%)
- ROAD DRAINAGE AREA, ROAD DRAINAGE AREA
- EXISTING WATER
- EXISTING SANITARY
- EXISTING GAS SERVICE
- FLOW DIRECTION
- CONTOURS
- UNDER GROUND INFILTRATION CHAMBER
- PROPOSED BIOSWALE

GENERAL NOTES:
SIGNAGE AND PARKING LIMITS TO BE COMPLETED IN DETAILED DESIGN

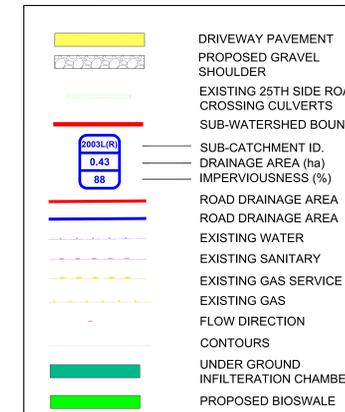
25th Side Road Plan view-7
From South of Martin Crescent to Big Bay Point Road
Context 2



TYPICAL CROSS-SECTIONS



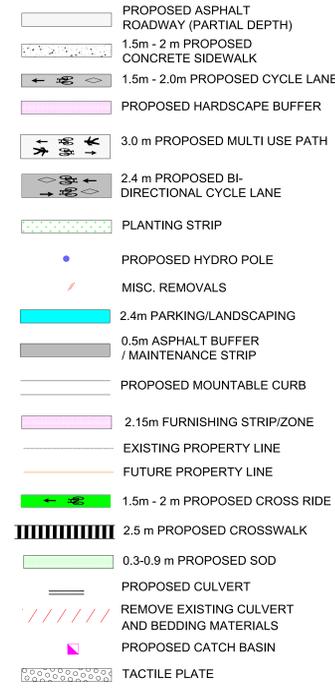
Context 2
(Green Space)
From Maple View Drive to Bay Point Road



TOWN OF INNISFIL
25TH SIDE ROAD

MAY, 2022

LEGEND

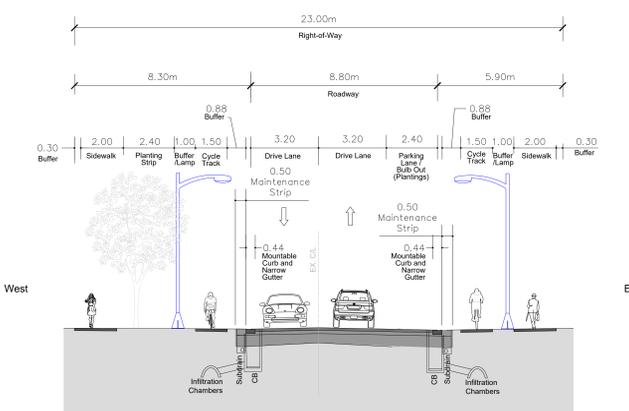
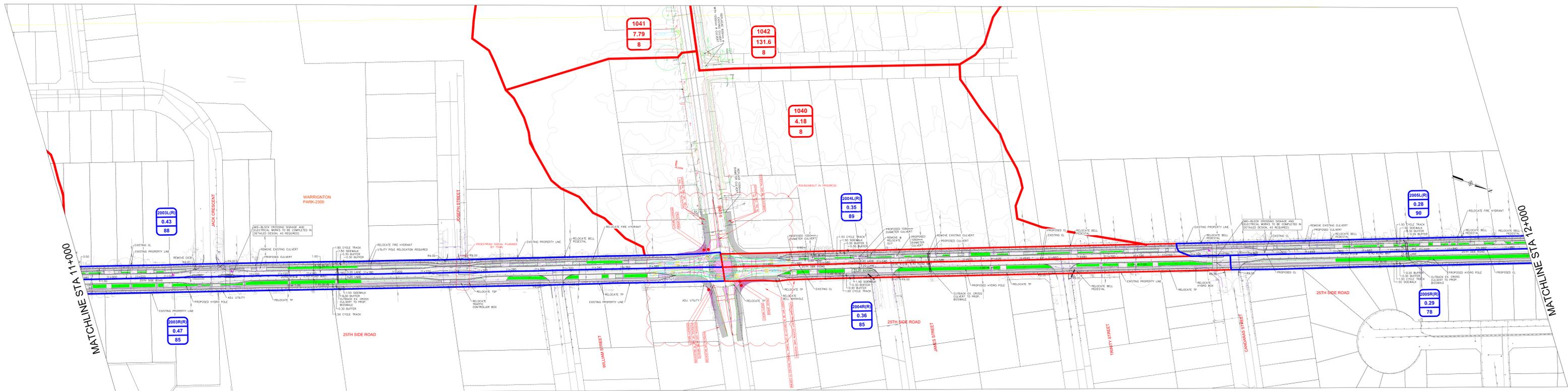


TITLE :
POST-DEVELOPMENT CONDITIONS
FIGURE 3-4

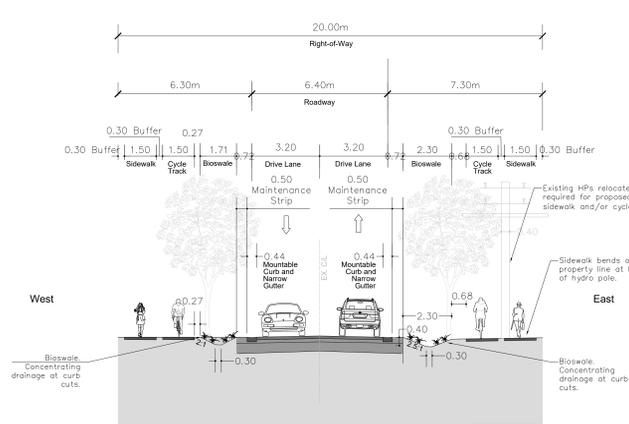
GENERAL NOTES:
SIGNAGE AND PARKING LIMITS TO BE COMPLETED
IN DETAILED DESIGN



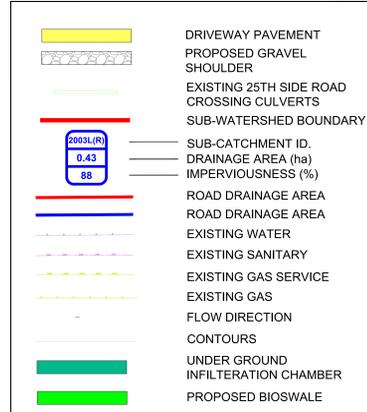
25th Side Road Plan view-2
From North of Willow Avenue to North of Candaras Street
Context 1



Context 3
(Downtown Commercial)
From Innisfil Beach Road to 80m North of Innisfil Beach



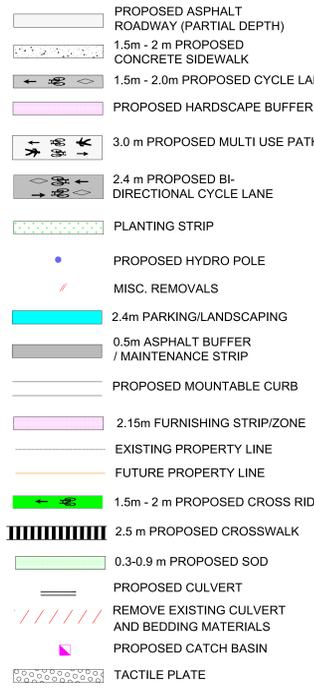
Context 1
(Low Density Residential)
From 80m North of Innisfil Beach to 70m North of Cook Street



TOWN OF INNISFIL
25TH SIDE ROAD

MAY, 2022

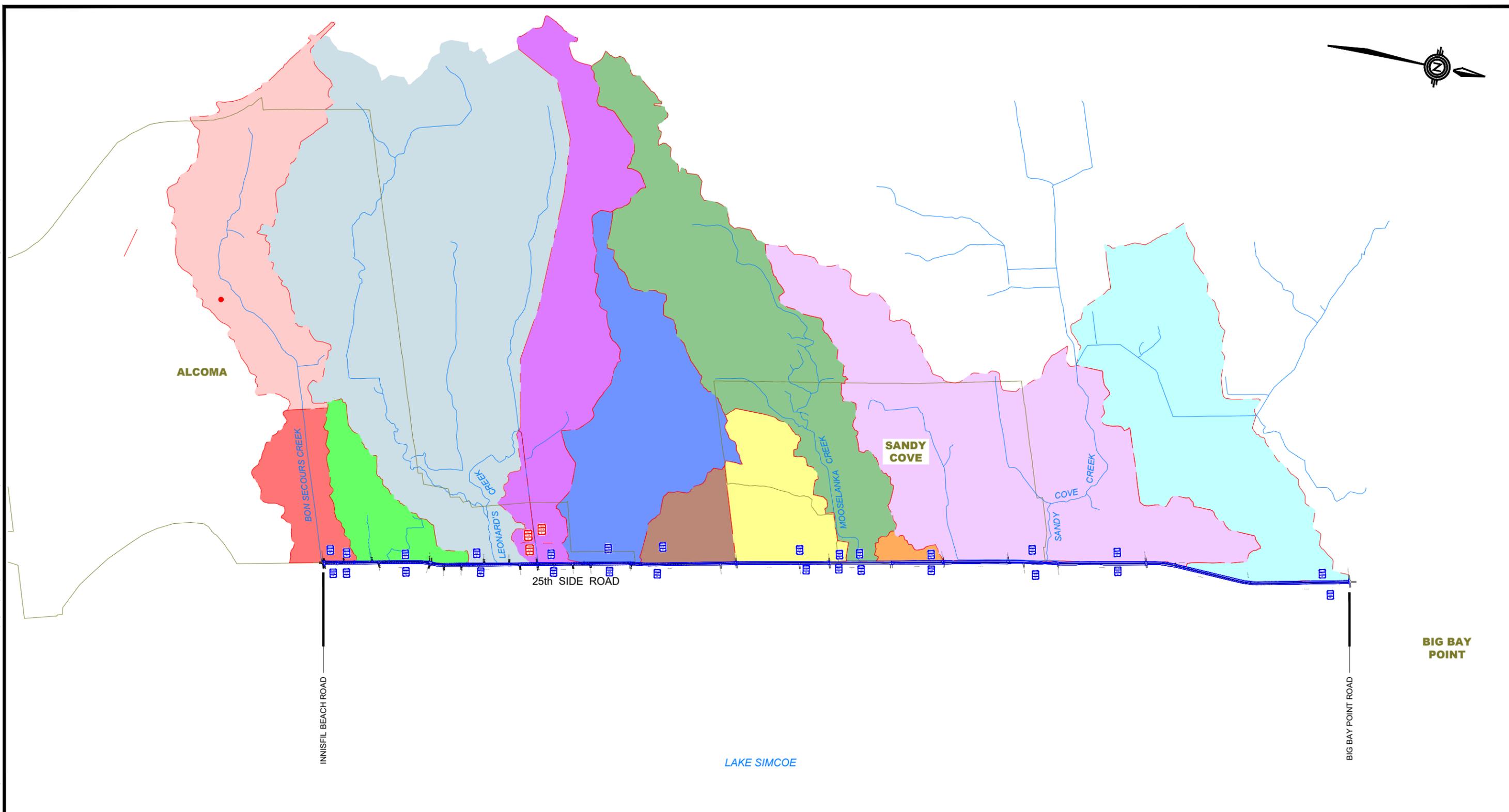
LEGEND



TITLE :
POST-DEVELOPMENT CONDITIONS
FIGURE 3-5

GENERAL NOTES:
SIGNAGE AND PARKING LIMITS TO BE COMPLETED
IN DETAILED DESIGN

FIGURE 3.6 - 25th SR Innisfil.dwg - Ex Sub-Catch Drainage Within Study Area C:\Users\caef070146\ACCDocs\WSP Canada projects (AMER)\and Development Ontario\Project Files\211-06027-00 P-20-24 Innisfil 25 Side Road\SWM\CAD\FIGURES\ Aug 04, 2022 - 12:45pm



LEGEND

- CATCHMENT BOUNDARY
- SETTLEMENT BOUNDARY
- WATERCOURSE
- 2002 L CATCHMENT ID.
- 0.70 ROAD DRAINAGE AREA (ha)
- 71 IMP (%)
- ROAD DRAINAGE BOUDARY



CLIENT	TOWN OF INNISFIL								
TITLE	25th SIDE ROAD, INNISFIL								
<p>PROPOSED ROAD DRAINAGE AREAS</p>			<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 50%;">Checked S.v.H.</td> <td style="width: 50%;">Drawn AutoCAD/B.K.B.</td> </tr> <tr> <td>Date AUGUST 2022</td> <td>Proj. No. 211-06027-00</td> </tr> <tr> <td>Scale AS SHOWN</td> <td>Figure No. 3.6</td> </tr> </table>	Checked S.v.H.	Drawn AutoCAD/B.K.B.	Date AUGUST 2022	Proj. No. 211-06027-00	Scale AS SHOWN	Figure No. 3.6
Checked S.v.H.	Drawn AutoCAD/B.K.B.								
Date AUGUST 2022	Proj. No. 211-06027-00								
Scale AS SHOWN	Figure No. 3.6								

3.1 Bioswales and Underground Infiltration Chambers

3.1.1 Bioswales

Bioswale units are soil filter systems that temporarily store and filter stormwater runoff. These units rely on the engineered soil media placed below the channel invert to provide stormwater runoff reductions and improve water quality.

The proposed bioswale within the subject site contains:

- 1 Filter Media Layer
 - a Sand (0.05 to 2 mm dia.)
 - b Fines
 - c Organic components
- 2 Mulch Layer:

75 mm layer of shredded hard wood on the surface of the filter bed.
- 3 Storage layer:

Gravel reservoir storage (i.e., granular material should be 50 mm diameter clear stone).
- 4 Pea gravel layer:

100 mm deep layer of pea gravel (i.e., a 3 to 10 mm diameter clear stone should be placed on top of the coarse gravel storage layer as a choking layer separating it from the overlying filter media bed).
- 5 Underdrain to remove excess water
- 6 Overflow pipe

The depths of the bioswale facilities within the subject site were estimated based on the required storage volumes for stormwater management control, infiltration rate of the native soil, porosity (void space ratio) of the gravel storage layer media and the targeted time period to achieve complete drainage between storm events. The proposed bioswales will have between 1.2 m to 1.5 m depths.

Bioswales should be separated from the seasonally high water table by a minimum of 1 m to ensure groundwater does not intersect the filter bed. Based on the results of the WSP geotechnical report (Section 1.5) groundwater was not encountered and all of the boreholes remained open and dry upon completion. Capped vertical standpipes consisting of an anchored 100 mm to 150 mm diameter perforated pipe with a lockable cap will be installed to the bottom of the bioswale facility to monitor the length of time

required to fully drain between storms. The location of the monitoring pipes will be determined at the detailed design stage. Overflow pipes are proposed within the bioswales to direct the excess runoff to the existing 25th Side Road crossing culverts in the event that the runoff from a storm exceeds the capacity of the bioswales.

Typical ratios of impervious drainage areas to bioswale footprint areas were considered to be between 5:1 to 15:1.

Under post-development conditions road runoff will be discharged into the proposed bioswales. A pre-treatment area consisting of a small trench filled with pea gravel between the edge of pavement and the bioswale is recommended to help prevent clogging of the bioswale facilities by coarse sediment particles before they reach the filter bed.

Image 3-1 and **Image 3-2** show the sketch of the proposed bioswales within Contexts 1 and 2.

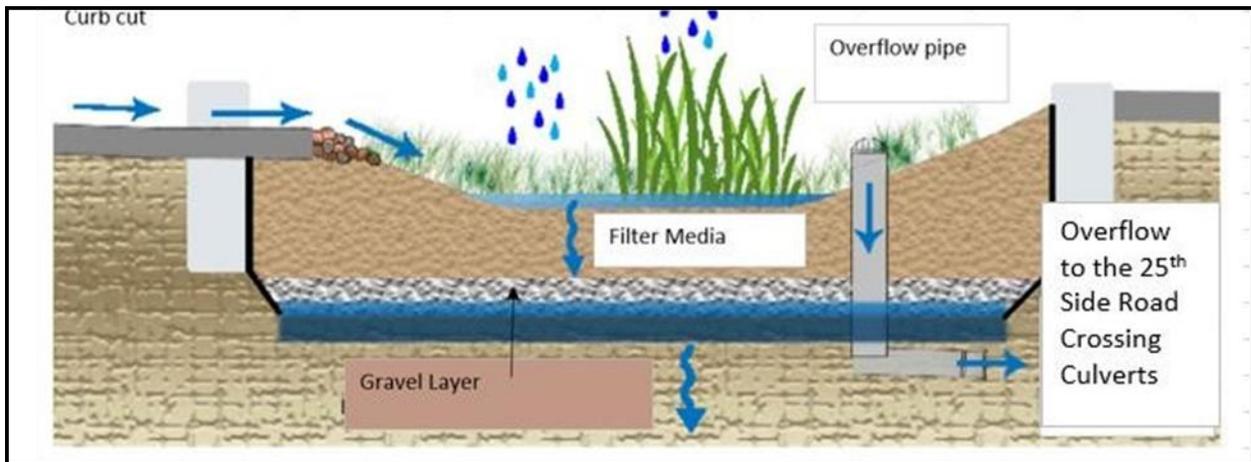


Image 3-1: Context 1 Bioswale Sketch

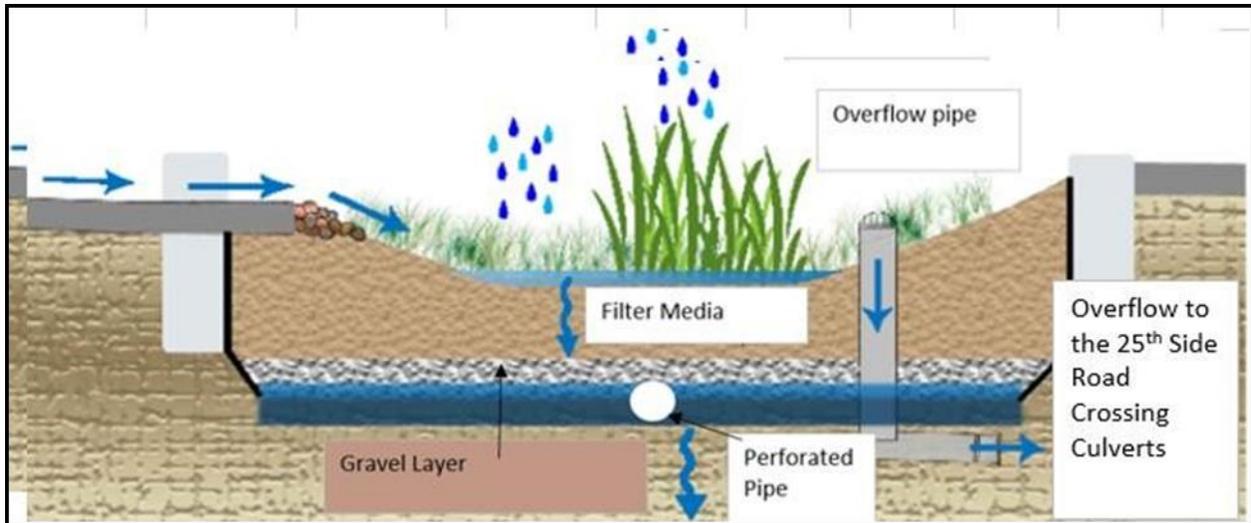


Image 3-2: Context 2 Bioswale Sketch

A detailed sketch of the proposed bioswale unit, the footprint area, size of the overflow pipes, bioswale depths and the estimated drawdown time of the bioswale within each catchment can be located within **Appendix A**.

Bioswales Maintenance

Routine visual inspection and standard maintenance are required for the proposed bioswale systems.

Since the engineering soil bed is constructed to meet the specified infiltration rate, the following policies should be implemented:

- Do not park or drive vehicles across the swale
- Do not mow the vegetation of the bottom of the swale. Mowing of grass/nuisance vegetation near the edges of the channel is acceptable.

Bioswale requires routine inspection and maintenance of the landscaping as well as periodic inspection for less frequent maintenance needs or remedial maintenance. Generally, routine maintenance will be the same as for any other landscaped area, weeding, pruning, and litter removal. Routine operation and maintenance tasks are key to public acceptance of highly visible bioretention units. Periodic inspections after major storm events will determine whether corrective action is necessary to address gradual deterioration or abnormal conditions.

For the first two years following construction the facility should be inspected semi-annually and after every major storm event (> 25 mm). Subsequently, inspections should be conducted in the spring each year and after major storm events.

While maintenance can be performed by landscaping contractors who are already providing similar landscape maintenance services on the subject site, they will need some additional training on bioretention needs.

This training should focus on elevation differences needed for ponding, mulching requirements, acceptability of ponding after a rainstorm, and fertilizer requirements. The planting plan should be kept for maintenance records and used to help maintenance staff identify which plants are weeds or invasive.

Bioretention maintenance requirements are summarized below.

- Inspect for vegetation density (at least 80% coverage), damage by foot or vehicular traffic, channelization, accumulation of debris, trash and sediment, and structural damage to pre-treatment devices (semi-annually for the first two years, and annually thereafter).
- Regular watering may be required during the first two years until vegetation is established (as needed for the first two years of operation).
- Remove trash and debris from pre-treatment devices (if installed), the bioretention area surfaces and inlets and outlets (at least twice annually, more frequently if desired for aesthetic reasons or if located in an area with high public exposure).
- Remove accumulated sediment from inlets and outlets as needed.
- Trim trees and shrubs as needed.
- Replace dead vegetation, remove invasive growth as needed.
- Repair eroded or sparsely vegetated areas as needed.
- Remove accumulated sediment on the bioretention area surface when dry and exceeds 25 mm depth (PDEP, 2006)
- If gullies are observed along the surface, re-grading and re-vegetating may be required.
- Flush out accumulated sediment with hoses or pressure washers from monitoring wells, underdrains, and overflow outlets annually.
- If visual inspection observes that standing water is present 72 hours after a rainfall event, then the practice may not be infiltrating at the desired rate, triggering further evaluation of the infiltration rate. Schedules may need to be adjusted over time as issues arise. If the monitored infiltration rate falls below the design rate, the practice should be investigated to determine the cause of the reduced rate. Quick, approximate soil texture tests can also be easily incorporated into a visual inspection to help determine whether the soil media might be clogged. If the soil media texture has become more silty or clayey, further investigation is warranted into a potential replacement of the soil media.

3.1.2 Underground Infiltration Chambers

Underground infiltration chambers are constructed below grade and therefore take up little or no space at the surface. The chambers can be installed below a broad range of land uses such as parking areas, lawns, pedestrian plazas, driveways, parks, and sports fields, but should generally be directed away from areas of heavy vehicular loading. They include a range of proprietary manufactured modular structures installed underground, that create large void spaces for the temporary storage of stormwater runoff and allow it to infiltrate into the underlying native soil. Stone reservoirs of the infiltration chamber systems should be filled with uniformly-graded, washed stone that provides 30 to 40% void space. Granular material should be a minimum 50 mm clear stone.

The proposed underground chamber systems within the subject site will have open bottoms, perforated side walls, and underlying granular stone reservoirs.

Capped vertical standpipes consisting of anchored 150 mm diameter perforated pipes with lockable caps will be installed at the bottom of the facilities to provide a means of inspecting and flushing the facilities out as part of routine maintenance. The location of the standpipes will be determined at the detailed design stage.

A non-woven, needle punched, or woven monofilament geotextile fabric will be installed around the stone reservoir of the underground infiltration chambers. The primary function of the geotextile is a separation between two dissimilar soils. When a finer-grained soil overlies a coarser grained soil or aggregate layer (e.g., stone reservoir), the geotextile prevents clogging of the void spaces from downward migration of soil particles. When a coarser grained aggregate layer (e.g., stone reservoir) overlies a finer grained native soil, the geotextile prevents slumping from downward migration of the aggregate into the underlying soil. Geotextile material specifications should conform to Ontario Provincial Standard Specification (OPSS) 1860 for Class II geotextile fabrics.

The bottom of the facility should be vertically separated by one meter from the seasonally high water table or top of bedrock elevation. Refer to Section 1.5 for the geotechnical results.

Typical ratios of impervious drainage areas to chamber footprint areas within the subject site were considered to be between 5:1 to 10:1.

Infiltration Chamber Maintenance

During construction, proper care should be taken to prevent sediment from entering the subsurface system, which may include installation of silt fencing and the bypassing of surface runoff around the system until the site is fully stabilized. At a minimum, all site

erosion and sediment protection BMPs should be installed and maintained. Careful removal and disposal of ESC measures once the site is stabilized will prevent occlusion and prevent collected debris from washing into the chamber system. Flush the system thoroughly upon completion.

Chamber system (with infiltration facilities) require regular inspection to ensure they continue to function as designed. Maintenance typically consists of cleaning out leaves, debris and accumulated sediment caught in pre-treatment devices, inlets and outlets annually or as needed. During the first two years of service, inspection and maintenance will be used to determine the pattern and timing of sediment and debris build up. Inspection via monitoring wells shall also be used to determine whether the facility drains within the maximum acceptable length of time (typically 72 hours). Inspection should be carried out within 24 hours following every major storm event (>25 mm single rainfall event where reports of localized flooding occur within the municipality), else semi-annually at a minimum.

- 1 Inspect all observation ports, the inflow and outflow connections, and any pre-treatment devices.
- 2 Measure the sediment accumulation within the system
- 3 Record any system backup / backflow, incomplete discharging of system or permanent water remaining in system.

If the time required to fully drain exceeds 72 hours, cleaning may be required to dislodge accumulated debris or sediment blockages.

- 4 Clean out any pre-treatment devices (OGS units, sediment traps / shields, filter system etc.). Follow manufacturer recommendations.
- 5 Remove sediments from all inflow and outflow connections using a vacuum pump, flushing the system with water to further dislodge debris from the chambers.

If after cleaning, the time to fully drain exceeds 72 hours cleanout of the underdrain pipe can be attempted. If slow drainage persists, the system may need removal and replacement of granular material and/or geotextile fabric. The expected lifespan of infiltration practices is not well understood; however, it can be expected that it will vary depending on pre-treatment practice maintenance frequency, and the sediment texture and load coming from the catchment.

Bioswale and Infiltration Chamber Maintenance Costs

Based on the Assessment of Life Cycle Costs for Low Impact Development Stormwater Management Practices report prepared by Toronto and Region Conservation dated April 2013, annual maintenance cost for a bioswale with a footprint area of 130 m² and infiltration chamber with footprint area of 104.7 m² would be \$952 and \$1,212

respectively. Thus the total maintenance cost for bioswale and infiltration chamber system would be \$58,347.9 and \$ 23,655 respectively. Please refer to **Appendix A** for more details.

Context 1: The design includes 1.5 m of sidewalks and 1.5 m of unidirectional cycle tracks on both sides of the road. A 3.2 m drive lane is proposed in each direction (excluding gutters) and a 0.44 m mountable curb with a narrow gutter is proposed. A 0.5 m maintenance strip to accommodate snow storage and allow overhanging of snow plow blades has also been included. It should be noted that space was dedicated on the west and east side of the road within Context 1 to implement bioswale facilities (i.e., 2 m and 2.3 m respectively), however, due to the presence of underground utilities (Bell Canada, Hydro, gas pipes, water mains and sanitary pipes) the available space to implement bioswale facilities within Context 1 will be smaller than what was shown in the Proposed Context 1 Cross-Section Sketch (**Image 3-3**).



Image 3-3: Proposed Context 1 Cross-Section

The proposed 25th Side Road will be graded on both sides of the road at detail design in such a way as to have sheet flow from the center of the road towards the curbs and from the sidewalks towards the bioswales. Curb cuts are proposed to direct roadway runoff from the center of the road to the proposed bioswales. Sheet flow from sidewalks and Cycle Tracks will be directed to the proposed bioswales as well. In addition, sheet flow from the adjacent neighbouring areas (west side of the road) will be discharged into the proposed bioswale. To provide pre-treatment at the location of curb-cuts between the edge of the pavement and the bioswale facilities small trenches filled with pea

gravel are proposed. The runoff will then be infiltrated into the underlying native soil of the bioswales. As the upstream bioswale planter fills up with runoff, it overflows out and enters to the next downstream planter. Granular check dams will be placed within the bioswale alignments to reduce water velocity. The location and number of the required check dams will be determined at detail design. The check dams will be designed to provide a maximum 0.2 m allowable ponding depths within the proposed bioswales.

Overflow pipes are proposed within the bioswales to direct the excess runoff to the existing 25th Side Road crossing culverts in the event that the runoff from a storm exceeds the capacity of the bioswales. The inlet of the proposed overflow pipes should be placed above the allowable bioswale ponding depths (i.e., 0.2 m). It should be noted that the existing 25th Side Road crossing culverts should be replaced with the recommended culverts in **Table 2-2** to have enough capacity for the 25-year storm events. The proposed crossing culverts should be placed in such a way to direct excess runoff from the proposed bioswales to the existing downstream watercourses.

The proposed bioswales within Catchments 2010 to 2012 will have perforated pipes due to the relatively low infiltration rate of the native soil. Please refer to Post-Development Conditions **Figure 3-1** to **Figure 3-5** for the location of Catchments 2010 to 2012. The proposed bioswales were sized to provide water balance, quality control and runoff reduction for the roadway drainage areas. To meet quality control targets, bioswales were sized to provide 80% TSS removal for the roadway runoff prior to being discharged into the existing 25th Side Road crossing culverts.

The existing 25th Side Road has roadside ditches on both sides of the road. Under post-development conditions, the existing roadside ditch's culverts will be shifted to the location of the proposed bioswales to convey runoff from the adjacent neighbouring areas (west side of the road) towards the 25th Side Road crossing culverts (see Post-Development Conditions Figures for the location of the proposed bioswales within Context 1). Since the ratios of impervious drainage areas to bioswale footprint areas were considered to be 15:1, in addition to roadway drainage areas, the proposed bioswales can retain runoff from small portions of the adjacent neighbouring drainage areas, therefore providing retrofitted stormwater management benefits.

Context 2: The design includes a 3.0 m multi-use pathway on the east side of the roadway and bioswales on both side of the roads. A 3.5 m drive lane is proposed in each direction. **Image 3-4** shows the Cross-Section for Context 2.

The proposed 25th Side Road will be graded at detail design in such a way to have a slope from the center of the road towards the proposed bioswale on both sides of the road. In addition, sheet flows from the 3 m multi-use pathway on the east side of the

roadway and the adjacent neighbouring drainage areas on the west side will be directed into the proposed bioswales. Small trenches filled with pea gravel are proposed between the edge of the pavement and the bioswale facilities to provide pre-treatment. The runoff will then be infiltrated into the underlying native soil of the bioswales. Since the infiltration rate of the native soil is lower than 15 mm/hr, the proposed bioswale within Context 2 will have 200 mm diameter perforated pipes. As the upstream bioswale planter fills up with runoff, it overflows and enters to the next downstream planter.

Check dams are proposed within Context 2 bioswales alignments and will provide a maximum 0.2 m ponding depth. The location and number of the required check dams will be determined at the detailed design stage.

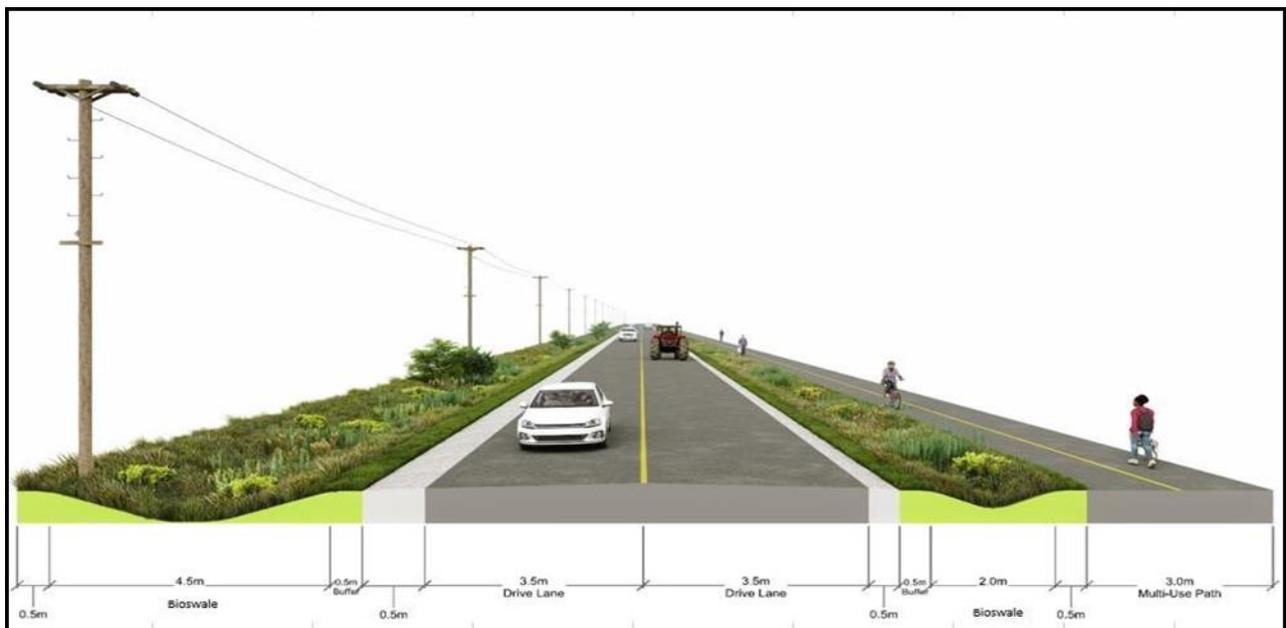


Image 3-4: Context 2 Cross-Section Sketch

Overflow pipes are proposed within the bioswales to direct the excess runoff towards the existing 25th Side Road crossing culverts in the event that the runoff from a storm exceeds the capacity of the bioswales. The inlet of the proposed overflow pipes should be placed above the allowable bioswales ponding depth of 0.2 m. As mentioned previously, the existing 25th Side Road crossing culverts should be replaced with the recommended culverts in **Table 2-2** to have enough capacity for the 25-year storm events. The proposed crossing culverts should be placed in such a way to direct excess runoff from the proposed bioswales to the existing downstream watercourses.

The proposed bioswales were sized to provide water balance, quality control (80% TSS removal) and runoff reduction for the roadway drainage areas prior to being discharged into the existing 25th Side Road crossing culverts.

As mentioned in Context 1, the existing 25th Side Road has roadside ditches on both sides of the road. Under post-development conditions, the existing roadside ditch culverts will be shifted to the location of the proposed bioswales to convey runoff from the adjacent neighbouring areas (west side of the road) towards the 25th Side Road crossing culverts (see Post-Development Conditions Figures for the location of the proposed bioswales within Context 2). Since the ratios of impervious drainage areas to bioswale footprint areas were considered to be 15:1, in addition to roadway drainage areas, the proposed bioswales can retain runoff from small portions of the adjacent neighbouring drainage areas therefore providing stormwater management retrofits.

Context 3: The design includes 2 m of sidewalks and 1.5 m of unidirectional cycle tracks, 0.6 m buffers, and 0.3 m detectable buffer strips on both sides of the road. A 3.2 m drive lane is proposed in each direction. In addition, a 2.4 m parking lane is proposed within the east side of the road. **Image 3-5** shows Context 3 Cross-Section.



Image 3-5: Context 3 Cross-Section Sketch

Opportunities exist to implement underground chamber systems within the overall roadway cross-section.

The proposed 25th Side Road will be graded at detail design to ensure slope from the center of the road and from sidewalks on both sides drain towards the proposed catchbasins. In addition, sheet flow from the adjacent neighbouring areas (west side) will be directed into the proposed catchbasins. The proposed catchbasins will have outlets directed towards the proposed storm pipes and underground chamber systems.

The proposed catchbasin outlets will be placed at different elevations. The first bottom outlet will convey runoff towards the underground chamber systems while the second outlet will convey runoff towards the proposed underground storm pipes. The underground chamber systems flow will then be infiltrated into the underlying native soil at the bottom of the systems.

The proposed underground chamber systems within the subject site were sized to provide water balance, quality control (80% TSS removal) and runoff reduction for the roadway runoff. The underground chamber systems will have perforated pipes and underdrain pipes. In the event that runoff exceeds the capacity of the systems, perforated pipes and underdrain pipes will convey the excess runoff towards proposed storm pipes.

As noted previously the existing 25th Side Road crossing culverts should be replaced with the recommended culverts in **Table 2-2** to have enough capacity for the 25-year storm events. The proposed crossing culverts should be placed in such a way to direct excess runoff from the proposed underground chamber systems and underground storm pipes to the existing downstream Creeks.

Please refer to Post-Development Conditions Figures for the location of the proposed underground chamber systems within Context 3.

Proposed Roundabout

The Town requested WSP to complete a preliminary roundabout design at the 9th Line and 25th Side Road intersection. Supplementary survey and geotechnical works were completed as part of the preliminary roundabout design (Section 1.5.2).

Under existing conditions, runoff generated from 9th Line Road is conveyed by roadside ditches on both sides of the 9th Line Road towards the 25th Side Road existing crossing culvert (see **Figure 2.5** for the existing 800 mm diameter crossing culvert location). In addition, runoff generated from the existing Catchment 104 is discharged into the existing roadside ditches on both sides of 9th Line Road prior to being discharged into the existing 800 mm diameter crossing culvert on the west side of 25th Side Road. Under post-development conditions, no major changes are made for the existing 800 mm diameter crossing culvert tributary drainage areas (Catchment 104 existing and Catchments, Catchments 2004 L & R). As such, the provided culvert capacity analysis in **Section 2.1.1** would be applicable under the proposed development of the roundabout at the intersection of 9th Line Road. To provide culvert capacity analysis for the existing 9th Line Road crossing culverts, Catchment 104 was delineated into three catchments (Catchment 1040, 1041, and 1042). Based on the topography survey plan for the study area, at the intersection of 9th Line and Wallace Avenue, runoff from the

east side of 9th Line Road is conveyed to the west side of 9th Line Road via an existing 500 mm diameter crossing culvert prior to being discharged into an existing 900 mm diameter culvert within the west side of the 9th Line Road ditch (See Figure 3-5). To estimate runoff rates towards the existing 500 mm and 900 mm diameters crossing culverts, a Visual OTTHYMO (VO6.1) model was simulated for the 10-year 4-hour Chicago and 12-hour SCS Type II design storms for Catchments 1041 and 1042. The modelling results are provided in **Table 3-2**.

Based on the modelling results, the existing 500 mm diameter crossing culvert has sufficient capacity to convey the 10-year design flow. However, it is recommended to replace the existing 900 mm diameter roadside ditch culvert on the west side of 9th Line Road with a 1050 mm diameter pipe. Please refer to **Figure 3-5** for the location of Catchments 1040, 1041 and 1042 and the proposed 1050 mm diameter culvert pipe.

Table 3-2: Full Capacity of the Existing Culverts (9th Line Road)

Catchment Name	Watercourse Name	Ex. Culvert		25- year 12 hr SCS II (m ³ /s)	100- year 12 hr SCS II (m ³ /s)	25- year 4- hr Chicago (m ³ /s)	100- year 4- hr Chicago (m ³ /s)	Culvert Capacity (m ³ /s)	Required Culvert size to pass 25-year flow (m)
		Span /d (m)	Height /d (m)						
104		0.8		2.47	3.63	1.83	2.68	0.66	2.4 x 1.50
	Leonard's Creek			10- year 12 hr SCS II (m ³ /s)		10- year 4 hr Chicago Storm (m ³ /s)		Culvert Capacity (m ³ /s)	Required Culvert size to pass 10-year flow (m)
1041		0.5		0.096		0.07		0.53	0.5
1042		0.9		1.63		1.19		1.39	1.05

Under post-development conditions the proposed LID facilities within Catchments 2004 and 2004 R will provide quality, quantity and water balance control for Catchment 2004 drainage area. As mentioned in **Section 2.1.1 (Table 2-2)** the existing 800 mm diameter 25th Side Road crossing culvert should be replaced with a 2.4 m x 1.5 m box culvert.

3.1.3 Post-Development Conditions Hydrologic Model

A Visual OTTHYMO (VO6.1) model was simulated to estimate the post-development peak flow rates for Catchments 2001 to 2012. Peak flow rates for the 4-hour Chicago and 12-hour SCS Type II design storms were determined for the 2-year to 100-year return periods. The post-development hydrologic model uses the same catchment delineation as the existing conditions model. The catchment parameters have been altered to reflect proposed conditions of the site. The details of the Visual OTTHYMO modelling results are provided in **Appendix C**.

3.2 Volume Control

As mentioned in **Section 1.3.1.1**, LSRCA criteria require linear redevelopment projects on sites without restrictions that create 0.5 or greater hectares of new and/or fully reconstructed impervious surfaces, shall capture and retain / treat on site, the larger of the following:

- 1 The runoff from a 12.5 mm event from the fully reconstructed impervious surface and newly constructed impervious area.
- 2 The runoff from a 25 mm event from the net increase in impervious area on the site.

Table 3-3: Volume to be Retained on Site

Criteria	Proposed Road Drainage Area (ha)	Imp (%)	Fully Reconstructed Impervious Areas (ha)	Net Increase in Impervious Area (ha)	Required Volume (m ³)
12.5 mm from the fully reconstructed area	15.29	82	12.54	-	1,567.5
25 mm from net increase in impervious area	15.29	63	-	2.17	542.5

From the above table, the volume control criteria is based on 12.5 mm of runoff from the fully reconstructed area. The 12.5 mm rainfall depth retention on site will be achieved by implementing the proposed bioswales and underground infiltration chambers within the roadway drainage areas. **Table 3-4** shows the LID units required storage volumes and total provided storage volumes to meet LSRCA Volume Control criterion for the study area. It should be noted that the depth of the proposed catchbasins is not included within Context 3 LID units' depth.

Table 3-4: Storage Requirements for Volume Control

Catchment Name	LID Footprint Area (ha)	Total Depth of LID Units (m)	Required Storage Volume to Retain 12.5 mm Rainfall Depth (m ³)	Total LID Units Provided Storage Volume (m ³)
Catchment 2001-1 L Context 3	0.006	*1.2	11.2	27.7
Catchment 2001-1 R Context 3	0.011	*0.7	9.8	31
Catchment 2001-2 L Context 1	0.01	1.9	13.1	39
Catchment 2001-2 R Context 1	0.01	1.2	15.6	30.9
Catchment 2002 L Context 1	0.04	1.2	82.0	254.6

Catchment Name	LID Footprint Area (ha)	Total Depth of LID Units (m)	Required Storage Volume to Retain 12.5 mm Rainfall Depth (m³)	Total LID Units Provided Storage Volume (m³)
Catchment 2002 R Context 1	0.04	1.2	94.5	268.9
Catchment 2003 L Context 1	0.02	2.3	48.2	166
Catchment 2003 R Context 1	0.04	1.2	50.9	228
Catchment 2004 L Context 1	0.015	1.6	38.9	111
Catchment 2004 R Context 1	0.05	1.2	38.4	269
Catchment 2005 L Context 1	0.008	1.5	31.4	64
Catchment 2005 R Context 1	0.03	1.2	28.3	185
Catchment 2006 L Context 1	0.05	1.2	71.2	291
Catchment 2006 R Context 1	0.03	1.9	72.9	240
Catchment 2007 L Context 1	0.06	1.2	61.7	358
Catchment 2007 R Context 1	0.64	1.2	66.8	400
Catchment 2008-1 L Context 1	0.01	1.6	7.1	45
Catchment 2008-2 L Context 3	0.04	*1.0	33.3	173
Catchment 20081 R Context 1	0.006	1.3	7.6	36
Catchment 20082 R Context 3	0.04	*1.0	28.3	172
Catchment 2009 L Context 3	0.032	*0.50	38.4	63
Catchment 2009 R Context 3	0.037	*0.50	37.4	67
Catchment 20101 L Context 3	0.022	*0.50	25.6	43

Catchment Name	LID Footprint Area (ha)	Total Depth of LID Units (m)	Required Storage Volume to Retain 12.5 mm Rainfall Depth (m ³)	Total LID Units Provided Storage Volume (m ³)
Catchment 20102 L Context 1	0.033	1.2	43.8	191
Catchment 20101 R Context 3	0.011	1.3	25.6	140
Catchment 20102 R Context 1	0.024	1.5	76.1	152
Catchment 2011 L Contexts 3&,2	0.053	1.2	93.3	258
Catchment 2011 R Contexts 2 & 3	0.056	1.2	95.2	268
Catchment 2012 L Context 2	0.0693	1.2	77.5	314
Catchment 2012 R Context 2	0.077	1.3	129.1	388
Total				5,285.0

3.3 Quality Control

Section 1.3.1.1 identified the required suspended solids removal treatment as Enhanced Protection Level (Level 1) which corresponds to a long-term average removal of 80% of suspended solids.

Table 3-5 shows the required storage volumes within the proposed LID units to remove the 80% of suspended solids from the roadway drainage areas.

The MOE (now MECP) SWM Manual (Table 3.2) was used to estimate water quality storage volume requirements for roadway drainage areas.

Table 3-5: Required Storage Volume to meet the 80% TTS Removal

Catchment Name	Area (ha)	Imp (%)	Required Infiltration Volume		TSS Removal (%)
			(m ³)	(m ³ /ha)	
Catchment 2001-1 L Context 3	0.10	90	5.37	53.67	80.00
Catchment 2001-1 R Context 3	0.10	79	3.78	38.15	80.00
Catchment 2001-2 L Context 1	0.11	95	7.67	69.73	80.00
Catchment 2001-2 R Context 1	0.13	96	1.25	72.31	80.00
Catchment 2002 L Context 1	0.70	94	46.39	66.27	80.00
Catchment 2002 R Context 1	0.80	95	54.83	68.54	80.00
Catchment 2003 L Context 1	0.43	90	23.08	53.67	80.00
Catchment 2003 R Context 1	0.47	87	21.07	44.84	80.00
Catchment 2004 L Context 1	0.35	89	18.06	51.60	80.00
Catchment 2004 R Context 1	0.36	85	14.85	41.26	80.00
Catchment 2005 L Context 1	0.28	90	15.03	53.67	80.00
Catchment 2005 R Context 1	0.29	78	10.93	37.69	80.00
Catchment 2006 L Context 1	0.65	88	31.11	47.86	80.00
Catchment 2006 R Context 1	0.73	80	27.95	38.28	80.00
Catchment 2007 L Context 1	0.58	85	23.40	40.35	80.00
Catchment 2007 R Context 1	0.64	84	22.81	35.64	80.00
Catchment 2008-1 L Context 1	0.07	84	2.54	37.87	80.00
Catchment 2008-2 L Context 3	0.38	70	13.34	34.84	80.00

Catchment Name	Area (ha)	Imp (%)	Required Infiltration Volume		TSS Removal (%)
			(m ³)	(m ³ /ha)	
Catchment 2008-1 R Context 1	0.07	83	2.55	34.86	80.00
Catchment 2008-2 R Context 3	0.39	59	12.13	31.33	80.00
Catchment 2009 L Context 3	0.40	77	14.92	37.29	80.00
Catchment 2009 R Context 3	0.39	77	14.53	37.26	80.00
Catchment 2010-1 L Context 3	0.25	82	9.75	39.00	80.00
Catchment 2010-2 L Context 1	0.47	74	17.15	36.49	80.00
Catchment 2010-1 R Context 3	0.25	82	9.75	39.00	80.00
Catchment 2010-2 R Context 1	0.47	86	18.93	40.28	80.00
Catchment 2011 L Context 2 & 3	0.87	86	29.58	34.00	80.00
Catchment 2011R Context 2 & 3	0.90	85	35.89	39.88	80.00
Catchment 2012 L Context 2	1.11	56	33.67	30.33	80.00
Catchment 2012 R Context 2	1.11	93	47.37	42.68	80.00
Total			589.7		

As shown in **Table 3-5**, a total storage volume of 589.7 m³ is required within LID units to provide 80% TSS removal for the roadway drainage areas. **Section 3.2** identified the LID systems having a total storage volume of 5,285 m³, of which 1,568 m³ is required to meet the LSRCA Volume Control criterion. Please refer to **Table 3-4** for the required storage volume to retain the 12.5 mm of rainfall depth on site. Based on the results, the proposed LID will have sufficient storage volumes to satisfy quality control requirements.

Section 1.3.1.1 noted that LSRCA initiated the Lake Simcoe Phosphorus Offsetting Policy (LSPOP) in 2017, which requires that all new development must control 100% of the phosphorus from leaving their property. This is referred to as the Zero Export

Target. Under this policy, any remaining phosphorus load that cannot be controlled would trigger the need for an offset to achieve a net zero target. As per Section 4.4.3 of the (LSPOP), in situations where the phosphorus load cannot be met a cash-in-lieu amount payable to the LSRCA shall be used in the retrofit if existing stormwater discharges elsewhere in a subwatershed or in adjacent subwatersheds.

Therefore, WSP conducted a phosphorus loading analysis for the subject site using the MECP Lake Simcoe Phosphorus Loading Development Tool. Detailed loading calculations can be found in **Appendix A**.

Pre-Development Phosphorus Loads

Pre-development conditions were simulated by applying land use types:

- ‘Hay-Pasture’ for the pervious portions of the road drainage areas
- ‘Low Residential Development’ for the remaining impervious areas of the road

The results determined the pre-development annual phosphorus loading of 14.10 kg/year for the subject site.

Post-Development Phosphorus Loads – No LID Features

Post-development conditions were simulated by applying a land use types:

- ‘High Residential Intensity Development’ for the pervious portions of the road drainage areas
- ‘Hay-Pasture’ for the pervious portions of the road drainage areas

The post-development annual phosphorus loading results were estimated to be 16.55 kg/year.

Post-Development Phosphorus Loads – LID Features

As mentioned previously, the proposed development with mitigation measures includes two low impact development facilities:

- Bioretention, which provides an overall phosphorus removal rate of 100%
- Infiltration Trench, which provides an overall phosphorus removal rate of 80%

With the proposed BMPs, the post-development phosphorus load results for the subject site are estimated to be 2.78 kg/year.

Therefore, a cash-in-lieu fee for the subject site is estimated to be \$248,256 as the Zero Export Target is not achieved. Using the offsetting ratio of 2.5, the offsetting cost of \$35,000/kg/year and an additional 15% administration fee outlined in Section 4.4.3 of the LSPOP.

3.4 Water Balance

The site is required to demonstrate that the pre-development infiltration volume will be maintained under post-development conditions.

The Thornthwaite and Mather methodology has been used for the pre- and post-development water balance analysis. The 1981 to 2010 Climate Normals data has been used to estimate the average annual precipitation of 932.9 mm/year for the subject site. The potential evaporation rate and water surplus have been estimated to be 598.2 mm/year and 334.7 mm/year respectively. Based on the preliminary water balance analysis, pre and post-development yearly infiltration volumes of 18,936 m³ and 11,315 m³ have been estimated for the subject site, respectively. Therefore, under un-mitigated post-development conditions, a yearly reduction in infiltration volume of 7,621 m³ is expected.

As mentioned in **Section 3.1**, two Low-Impact Development measures are proposed for the subject site to address the water balance of the proposed site exceeding the pre-development water balance characteristics.

Based on the results of the research study in the Low Impact Development Stormwater Management Planning and Design Guide, a 70% runoff reduction will occur by installing the bioswale units without underdrains. In addition, a 60% runoff reduction will occur by implementing underground infiltration chamber systems (see **Appendix B** for more details). Thus, a yearly infiltration rate of 49,808 m³ was estimated for the subject site which will mitigate the previously identified increase in runoff and address the water balance requirements.

Table 3-6: Comparison of Water Balance Under Pre, Post and Post-Development with Mitigation Measures

Hydrologic Cycle Components	Pre-Development Condition Volumes (m ³)	Post-Development Condition Volumes (m ³)	Post-Development Conditions Volumes with Mitigation Measures (m ³)
Infiltration	18,936	11,315	49,808
Evapotranspiration	28,451	24,176	48,625
Runoff	95,253	107,149	44,198
Precipitation	142,640	142,640	142,640

3.5 Quality Control

The post-development peak flow rates should be controlled to the corresponding pre-development peak flow rates for 2 to 100-year storm events. A Visual OTTHYMO Model (VO6.1) was used to estimate the minimum required storage volumes to meet quantity control targets for the roadway drainage areas.

The LID units within the study area are proposed to provide the required storage volumes for the LSRCA Volume Control criterion, quantity control and quality control requirements. **Table 3-4 (Section 3.2)** shows the total provided storage volume within the LID units.

The required storage volumes to meet the LSRCA Volume Control criterion and quality control targets have been provided in **Table 3-3 (Section 3.2)** and **Table 3-4 (Section 3.3)** respectively. As shown in **Table 3-3** and **Table 3-4**, the LID units will have available storage volume for quantity control requirements as well. As such, the Visual OTTHYMO modeling results were used to confirm the available storage volumes within the proposed LID units will satisfy the minimum quantity control storage volume requirements for the roadway drainage areas. Control flow devices will be sized at the detailed design stage. The results are summarized in **Table 3-7** and **Table 3-8**. Please refer to **Appendix C** for more details.

Table 3-7: Quantity Control Required Storage Volumes (12-hr SCS II)

Catchment Name	Pre-development Flow 12-hr SCS II (m ³ /s)	Post-development Flow 12-hr SCS II (m ³ /s)	Required Storage Volume (m ³)	Available Storage Volume for Quantity Control (m ³)
Catchment 2001 L Contexts 1 & 3	0.083	0.085	25	29.2
Catchment 2001 R Contexts 1&3	0.091	0.092	27	31.5
Catchment 2002 L Context 1	0.282	0.285	40	126.1
Catchment 2002 R Context 1	0.324	0.321	N/A	119.5
Catchment 2003 L Context 1	0.142	0.175	94	94.5
Catchment 2003 R Context 1	0.154	0.191	102	156.5

Catchment Name	Pre-development Flow 12-hr SCS II (m³/s)	Post-development Flow 12-hr SCS II (m³/s)	Required Storage Volume (m³)	Available Storage Volume for Quantity Control (m³)
Catchment 2004 L Context 1	0.135	0.142	51	54.1
Catchment 2004 R Context 1	0.148	0.146	N/A	215.7
Catchment 2005 L Context 1	0.138	0.114	N/A	17.5
Catchment 2005 R Context 1	0.133	0.115	N/A	146.2
Catchment 2006 L Context 1	0.241	0.263	99	188.6
Catchment 2006 R Context 1	0.243	0.289	135	139.2
Catchment 2007 L Context 1	0.263	0.234	N/A	272.9
Catchment 2007 R Context 1	0.285	0.256	N/A	310.0
Catchment 2008 L Contexts 1&3	0.075	0.175	162	162.3
Catchment 2008 R Contexts 1&3	0.085	0.171	141	157.6
Catchment 2009 L Context 1	0.195	0.157	N/A	9.6
Catchment 2009 R Context 1	0.25	0.25	N/A	15.4
Catchment 2010 L Contexts 1&3	0.326	0.284	N/A	138.6
Catchment 2010 R Contexts 1&3	0.34	0.38	159	161.8
Catchment 2011 L Contexts 2&3	0.385	0.331	N/A	134.8
Catchment 2011 R Contexts 2&3	0.382	0.356	N/A	136.7
Catchment 2012 L Context 2	0.461	0.398	N/A	202.4
Catchment 2012 R Context 2	0.466	0.44	N/A	212.0

Catchment Name	Pre-development Flow 12-hr SCS II (m³/s)	Post-development Flow 12- hr SCS II (m³/s)	Required Storage Volume (m³)	Available Storage Volume for Quantity Control (m³)
Catchment 2010-1 R Context 3	0.34	0.38	159	161.8
Catchment 2010-2 R Context 1	0.385	0.331	N/A	134.8
Catchment 2011 L Context 2 & 3	0.382	0.356	N/A	136.7
Catchment 2011R Context 2 & 3	0.461	0.398	N/A	202.4
Catchment 2012 L Context 2	0.466	0.44	N/A	212.0
Catchment 2012 R Context 2	0.34	0.38	159	161.8

Table 3-8: Quantity Control Required Storage Volumes (4-hr Chicago Storm)

Catchment Name	Pre-development Flow 12-hr SCS II (m³/s)	Post-development Flow 12- hr SCS II (m³/s)	Required Storage Volume (m³)	Available Storage Volume for Quantity Control (m³)
Catchment 2001 L Contexts 1 & 3	0.098	0.102	25	29.2
Catchment 2001 R Contexts 1&3	0.107	0.109	24	31.5
Catchment 2002 L Context 1	0.314	0.339	83	126.1
Catchment 2002 R Context 1	0.366	0.381	84	119.5
Catchment 2003 L Context 1	0.159	0.209	80	94.5
Catchment 2003 R Context 1	0.182	0.226	79	156.5
Catchment 2004 L Context 1	0.149	0.17	51	54.1
Catchment 2004 R Context 1	0.167	0.172	38	215.7
Catchment 2005 L Context 1	0.155	0.136	N/A	17.5

Catchment Name	Pre-development Flow 12-hr SCS II (m³/s)	Post-development Flow 12- hr SCS II (m³/s)	Required Storage Volume (m³)	Available Storage Volume for Quantity Control (m³)
Catchment 2005 R Context 1	0.142	0.135	91	146.2
Catchment 2006 L Context 1	0.269	0.31	91	188.6
Catchment 2006 R Context 1	0.26	0.332	114	139.2
Catchment 2007 L Context 1	0.28	0.277	50	272.9
Catchment 2007 R Context 1	0.319	0.304	N/A	310.0
Catchment 2008 L Contexts 1 & 3	0.078	0.197	122	162.3
Catchment 2008 R Contexts 1 & 3	0.101	0.184	90	157.6
Catchment 2009 L Context 1	0.22	0.179	N/A	9.6
Catchment 2009 R Context 1	0.296	0.296	N/A	15.4
Catchment 2010 L Contexts 1 & 3	0.34	0.325	N/A	138.6
Catchment 2010 R Contexts 1 & 3	0.38	0.449	140	161.8
Catchment 2011 L Contexts 2 & 3	0.407	0.364	N/A	134.8
Catchment 2011 R Contexts 2 & 3	0.435	0.109	N/A	136.7
Catchment 2012 L Context 2	0.508	0.417	N/A	202.4
Catchment 2012 R Context 2	0.516	0.506	N/A	212.0

4 CONCLUSION

A stormwater management plan has been prepared to support the proposed 25th Side Road reconstruction project in The Town of Innisfil. The key points are summarized below.

Bioswale and underground infiltration chamber systems are proposed to provide LSRCA Volume Control requirements, water quantity, water quality and water balance storage volume requirements for the road drainage areas. The depths of the LID units within the subject site were estimated based on the required storage volumes for stormwater management control, infiltration rate of the native soil, porosity (void space ratio) of the gravel storage layer media and the targeted time period to achieve complete drainage between storm events. The geotechnical report provided by WSP dated April 2022 indicated groundwater was not encountered during the investigation, and all of the boreholes remained open and dry upon completion. Thus, the proposed LID systems will have a 1 m separation from seasonal groundwater levels.

– Culvert Capacity Analysis

The capacity of the existing culverts along 25th Side Road are significantly lower than the peak flows from the 25-year and 100-year design storm events. The existing culverts should be replaced under proposed conditions.

– Volume Control

Runoff from a 12.5 mm event from the fully reconstructed road impervious surface will be retained on site by utilizing LID units.

– Quality Control

The site is required to target a long-term removal of 80% total suspended solids (TSS) on an annual loading basis. LID units were sized to satisfy the required 80% TSS removal based on the MOE (now MECP) SWM Manual (Table 3.2) water quality storage volume requirements.

The pre- and post-development annual phosphorus loading of 14.10 kg/year and 16.55 kg/year were estimated for the subject site. Under post-development conditions with mitigation measures, an annual phosphorus loading of 2.78 kg/year was estimated. Thus, a required cash-in-lieu fee for the subject site is estimated to be \$248,256.

– **Water Balance**

The site is required to demonstrate that the pre-development infiltration volume will be maintained under post-development conditions. The results of the preliminary water balance analysis reveal pre- and post-development yearly infiltration rates of 18,936 m³ and 11,315 m³ which is equal to a yearly reduction of 7,621 m³ under post-development conditions. Under post-development conditions with mitigation measures, an infiltration rate of 49,808 m³ is estimated. Therefore, the water balance control target will be met by applying the LID units within the subject site.

– **Quantity Control**

The post-development peak flow rates should be controlled to the corresponding pre-development peak flow rates for 2 to 100-year storm events. A Visual OTTHYMO Model (VO6.1) was used to estimate the minimum required storage volumes within the LID units to meet quantity control targets for the roadway drainage areas. Based on the modeling results the provided storage volumes within the LID units will satisfy the quantity control targets for the study area.

– **Utility Constrains**

One of the typical constraints of implementing a Green Infrastructure Project is the presence of underground utilities and particularly, the provision of adequate access to utility lines for maintenance. Although adequate space between the edge of the proposed LID units and the alignment of the existing utilities (i.e., Bell Canada, gas line, sanitary, water main, storm pipes, etc.) has been provided for the subject site (1 m space), a Subsurface Utility Engineering (SUE) process should be completed to find the depth and exact location of the existing utilities. There is a known utility constraint within Catchment 2007. To restrict the movement of water into the existing underlying sanitary pipes, utility protection measures should be performed.

Protection measures may include soil or engineer fill with impermeable liner or an insulating wrap.

This report has demonstrated that the proposed strategy will address stormwater management related impacts from the project and meet the intent of the Town of Innisfil and Lake Simcoe Region Conservation Authority.

APPENDIX

A

Background Information



Soils in the Innisfil Creeks subwatershed

Figure 2-18

Legend

-  Road
-  Municipal Boundary
-  Watercourse

Soil Hydro Class

-  A - high infiltration rates
-  B - moderate infiltration rates
-  C - slow infiltration rates
-  D - very slow infiltration rates

Soil Texture Class

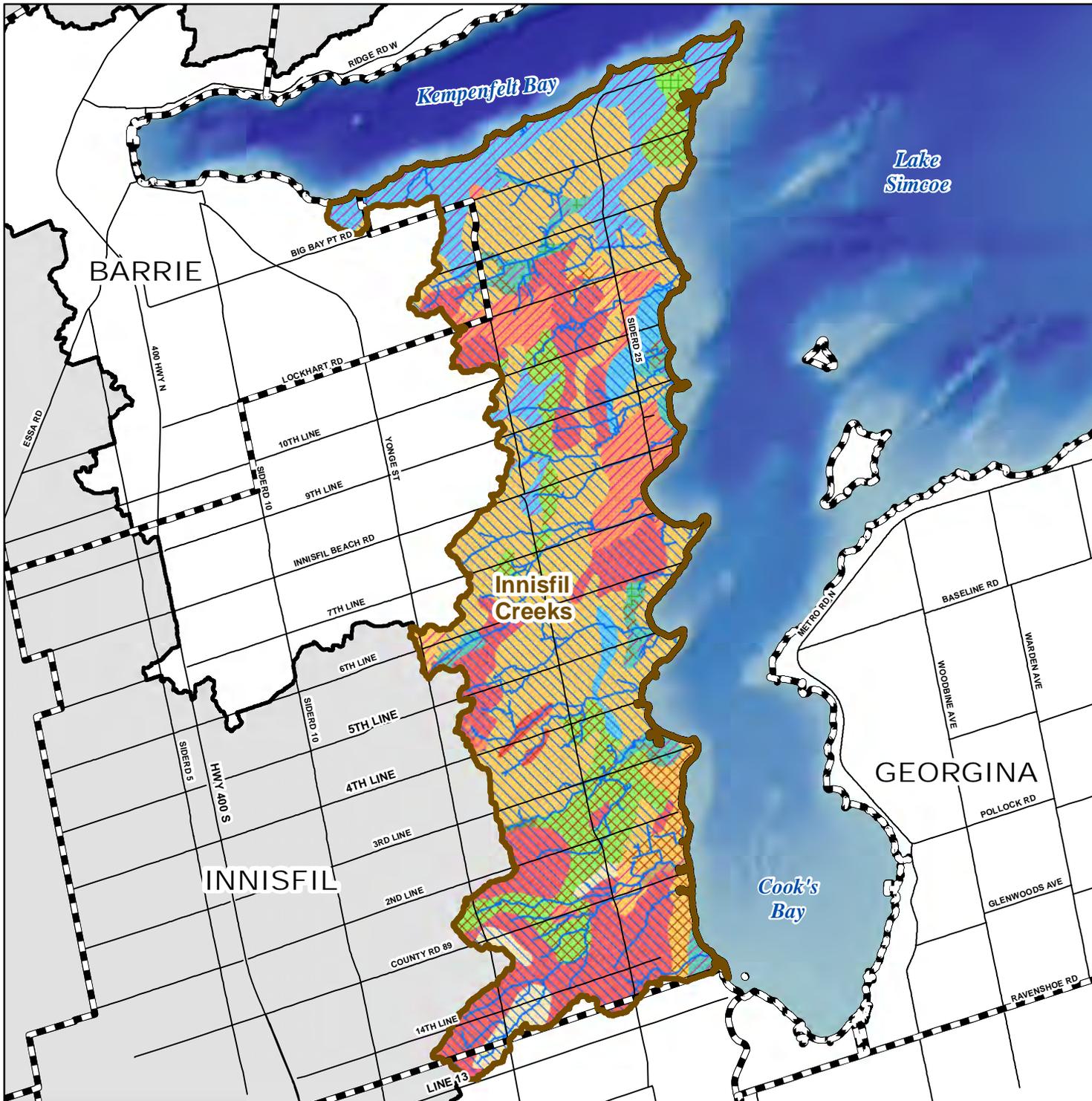
-  GRAVELLY SANDY LOAM
-  SAND
-  SANDY LOAM
-  FINE SANDY LOAM
-  LOAM
-  LOAMY SAND
-  SILTY LOAM
-  SILTY CLAY LOAM
-  ORGANIC



Lake Simcoe Region
conservation authority



This product was produced by the Lake Simcoe Region Conservation Authority and some information depicted on this map may have been compiled from various sources. While every effort has been made to accurately depict the information, data / mapping errors may exist.
This map has been produced for illustrative purposes only.
LSRCA GIS Services DRAFT created July 2010.
© LAKE SIMCOE REGION CONSERVATION AUTHORITY, 2010. All Rights Reserved
The following datasets roads, municipal boundaries and Oak Ridges Moraine are © Queens Printer for Ontario, 2010. Reproduced with Permission

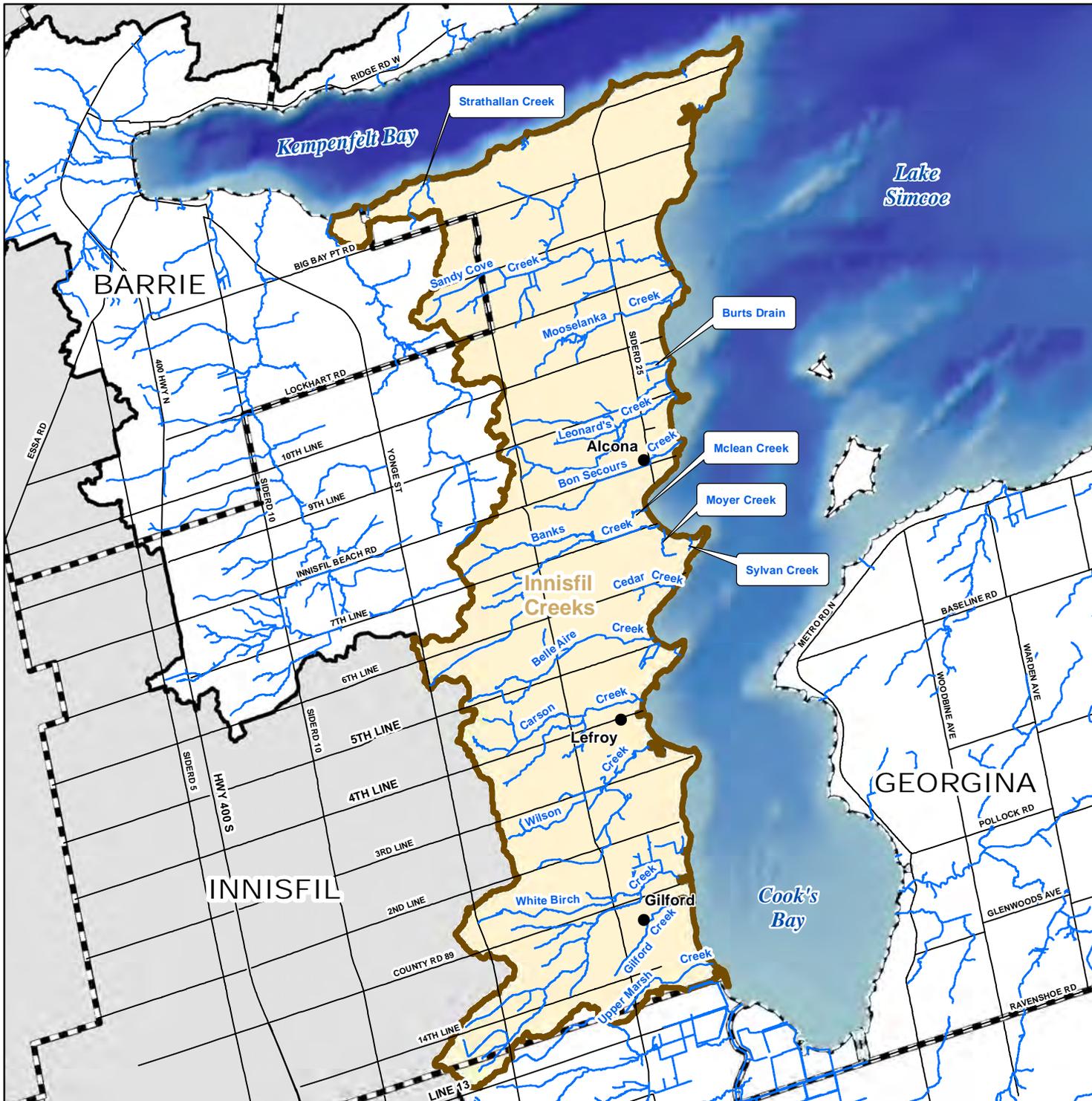


The Innisfil Creeks subwatershed

Figure 2-1

Legend

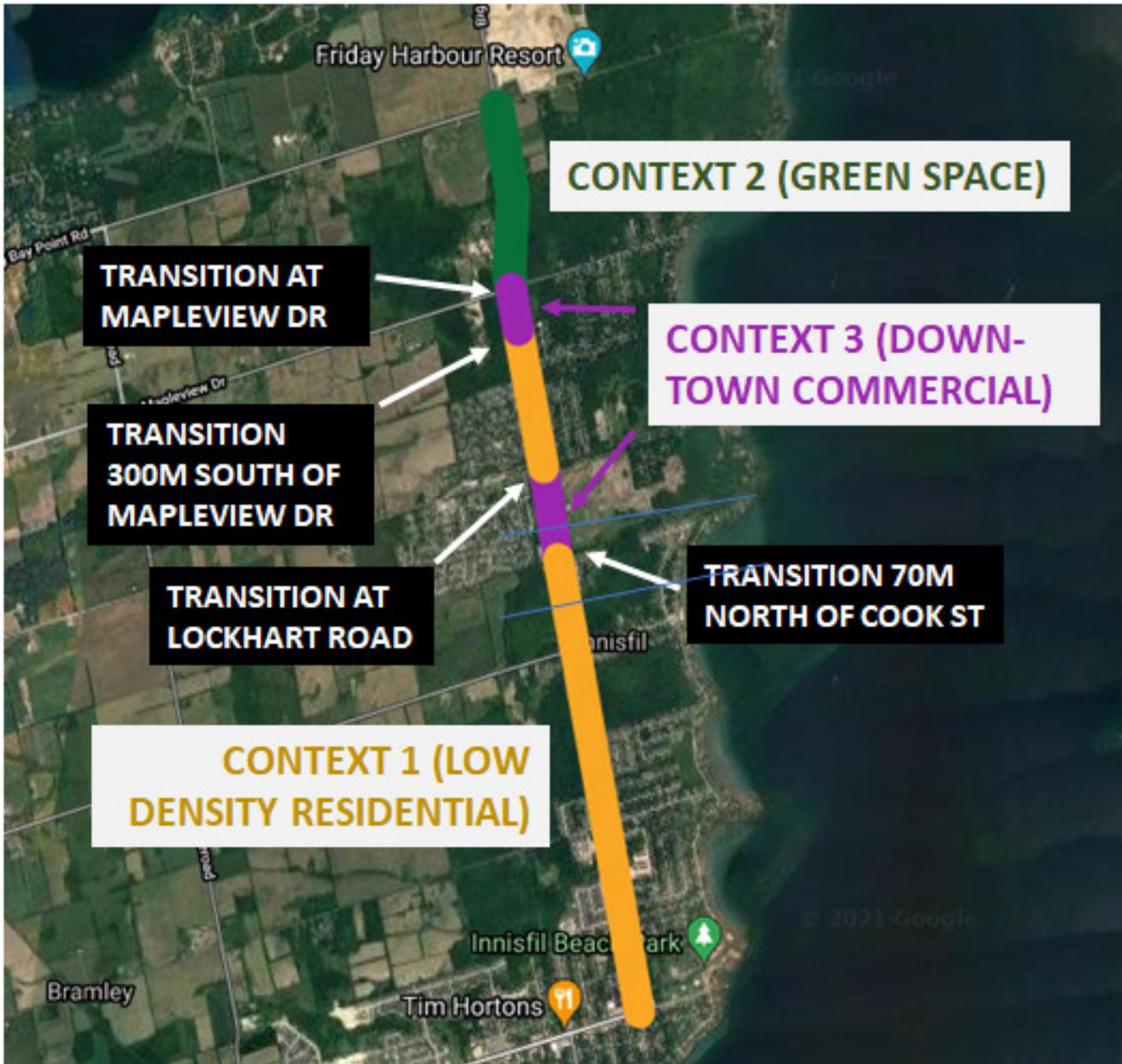
-  Road
-  Municipal Boundary
-  Watercourse
-  Innisfil Creeks Subwatershed

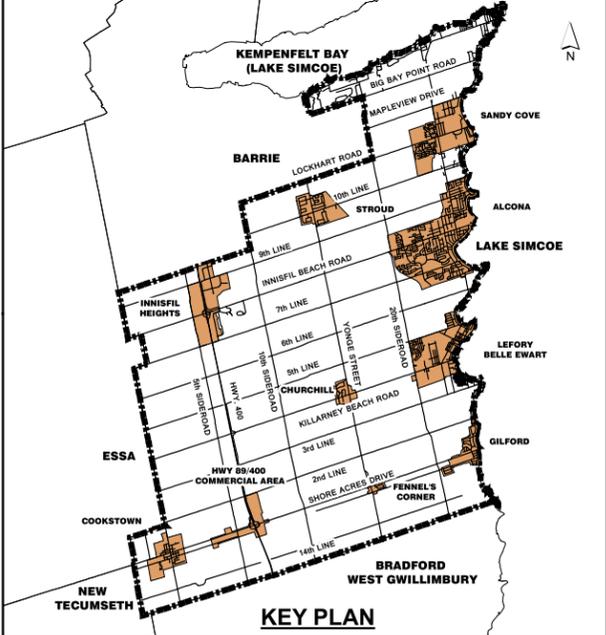
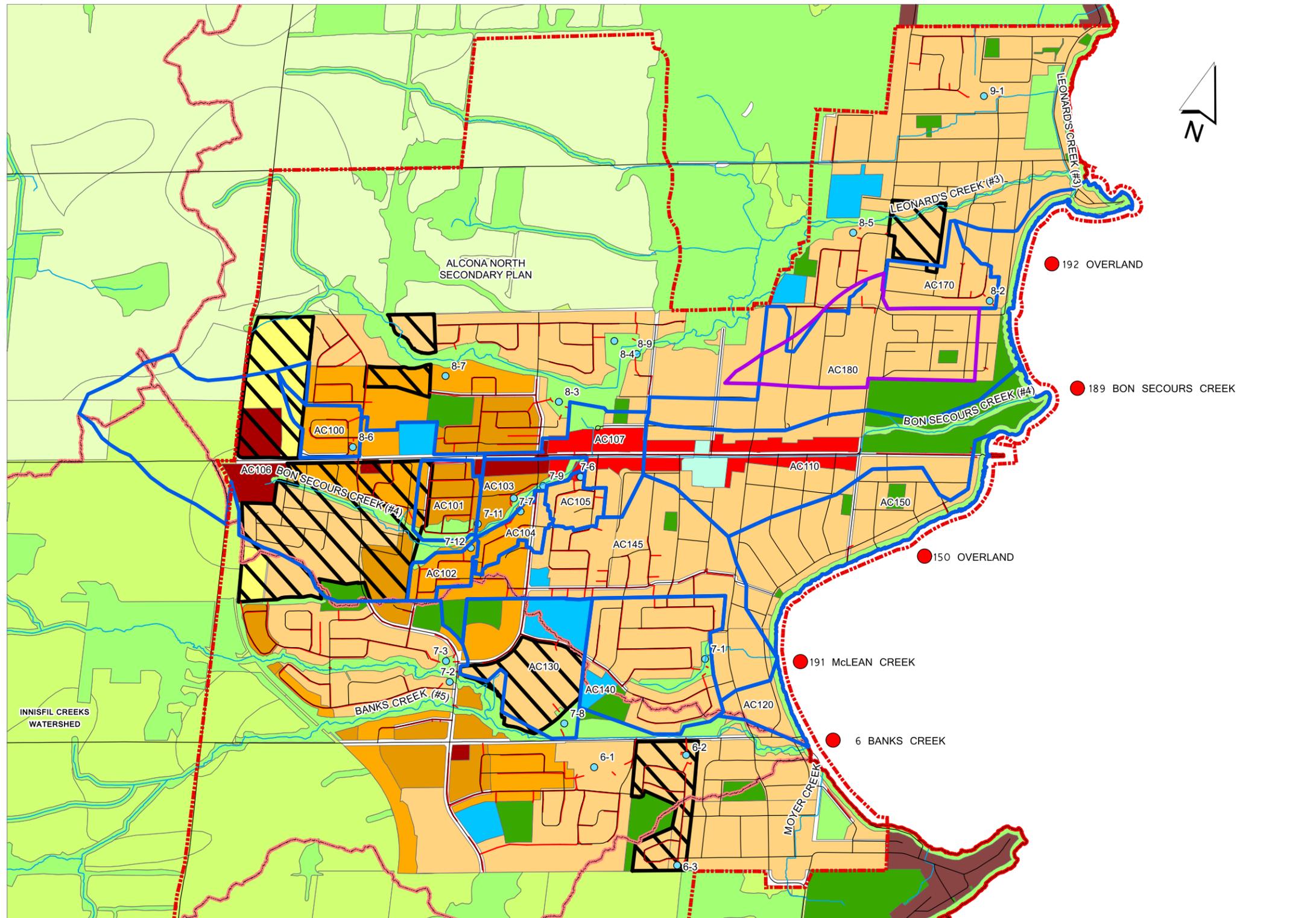


Lake Simcoe
Region
Conservation
Authority



This product was produced by the Lake Simcoe Region Conservation Authority and some information depicted on this map may have been compiled from various sources. While every effort has been made to accurately depict the information, data / mapping errors may exist.
 This map has been produced for illustrative purposes only.
 LSRCA GIS Services DRAFT created July 2010.
 © LAKE SIMCOE REGION CONSERVATION AUTHORITY, 2010. All Rights Reserved
 The following datasets roads, municipal boundaries and Oak Ridges Moraine are © Queens Printer for Ontario, 2010. Reproduced with Permission





Legend

- ROADS
- WATERCOURSE
- STORM SEWER
- STORMWATER MANAGEMENT POND
- AC150 VO2 CATCHMENT ID
- OP DESIGNATED FUTURE DEVELOPMENT AREAS
- SUBCATCHMENT BOUNDARY
- STUDY AREA
- HYDROLOGIC REFERENCE POINT

SOURCE:
TOWN OF INNISFIL
(OFFICIAL PLAN TOWN OF INNISFIL DATE: JULY, 2006)

DISCLAIMER
The information contained in this drawing is solely for the use of the Corporation of the Town of Innisfil for the purpose for which it has been prepared. The Corporation of the Town of Innisfil and C.C. Tatham & Associates Ltd. undertakes no duty to or accepts any responsibility to any third party who may rely upon this drawing.

This drawing may not be used for any purpose other than that provided in the contract between the Corporation of the Town of Innisfil and C.C. Tatham & Associates Ltd. nor may any detail or element of this drawing be removed, reproduced, electronically stored or transmitted in any form without the express written consent of C.C. Tatham & Associates Ltd.

OFFICIAL PLAN LAND USE

- | | | | |
|--------------------------------|------------------------------------|----------------------------|----------------------------|
| FUTURE URBAN | COMMERCIAL-CORE | INDUSTRIAL-GENERAL | RESIDENTIAL-LOW DENSITY 1 |
| FUTURE RESIDENTIAL | COMMERCIAL-HIGHWAY | INSTITUTIONAL | RESIDENTIAL-LOW DENSITY 2 |
| AGRICULTURE AREA | COMMERCIAL-NEIGHBOURHOOD | LANDFILL | RESIDENTIAL-MEDIUM DENSITY |
| AGRICULTURE-RURAL AREA | COMMERCIAL-NEIGHBOURHOOD/MIXED USE | LANDFILL-CLOSED | RESIDENTIAL-RETIREMENT |
| AGRICULTURE-SPECIAL RURAL AREA | COMMERCIAL-SHORELINE | NATURAL ENVIRONMENTAL AREA | RESIDENTIAL-VILLAGE |
| BUSINESS PARK | COMMERCIAL-VILLAGE | NEIGHBOURHOOD PARK | STORMWATER MANAGEMENT |
| COMMERCIAL-CONVENIENCE | COMMUNITY SERVICE | PARKS AND OPEN SPACE | |
| | INDUSTRIAL-EXTRACTIVE | RESIDENTIAL-ESTATE | |

C.C. Tatham & Associates Ltd.
Consulting Engineers
Collingwood Bracebridge Orillia Barrie

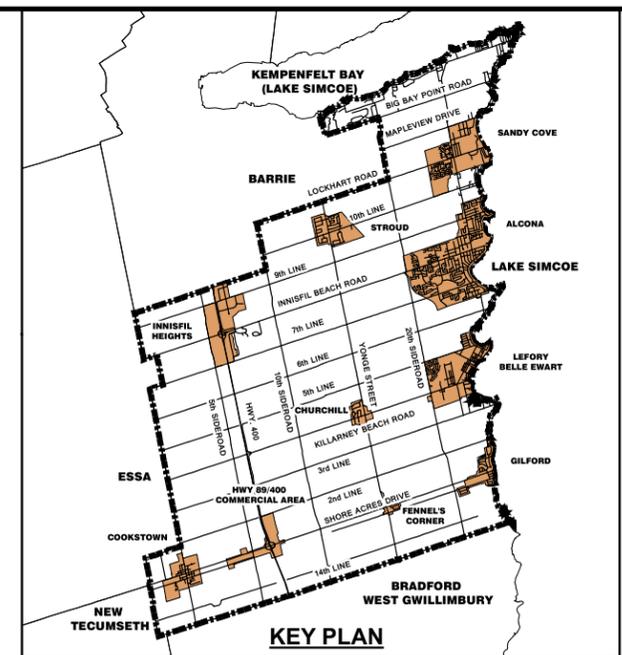
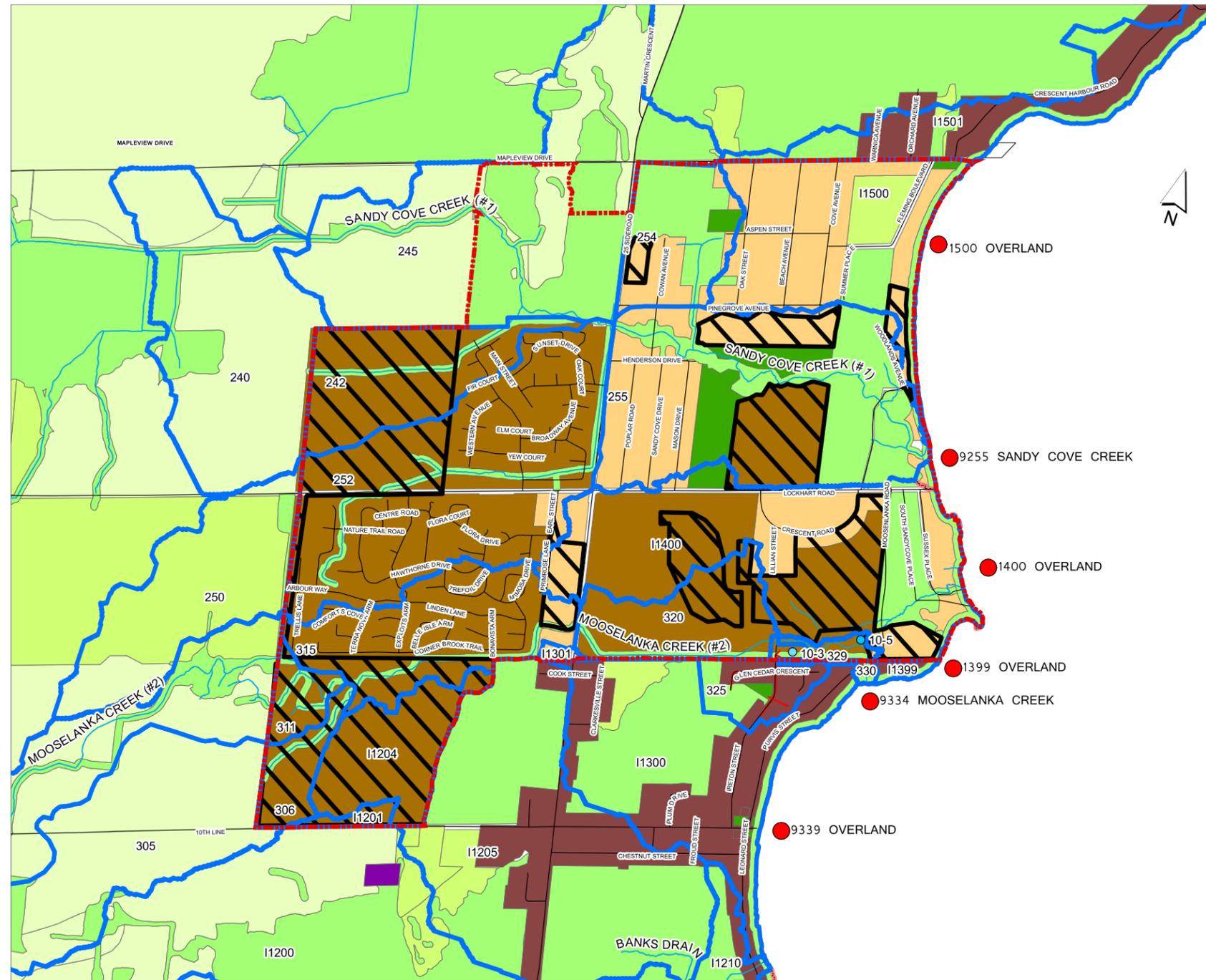


COMPREHENSIVE STORMWATER MANAGEMENT MASTER PLAN

STUDY AREA CATCHMENT FIGURE- ALCONA CENTRAL

FIGURE 9-A

DATE: MARCH 2016



Legend

- ROADS
- WATERCOURSE
- STORM SEWER
- STORMWATER MANAGEMENT POND
- 320 V02 CATCHMENT BOUNDARY
- OP DESIGNATED FUTURE DEVELOPMENT AREAS
- SUBCATCHMENT BOUNDARY
- STUDY AREA
- HYDROLOGIC REFERENCE POINT

SOURCE:
TOWN OF INNISFIL
(OFFICIAL PLAN TOWN OF INNISFIL DATE: JULY, 2006)

DISCLAIMER

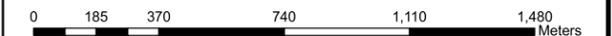
The information contained in this drawing is solely for the use of the Corporation of the Town of Innisfil for the purpose for which it has been prepared. The Corporation of the Town of Innisfil and C.C. Tatham & Associates Ltd. undertakes no duty to or accepts any responsibility to any third party who may rely upon this drawing.

This drawing may not be used for any purpose other than that provided in the contract between the Corporation of the Town of Innisfil and C.C. Tatham & Associates Ltd. nor may any detail or element of this drawing be removed, reproduced, electronically stored or transmitted in any form without the express written consent of C.C. Tatham & Associates Ltd.

OFFICIAL PLAN LAND USE

- | | | | |
|--------------------------------|------------------------------------|----------------------------|----------------------------|
| FUTURE URBAN | COMMERCIAL-CORE | INDUSTRIAL-GENERAL | RESIDENTIAL-LOW DENSITY 1 |
| FUTURE RESIDENTIAL | COMMERCIAL-HIGHWAY | INSTITUTIONAL | RESIDENTIAL-LOW DENSITY 2 |
| AGRICULTURE AREA | COMMERCIAL-NEIGHBOURHOOD | LANDFILL | RESIDENTIAL-MEDIUM DENSITY |
| AGRICULTURE-RURAL AREA | COMMERCIAL-NEIGHBOURHOOD/MIXED USE | LANDFILL-CLOSED | RESIDENTIAL-RETIREMENT |
| AGRICULTURE-SPECIAL RURAL AREA | COMMERCIAL-SHORELINE | NATURAL ENVIRONMENTAL AREA | RESIDENTIAL-VILLAGE |
| BUSINESS PARK | COMMERCIAL-VILLAGE | NEIGHBOURHOOD PARK | STORMWATER MANAGEMENT |
| COMMERCIAL-CONVENIENCE | COMMUNITY SERVICE | PARKS AND OPEN SPACE | |
| | INDUSTRIAL-EXTRACTIVE | RESIDENTIAL-ESTATE | |

C.C. Tatham & Associates Ltd.
Consulting Engineers
Collingwood Bracebridge Orillia Barrie



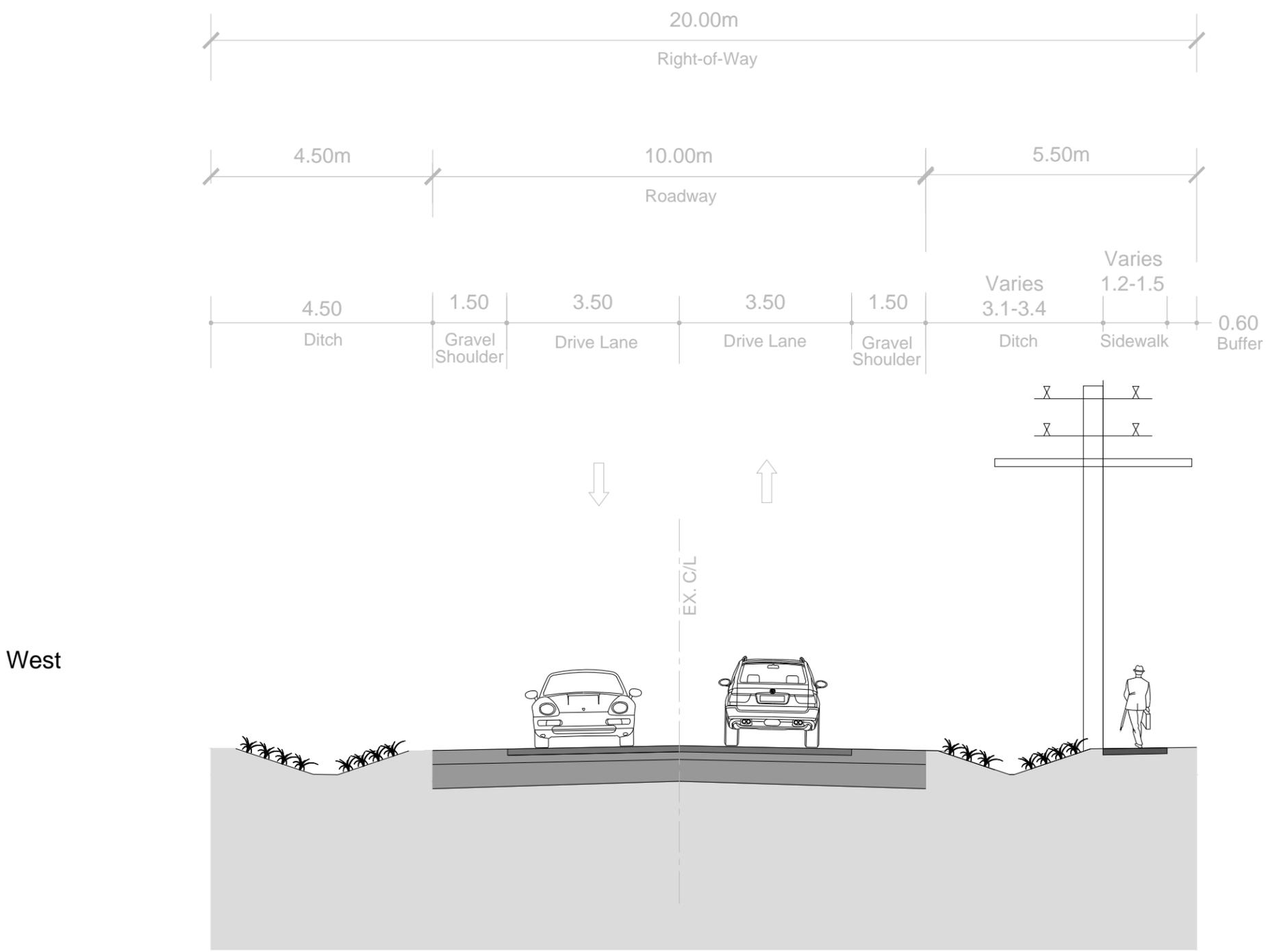
COMPREHENSIVE STORMWATER MANAGEMENT MASTER PLAN

STUDY AREA CATCHMENT FIGURE SANDY COVE

FIGURE 9-F

DATE: MARCH 2016

C:\Users\Vivian.Mak\WSP\0365211-06027-00 Innisfil 25th Sideroad - 14 Tech_Prof Services\14.07 Municipal_Roads\2021.07.21 - Typical Sections



West

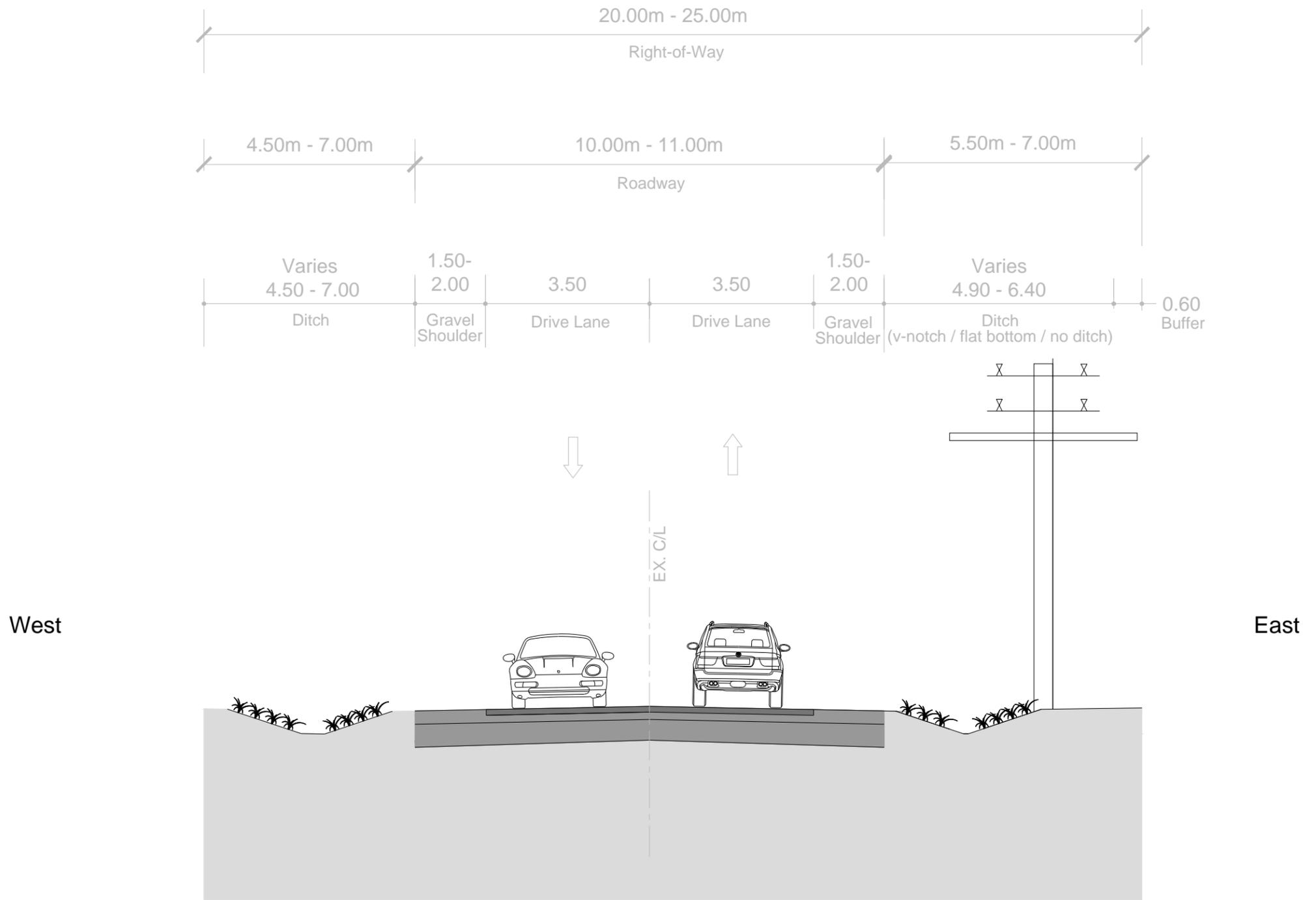
East

DRAFT - for discussion

Figure 1A
 25th Side Road - Low Density Residential - Existing Condition (south of Rose Lane)
 Innisfil 25th Side Road



C:\Users\vivian.mak\WSP\0365211-06027-00\Innisfil\25th Side Road - 14_Tech_Prof Services\14.07_Municipal_Roads\2021.07.21 - Typical Sections

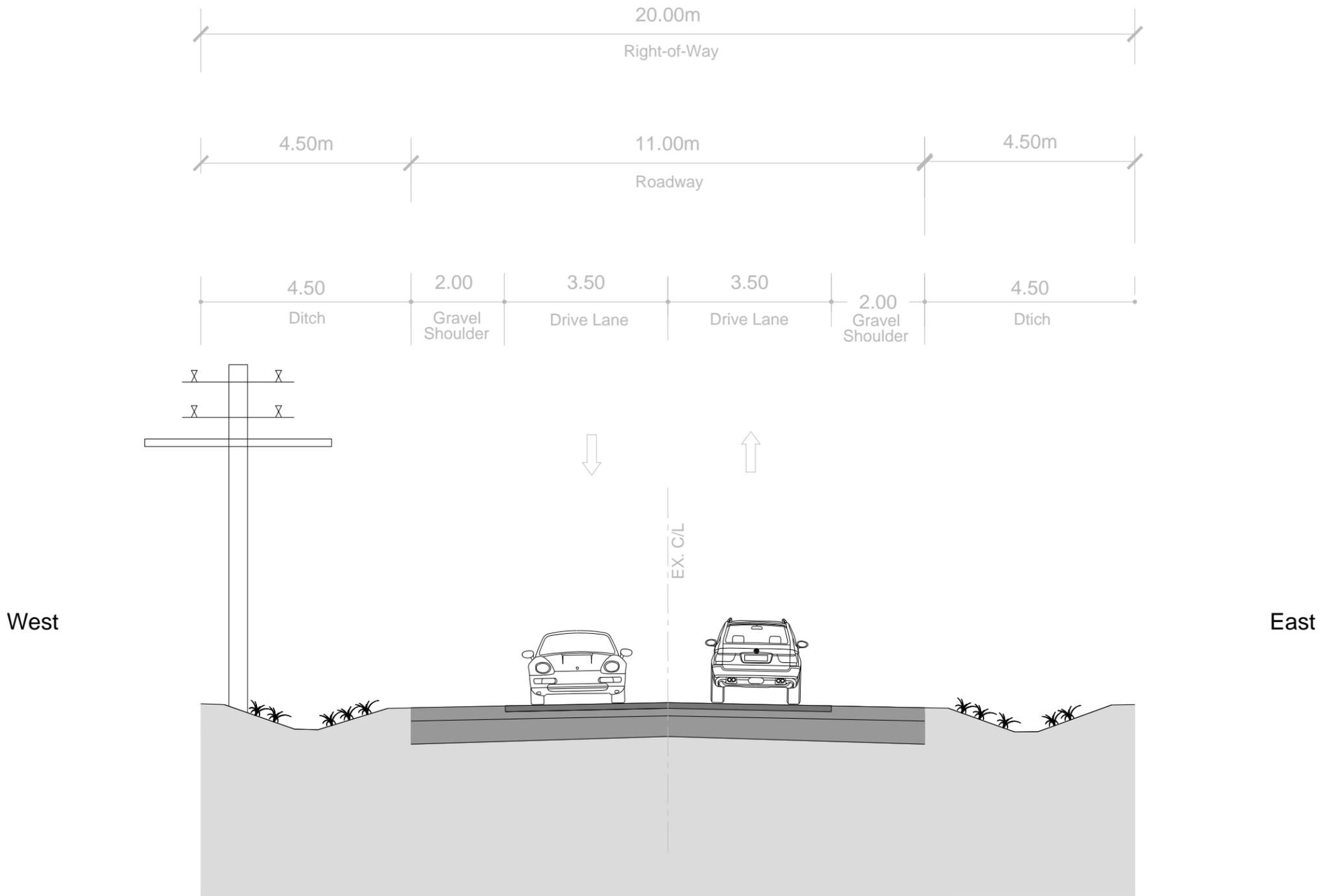


DRAFT - for discussion

Figure 1B
25th Side Road - Low Density Residential - Existing Condition (north of Rose Lane)
Innisfil 25th Side Road



C:\Users\WVian.Mak\WSP\0365211-06027-00 Innisfil 25th Sideroad - 14_Tech_Prof Services\14.07 Municipal_Roads\2021.07.21 - Typical Sections

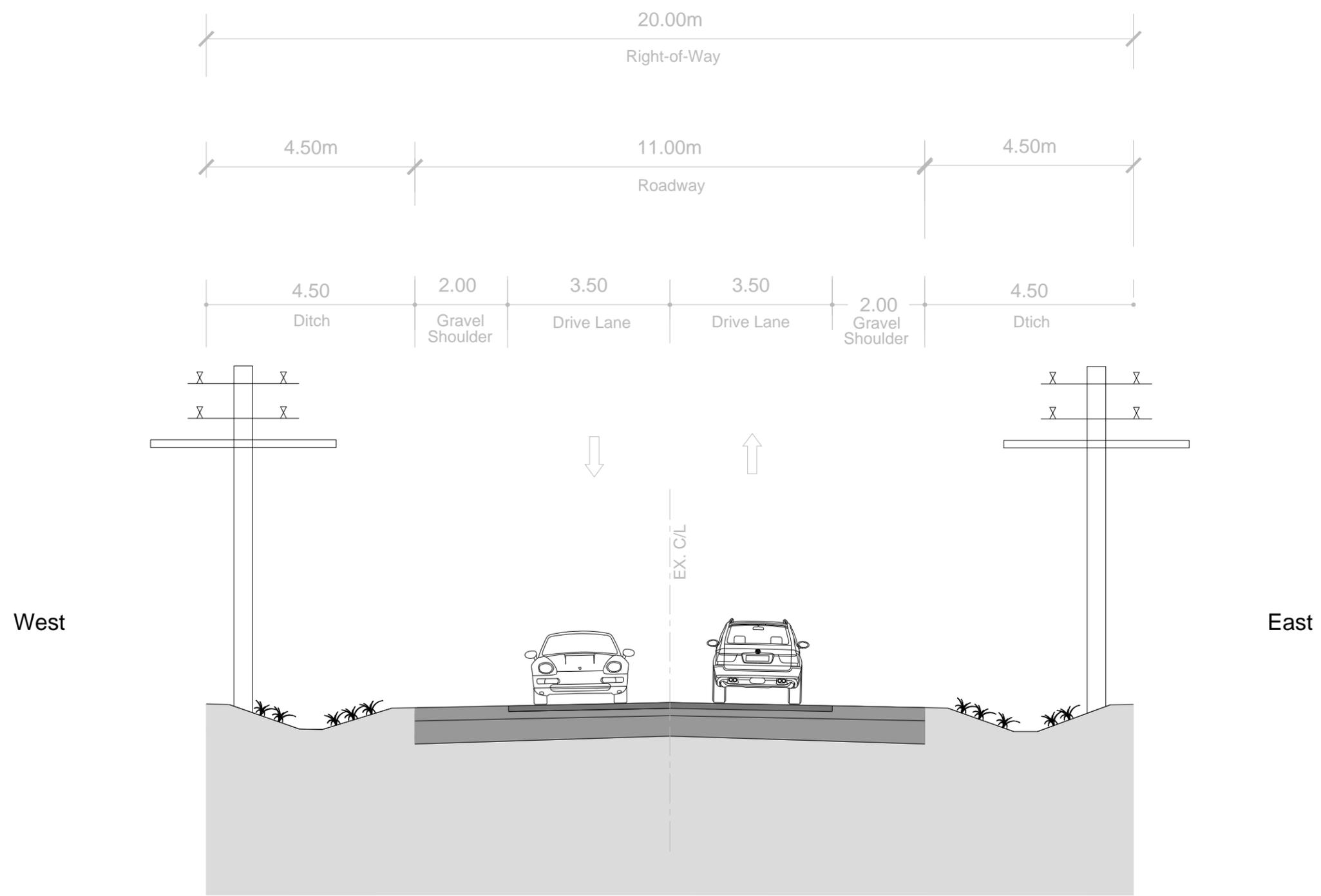


DRAFT - for discussion

Figure 5
25th Side Road - Green Space - Existing Condition
Innisfil 25th Side Road



C:\Users\WVian.Mak\WSP\0365211-06027-00 Innisfil 25th Sideroad - 14_Tech_Prof Services\14.07 Municipal_Roads\2021.07.21 - Typical Sections

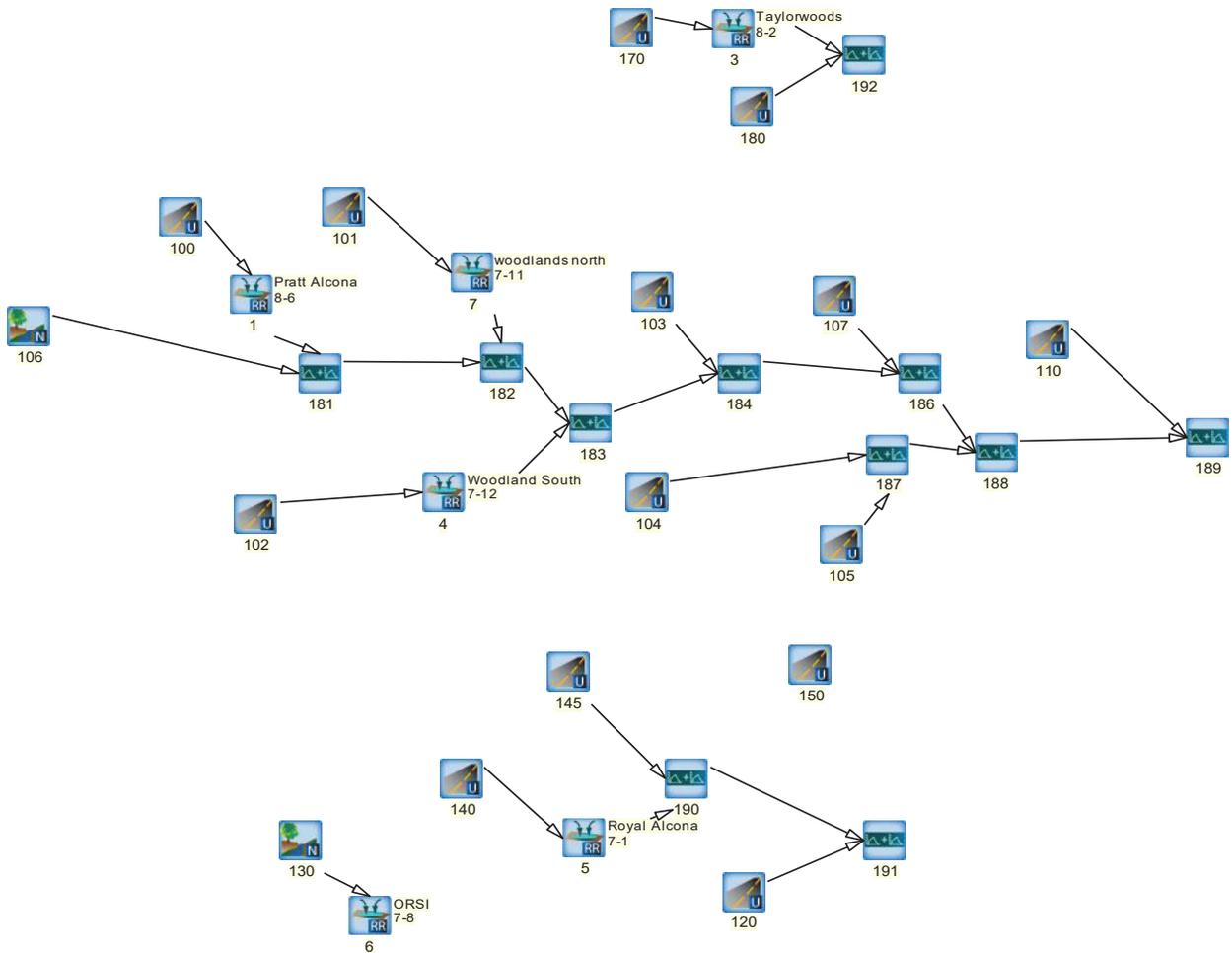


DRAFT - for discussion

Figure 9
25th Side Road - Downtown Commercial - Existing Condition
Innisfil 25th Side Road



INNISFIL COMPREHENSIVE STORMWATER MANAGEMENT MASTERPLAN
EXISTING CONDITIONS
ALCONA



Nashyd



Standhyd



Addhyd



Route Pipe



Route Channel



Route Reservoir



Duhyd



Diverhyd



C.C. TATHAM & ASSOCIATES LTD.
 Consulting Engineers

Project: Innisfil SWM Master Plan

File No.: 413448

Subject: Otthymo Flow Schematic

Date: Jun-14 **Figure:** 1



Assessment of Life Cycle Costs for Low Impact Development Stormwater Management Practices



ASSESSMENT OF LIFE CYCLE COSTS FOR LOW IMPACT DEVELOPMENT STORMWATER MANAGEMENT PRACTICES

Final Report

Prepared by:

Toronto and Region Conservation
Project Leads:
Tim Van Seters, Christy Graham, Lisa Rocha

and

University of Toronto, Department of Civil Engineering
Project Leads:
Mariko Uda, Chris Kennedy

under the

Sustainable Technologies Evaluation Program

April 2013

© Toronto and Region Conservation Authority

NOTICE

The contents of this report do not necessarily represent the policies of the supporting agencies. Although every reasonable effort has been made to ensure the integrity of the report, the supporting agencies do not make any warranty or representation, expressed or implied, with respect to the accuracy or completeness of the information contained herein. Mention of trade names or commercial products does not constitute endorsement or recommendation of those products. No financial support was received from developers, manufacturers or suppliers of technologies evaluated in this project.

PUBLICATION INFORMATION

This research was undertaken collaboratively between the Toronto and Region Conservation Authority's (TRCA) Sustainable Technologies Evaluation Program (project leads: Tim Van Seters, Lisa Rocha, Christy Graham) and the University of Toronto, Civil Engineering Department (project leads: Mariko Uda and Chris Kennedy).

Citation: Uda, M., Van Seters, T., Graham, C., Rocha, L., 2013. *Evaluation of Life Cycle Costs for Low Impact Development Stormwater Management Practices*. Sustainable Technologies Evaluation Program, Toronto and Region Conservation Authority.

Comments on this document, or requests for other studies conducted under STEP should be directed to:

Tim Van Seters
Manager, Sustainable Technologies
Toronto and Region Conservation Authority
5 Shoreham Drive,
Downsview, Ontario
M3N 1S4

Tel: 289-268-3902
Fax: 416-661-6898
E-mail: tvanseters@trca.on.ca

THE SUSTAINABLE TECHNOLOGIES EVALUATION PROGRAM

The Sustainable Technologies Evaluation Program (STEP) is a multi-agency program, led by the Toronto and Region Conservation Authority (TRCA). The program helps to provide the data and analytical tools necessary to support broader implementation of sustainable technologies and practices within a Canadian context. The main program objectives are to:

- monitor and evaluate clean water, air and energy technologies;
- assess barriers and opportunities for implementing technologies;
- develop supporting tools, guidelines and policies; and
- promote broader use of effective technologies through research, education and advocacy.

Technologies evaluated under STEP are not limited to physical products or devices; they may also include preventative measures, alternative urban site designs, and other innovative practices that help create more sustainable and liveable communities.

ACKNOWLEDGEMENTS

Funding support for this project was generously provided by:

- Government of Canada's Great Lakes Sustainability Fund
- City of Toronto
- Regional Municipality of York
- Regional Municipality of Peel
- National Science and Research Council Industrial Postgraduate Scholarship

EXECUTIVE SUMMARY

This project evaluates the capital and life cycle costs of Low Impact Development (LID) practices over a 50 year time horizon based on a detailed assessment of local input costs, maintenance requirements, rehabilitation costs and design scenarios relevant to Canadian climates. The LID practices evaluated include bioretention cells, permeable pavement, infiltration trenches and chambers, enhanced swales, rainwater harvesting and green roofs. Dry swales and perforated pipe systems were considered to be similar to bioretention and infiltration trenches, respectively, and therefore were not evaluated as separate practices. The savings from LID approaches associated with improved aesthetics, air quality, community livability and other public benefits were not assessed, as these savings are best evaluated in relation to specific case study examples.

A robust and replicable methodology was used to compile capital and life cycle costs for the LID practices evaluated in this project. Model designs were developed for up to 3 typical variations of each LID practice assuming a 2000 m² paved and/or roof drainage area. An RSMeans database, widely used for construction and maintenance cost estimation, was used as the basis for most of the costing. Where RSMeans cost data were not available, costs were derived from other sources (e.g. supplier quotes, experienced construction managers). Maintenance and rehabilitation schedules for each practice were assessed based on local guidance manuals and literature sources.

Model LID practice design costs evaluated in this study indicated that bioretention, infiltration chambers, infiltration trenches and enhanced swales are some of the least expensive practices to implement when only the practice cost itself is considered. The practice of rainwater harvesting provides additional savings by reducing the cost of potable water supplies. Permeable pavements are comparably more expensive than most other practices, but in many instances these costs would be offset to some extent by a reduction in the need to pave the drainage area, since the pavements serve both as a parking surface and stormwater treatment practice. The practice also does not require as much land as some other practices, making it particularly well suited to retrofit contexts. Green roofs are the most expensive practice as they are installed in less accessible locations and need to be carefully engineered to protect the integrity of the building envelope. This practice is often selected because of its aesthetic, biodiversity and energy saving benefits, as well as its overall contribution to green building rating schemes, the value of which were not considered in the cost assessment provided in this study.

An analysis of different treatment scenarios for an asphalt parking lot revealed that LID practices had comparable life cycle costs to conventional treatment using an oil grit separator (OGS). Incorporating the stormwater treatment benefits of the practices into the analysis showed that LID practice life cycle costs were between 35 and 77% less than conventional OGS treatment.

A spreadsheet decision support tool based on the cost calculations gathered during this study was developed to assist industry professionals calculate the initial capital and life cycle costs of site specific LID practice designs. The tool provides users with a more comprehensive understanding of all relevant costs, facilitates cost comparisons, and allows users to optimize proposed designs based on both performance and cost. The tool is available free of charge on the Toronto and Region Conservation's Sustainable Technologies Evaluation Program website.

TABLE OF CONTENTS

EXECUTIVE SUMMARY	iv
1.0 INTRODUCTION	1
1.1 Background.....	1
1.2 Project Objectives	2
2.0 LIFE CYCLE COSTING METHODOLOGY	3
2.1 Costing Methodology.....	3
2.1.1 <i>Preparation of Model Designs</i>	3
2.1.2 <i>Construction Costing</i>	3
2.1.3 <i>Establishing Maintenance and Rehabilitation Requirements and Costs</i>	4
2.1.4 <i>Life Cycle Cost Calculation</i>	5
2.1.5 <i>Comparison to Literature</i>	6
3.0 CAPITAL AND LIFE CYCLE COSTS.....	7
3.1 Bioretention.....	7
3.1.1 <i>Model Scenarios and Designs</i>	7
3.1.2 <i>Capital Costs</i>	10
3.1.3 <i>Life Cycle Costs</i>	11
3.2 Permeable Pavement	12
3.2.1 <i>Model Scenarios and Designs</i>	12
3.2.2 <i>Capital Costs</i>	15
3.2.3 <i>Life Cycle Costs</i>	16
3.3 Infiltration Trenches	17
3.3.1 <i>Model Scenarios and Designs</i>	17
3.3.2 <i>Capital Costs</i>	22
3.3.3 <i>Life cycle Costs</i>	22
3.4 Infiltration Chambers	23
3.4.1 <i>Model Scenarios and Designs</i>	23
3.4.2 <i>Capital Costs</i>	27
3.4.3 <i>Life Cycle Costs</i>	27
3.5 <i>Enhanced Grass Swales</i>	28
3.5.1 <i>Model Scenarios and Designs</i>	28
3.5.2 <i>Capital Costs</i>	29
3.5.3 <i>Life Cycle Costs</i>	30
3.6 Rainwater Harvesting.....	30
3.6.1 <i>Model Scenarios and Designs</i>	30
3.6.2 <i>Capital Costs</i>	36
3.6.3 <i>Life Cycle Costs</i>	36
3.7 Extensive Greenroof	37

3.7.1 Model Scenario and Designs	37
3.7.2 Capital Costs	38
3.7.3 Life Cycle Costs	39
4.0 COMPARISON OF LID PRACTICE COSTS	41
4.1 Capital Costs	41
4.2 Life Cycle Costs	43
4.3 Comparisons to Conventional Grey Infrastructure	46
4.4 Comparison of Study Findings to Other Literature	48
4.4.1 Review of literature on LID construction costs	49
4.4.2 Review of literature on LID maintenance and rehabilitation costs	52
5.0 CONCLUSIONS	55
6.0 REFERENCES	56

LIST OF TABLES

Table 3.1: Bioretention capital costs	11
Table 3.2: Bioretention life cycle costs	11
Table 3.3: Permeable pavement and conventional asphalt capital costs	16
Table 3.4: Permeable pavement and conventional asphalt life cycle costs	17
Table 3.5: Infiltration trench capital costs	22
Table 3.6: Infiltration trench life cycle costs.....	22
Table 3.7: Infiltration chambers capital costs	27
Table 3.8: Infiltration chambers life cycle costs.....	28
Table 3.9: Enhanced grass swale capital costs	29
Table 3.10: Enhanced grass swale life cycle costs	30
Table 3.11: Rainwater harvesting capital costs.....	36
Table 3.12: Rainwater harvesting life cycle costs	36
Table 3.13: Extensive greenroof capital costs.....	39
Table 3.14: Extensive greenroof life cycle costs	40
Table 4.1: Life cycle costs for all practices.....	44
Table 4.2: Estimated reductions in runoff, TSS concentrations and loads for six asphalt treatment scenarios	47
Table 4.3: LID capital cost comparison to literature/other models	50
Table 4.4: LID maintenance and rehabilitation cost comparison to literature/other models	53

LIST OF FIGURES

Figure 3.1: Bioretention full infiltration design	8
Figure 3.2: Bioretention partial infiltration design.....	9
Figure 3.3: Bioretention no infiltration design.....	10
Figure 3.4: Permeable pavement full infiltration design	13
Figure 3.5: Permeable pavement partial infiltration design.....	14
Figure 3.6: Permeable pavement no infiltration design	15
Figure 3.7: Plan view of the infiltration trench receiving roof runoff only	18

Figure 3.8: Cross section of infiltration trench receiving roof runoff only	19
Figure 3.9: Plan view of the infiltration trench receiving road and roof runoff.....	20
Figure 3.10: Cross section of the infiltration trench receiving road and roof runoff	21
Figure 3.11: Sections of corrugated wall chamber	23
Figure 3.12: Plan view of infiltration chambers receiving roof runoff only	24
Figure 3.13: Plan view for infiltration chambers receiving road and roof runoff.....	25
Figure 3.14: Cross section of infiltration chambers receiving road and roof runoff	26
Figure 3.15: Plan view of enhanced grass swale	29
Figure 3.16: Cross section of enhanced grass swale	29
Figure 3.17: Pre-cast concrete tank design	32
Figure 3.18: Site design for buried pre-cast concrete tank.....	33
Figure 3.19: Plastic tank design	34
Figure 3.20: Site design for indoor plastic tank.....	35
Figure 3.21: Plan view of greenroof design	38
Figure 3.22: Cross section of greenroof design	38
Figure 4.1: Capital costs for all practices per m ² of roof and/or paved drainage area	42
Figure 4.2: Present values for 25 year and 50 year evaluation periods for all practices per m ² of roof and/or paved drainage area	45
Figure 4.3: Capital and life cycle costs for different asphalt runoff treatment scenarios	47
Figure 4.4: Capital and life cycle costs expressed per kilogram of total suspended solids load reduced ..	48

Appendix A: Detailed Costing

Appendix B: Maintenance Costs

Appendix C: Life Cycle Maintenance Costs

1.0 INTRODUCTION

1.1 Background

Over the past several years, the practice of stormwater management in Ontario has shifted from an approach focused narrowly on centralized water quality treatment and peak flow control towards a broader, more decentralized approach oriented towards maintaining or re-establishing the pre-development hydrologic regime. This new approach utilizes a series of decentralized micro controls at or near the source of drainage networks to supplement conventional detention facilities. Alterations to the pre-development urban water cycle are minimized through site planning techniques and measures aimed at infiltrating, filtering, evaporating and detaining runoff, as well as preventing pollution. In Ontario and some parts of the United States, this approach is commonly referred to as Low Impact Development (LID), and includes measures such as green roofs, permeable pavement, bioretention, infiltration trenches, swales and alternative site design strategies.

Within the Greater Toronto Area, the results of several years of watershed monitoring and modeling, published in documents such as the Toronto and Region Conservation's (TRCA) Watershed Plans for the Rouge (2007), Humber (2008) and Don (2009) Rivers have concluded that this shift towards Low Impact Development is essential to protect watershed health and improve the resilience of watercourses to the hydrologic impacts associated with climate change. In July 2010 TRCA and Credit Valley Conservation (CVC) released the *Low Impact Development Stormwater Management Planning and Design Guide* (hereafter referred to as *LID Guide*) to assist local developers, consultants, municipalities and landowners to better understand, plan and implement LID stormwater management practices. The *LID Guide* provides a wealth of information on the planning, selection, and design of LID, and helps to streamline the design and review process to encourage widespread adoption of these technologies. Uncertainties remain, however, about the capital and long terms costs associated with these technologies relative to conventional end-of-pipe approaches.

While there are software tools and literature that provide detailed cost data for LID practices, particularly with respect to the capital costs of materials and labour, many of these resources (e.g. WERF, 2009; Olson et al, 2010) are based on markets in the U.S. or other countries, and are therefore not directly applicable to local conditions. These resources also often use designs that are either no longer considered best practice, or are not in accordance with cold climate design adaptations commonly used in Ontario. Life cycle costs provided in this report are directly applicable to Ontario because they are derived, to the extent possible, from local sources and based on design specifications provided in the *LID Guide*, which incorporates design modifications and maintenance considerations relevant to local geologic and climatic conditions.

In addition to research on the capital and long term operation and maintenance costs of LID, there are also several studies that attempt to quantify the value of LID based on the full range of costs and benefits to the individual site owner, the community and broader public. One such

study, conducted by the USEPA in 2007 reported lower total costs for 11 of 12 green infrastructure projects relative to conventional grey infrastructure. Savings were often realized due to reduced costs for site grading and preparation, stormwater drainage infrastructure, curbs and gutters, site paving and downstream stormwater treatment. Other studies have attempted to monetize the broader public benefits of the practices (e.g. Odefey et al, 2012; Buckley et al, 2012a, 2012b; Marbek, 2010). These include avoided costs associated with reduced runoff and water quality (e.g. reduced frequency of combined sewer overflows, lower stream erosion rates) as well as benefits related to energy, air quality, climate change, urban heat island, habitat improvements and aesthetics. These studies have shown that LID approaches can lead to significant long term fiscal savings for local governments.

1.2 Project Objectives

The purpose of this project is to evaluate the capital and life cycle costs of LID practices over a 50 year time horizon based on a detailed assessment of local input costs, maintenance requirements and specific design scenarios presented in the *LID Guide*. The following practices are evaluated:

- Bioretention cells
- Permeable Interlocking Concrete Pavement
- Infiltration trenches
- Infiltration chambers
- Enhanced swales
- Rainwater harvesting, and
- Green roofs

Dry swales and perforated pipe systems were considered to be similar to bioretention and infiltration trenches, respectively, and therefore were not costed out as separate practices. The savings from LID approaches associated with improved aesthetics, air quality, community livability and other public benefits were not assessed, as these are best evaluated in relation to specific case study examples.

A spreadsheet decision support tool based on the cost calculations gathered during this study was developed to assist industry professionals calculate the initial capital and life cycle costs of site specific LID practice designs. The tool provides users with a more comprehensive understanding of all relevant costs, facilitates cost comparisons, and allows users to optimize proposed designs based on both performance and cost. The tool is available free of charge on the Toronto and Region Conservation's Sustainable Technologies Evaluation Program website.

2.0 LIFE CYCLE COSTING METHODOLOGY

2.1 Costing Methodology

The following steps were followed to develop detailed costs of all the LID measures.

2.1.1 Preparation of Model Designs

Model designs were developed for up to 3 typical variations of each LID practice assuming a 2000 m² paved and/or roof drainage area. The conceptual designs were developed for costing purposes based on design guidelines provided in the *LID Guide* (TRCA and CVC, 2010). This information was supplemented with other guidelines, literature references and professional advice when additional information was needed. Several conceptual designs were based on existing applications of the practices within the GTA.

The process and steps involved in construction of the practices were obtained from the *LID guide*, a review of regulatory requirements, and other references as needed. This step in the costing process describes the construction sequence, construction methods, and details of additional tasks required prior to undertaking construction (e.g. soil testing).

2.1.2 Construction Costing

All material, delivery, labour, equipment (rental, operating, operator), hauling and disposal costs were included in the cost spreadsheet. The RSMeans database (Toronto, 2010) was used as the basis for most of the costing. This standard database used widely for construction cost estimation provides detailed unit material (including delivery), labour and equipment costs. The costs in RSMeans marked “O&P” were used, which are the installing contractor’s price including their overhead and profit. It was assumed there would be no general contractor for the construction project.¹ Standard Union labour costs were used, which are about 18%² higher than Open Shop labour costs. Note that the RSMeans costs do not include sales tax.

Where data were not available in RSMeans, costs were solicited from other sources (e.g. suppliers, experienced construction managers). These costs were often Open Shop labour rates and did not include sales tax. For rainwater harvesting, costs were obtained from an existing tool developed in 2010 through a partnership between University of Guelph, TRCA and Connect the Drops (STEP, 2011). The costs in the tool were also based on RSMeans 2010, and were cross

¹ If a general contractor were used, there would be an average 10% markup as well as general contractor main office overhead & profit (RSMeans)

² Standard union costs are 16% more than open shop costs for a light truck driver, and 19% more for a light equipment operator as well as for a common building labourer (RSMeans, 2010 US average). Therefore on average 18% higher.

checked and supplemented as needed to ensure consistency with the methodology used in this study.

In compiling the cost data it was assumed that the practice was being constructed as part of a larger new development, and therefore mobilization/demobilization costs were not included unless a particular piece of equipment (e.g. crane for green roof) would not normally have been present on the site. Also, it was assumed that excavated soil could be dumped elsewhere on site.

Costs that would have been incurred whether or not the LID was being constructed were normally not included (e.g., for rainwater harvesting, the pipes collecting runoff from the roof were not included because they would be required regardless). One exception is for green roofs, where the cost of the roof with and without the roof membrane was assessed.

For all LIDs, the following overhead costs were assumed:

- Construction management (4.5%),
- Design (2.5%), small tools (0.5%),
- Clean up (0.3%).

These are at the low end of the cost range suggested by RSMeans. Also, no contingency costs were included.

In rare instances, suitable costing data could not be found, in which case costs were estimated based on other data or costs from similar equipment or task. All assumptions and sources of data were documented.

2.1.3 *Establishing Maintenance and Rehabilitation Requirements and Costs*

Maintenance tasks and frequencies were determined based on the *LID guide* and other references where necessary. Assessment of the life span of the practices was based on literature where available, but in cases where there was conflicting information, a judgment was made based on a 'weight of evidence' approach. Assumptions on practice life spans are provided in each case to provide readers with a basis for interpretation of results.

The costs of maintenance and rehabilitation were determined using the same approach as for the construction costing. One difference, however, was that (de)mobilization of equipment was included as equipment would not already be on site. Design costs were not included in the rehabilitation or replacement costs as it was assumed that the original LID practice design would be used to inform this work.

2.1.4 Life Cycle Cost Calculation

Once all capital, maintenance and rehabilitation costs were determined, the lifecycle cost for each model design was calculated based on an evaluation period of 50 years, which is typical of the time span over which infrastructure decisions are made. The approach used was similar to that in the *Best Management Practice and Low Impact Development Whole Life Cost Models* developed by the Water Environment Research Foundation (2009). WERF's analysis includes any rehabilitation required within the 50 year period. At the end of 50 years, the LID is considered to have no salvage value, and no extra value is attributed to the additional lifespan expected for the LID beyond the 50 year mark.

The present value of the cost of each LID model design was calculated as follows:

$$PV = \text{design and construction cost} + PV \text{ of maintenance} + PV \text{ of rehabilitation}$$

The following present value formula was used to obtain the present value of the future cost:

$$PV = FC / (1 + r)^n$$

where,

PV = present value in \$

FC = future cost in \$

r = discount rate

n = year of future cost

Discount rates of 0, 3, and 5% were considered. Inflation was assumed to be 0%.

In addition to the 50-year analysis, a 25-year analysis was conducted. This was done to eliminate the impact on cost of any major rehabilitation that occurs in later years. Note that for the 50-year analysis, major maintenance activities that would normally be done at the 50 year mark were not included as the LID was assumed to retire after 50 years. For the 25-year analysis, however, these major maintenance activities were included at year 25 as it was expected that the LID would continue to be used.

In addition to the Net Present Values, the *annual average maintenance cost* and the *rehabilitation cost* were determined. The *annual average maintenance cost* does not include rehabilitation and as such represents an average of regular maintenance activities over the 50 year time period. The *rehabilitation cost* includes not only the cost of the actual rehabilitation but also of the consequent changes in maintenance activities. Thus the cost of the actual rehabilitation (not including maintenance activities) were added and maintenance tasks were removed³, added⁴ or

³ When a rehab occurs, some maintenance activities are no longer needed in that year (e.g., no need to repair small leak in green roof membrane).

⁴ When a rehab occurs, some additional maintenance activities are required (e.g. watering green roof).

shifted⁵ in time as a result of the rehabilitation. The total cost of maintenance plus rehabilitation over 50 years was then summed. The difference between this sum and the total maintenance cost over 50 years in the scenario where no rehabilitation was required was calculated. This difference was the *rehabilitation cost*.

2.1.5 Comparison to Literature

A literature review was conducted for each LID to compare the construction and maintenance costs established in this study to other sources. The literature review was not meant to be comprehensive, as there are limited cost data available on LID practices, and those that are available are not necessarily applicable to local conditions. Thus the literature reviews consisted of comparisons to only a few references. Since different studies included different design assumptions, not all of which were clearly described, a straightforward comparison to our results was difficult to achieve.

⁵ When a rehab occurs, some maintenance activities may be shifted to later years (e.g. do not have to repair small leak in green roof membrane for next 10 years).

3.0 CAPITAL AND LIFE CYCLE COSTS

3.1 Bioretention

Bioretention uses the natural properties of soils, plants and associated microbial activity to infiltrate water and remove pollutants from stormwater runoff. It can be designed in various ways but the most common form consists of a shallow, excavated depression with layers of stone, prepared soil mix, mulch and specially selected native vegetation that is tolerant to road salt and periodic inundation. They remove pollutants from runoff through filtration by soil media and uptake by plant roots, and reduce runoff volume through evapotranspiration. The practice provides aesthetic benefits and can easily be modified to fit a wide variety of space and drainage contexts, making it one of the more common LID practices for reducing runoff volumes and achieving groundwater recharge targets on development sites.

Bioretention can be designed with full, partial or no infiltration depending on the underlying soil permeability and objectives of the project. Partial infiltration systems with underdrains are recommended where the underlying native soil has a permeability of less than 15 mm/h. In areas with contaminated native soils, or high groundwater tables, the practice may be designed with no infiltration, in which case it would contribute to lower runoff volumes entirely through temporary storage and evapotranspiration.

3.1.1 Model Scenarios and Designs

Full infiltration

Bioretention areas designed for full infiltration do not have underdrains, and are installed where the native soils are relatively permeable (>15 mm/h). In the simple design used for costing (see Figure 3.1), runoff from a 2000 m² parking lot drains into a 130 m² system through curb inlets spaced 6 m apart with splash pads to dissipate the energy of the flowing water. The drainage area is roughly 15 times greater than the footprint of the facility, which is the maximum allowed in the *LID Guide*. Pre-treatment is provided through the splash pads and 75 mm mulch layer, which captures fine sediment and debris, and helps maintain the integrity of the filter media by preventing fines from migrating into the filter media. An overflow is provided to convey runoff from storms large enough to fill the system, which in this case would be equivalent to a 37 mm rain event. Two monitoring wells were added to facilitate inspection and eventual maintenance of the system.

Partial infiltration

The partial infiltration system shown in Figure 3.2 is similar to the full infiltration system, but includes a raised underdrain and granular storage reservoir, which increases the depth of the system from 1.28 in the full infiltration example, to just over 2 m. The depth of granular material

below the underdrain was sized to store and infiltrate runoff from a 25 mm event over the drainage area, not including moisture retention in the overlying soils. The additional granular material, underdrain and clean out pipes all add to the cost of this scenario relative to full infiltration.

No infiltration

The no infiltration design is the least common, and is implemented only where there are constraints to infiltration. The granular reservoir in the no infiltration model design is 40 cm shallower than the partial infiltration model design, and it includes an impermeable liner (Figure 3.3). It functions largely as a filtration system for water quality improvement, with some reduction of runoff through evapotranspiration by plants.

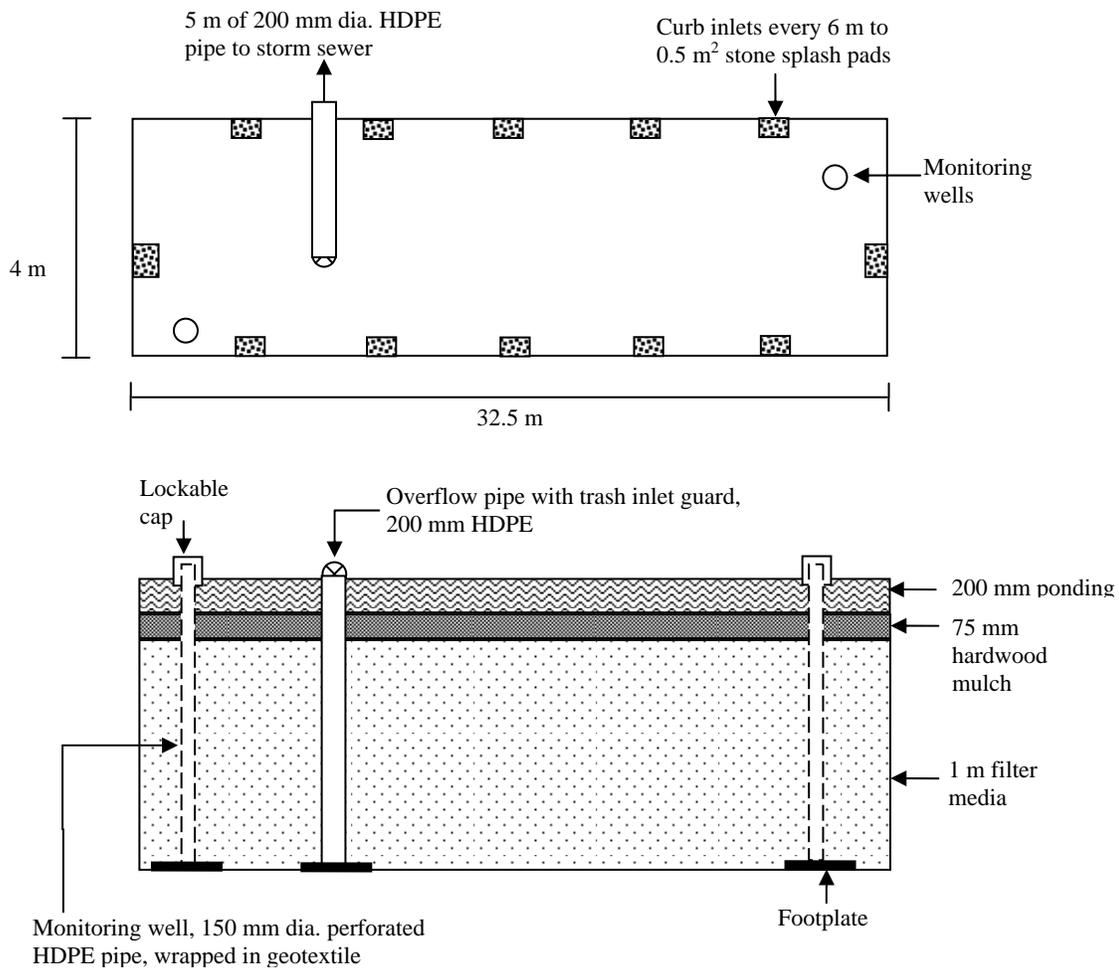


Figure 3.1: Bioretention full infiltration design. Plan view (top) and cross section (bottom)

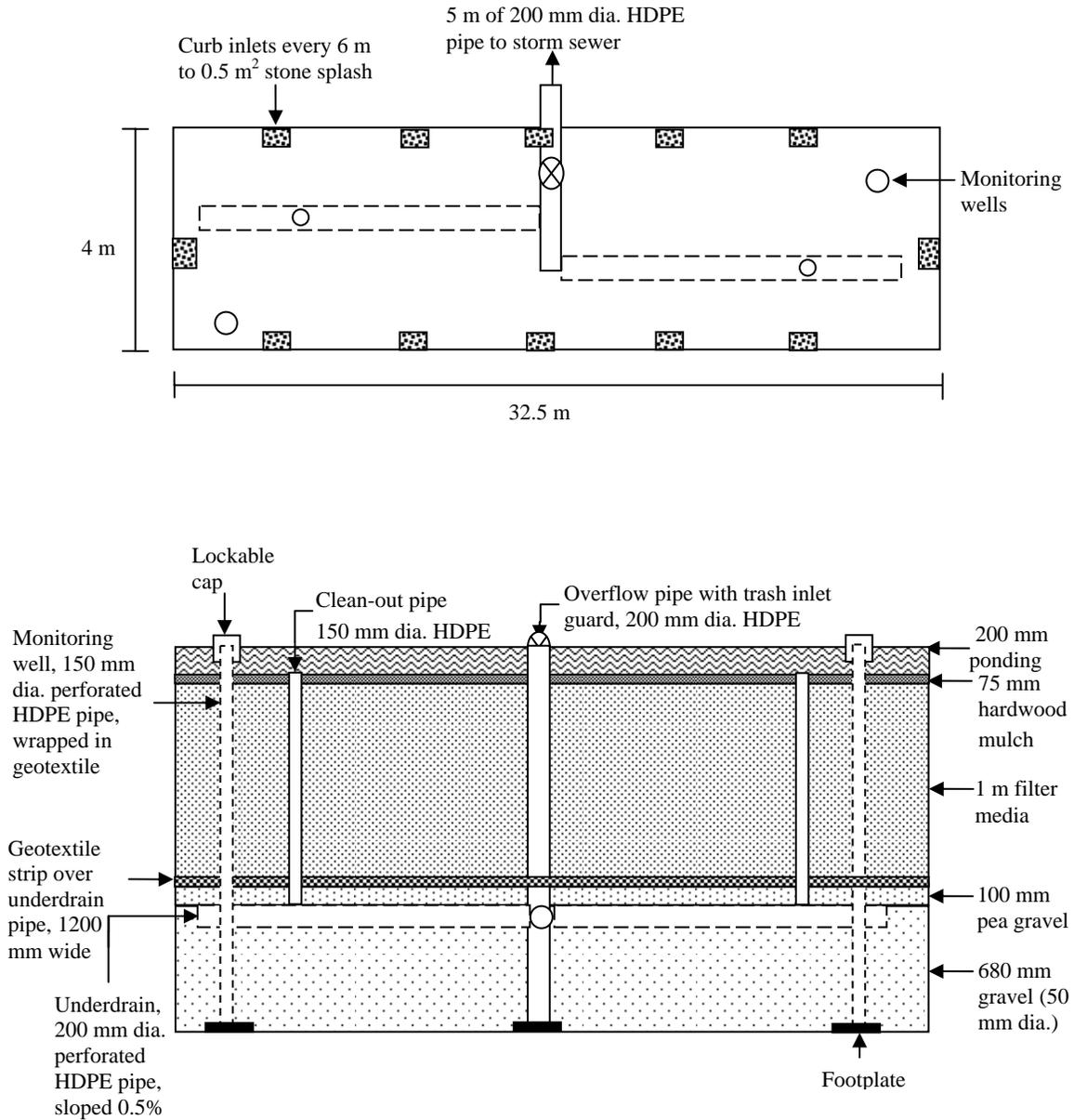


Figure 3.2: Bioretention partial infiltration design. Plan view (top); cross section (bottom)

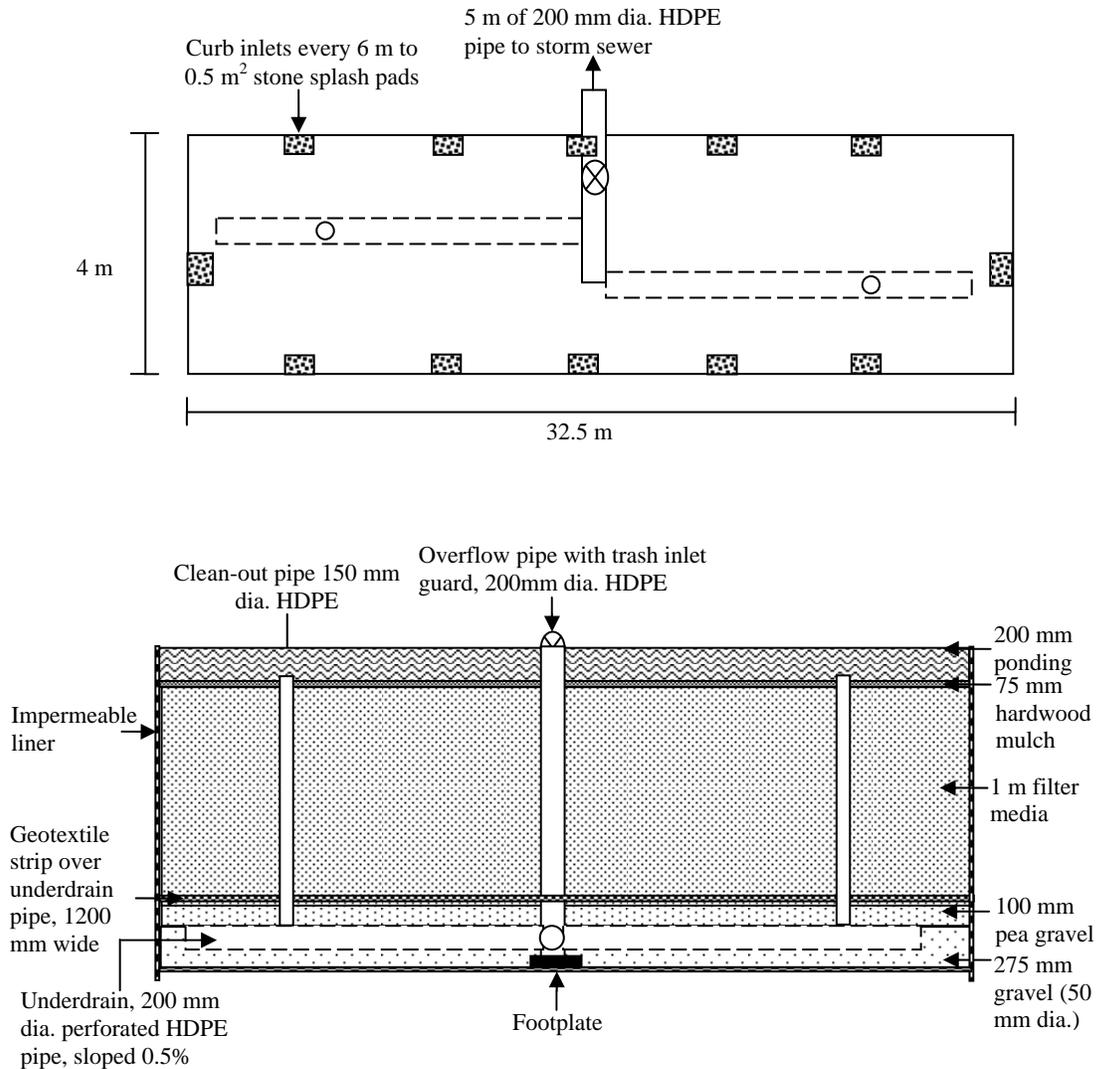


Figure 3.3: Bioretention no infiltration design. Plan view (top); cross section (bottom)

3.1.2 Capital Costs

The major capital cost categories for bioretention are shown in Table 3.1. A detailed breakdown of costs is provided in Appendix A. Full infiltration bioretention systems are considerably cheaper than partial or no infiltration designs because they do not require underdrains or granular storage reservoirs, and are shallower and therefore cheaper to excavate and construct. The no infiltration system has the highest material and installation costs because of the impermeable liner, but planning and site preparation is less expensive because there is no requirement for digging soil pits and infiltration testing which, when conducted according to specifications in the *LID Guide*, can account for over 8% of total costs in the other scenarios. In practice, soil infiltration capacity is often estimated more cheaply using soil texture and/or a more limited number of infiltration tests.

Table 3.1: Bioretention capital costs (130 m²)

Input Parameters	Full Infiltration	Partial Infiltration	No Infiltration
Planning & Site Preparation	\$6,652	\$7,955	\$4,048
Excavation	\$2,087	\$3,160	\$2,551
Materials & Installation	\$23,234	\$30,361	\$32,429
Total	\$31,973	\$41,476	\$39,028

3.1.3 Life Cycle Costs

As mentioned in the previous chapter, life cycle costs were calculated based on three different discount rates. Net present values based on a discount rate of 5% are shown in Table 3.2. There are few data on the operation and maintenance of bioretention areas because only recently have they started to become more widely implemented. However, it was assumed that if the bioretention area was routinely maintained, it would need major rehabilitation only once in 25 years, at a cost of roughly \$6345. This rehabilitation cost includes replacement of the filter media, re-mulching and replanting. Average costs of regular maintenance and landscaping are similar over the entire 50 year time period (\$945 to \$952). The exceptions are higher costs for watering and inspection in the early phases of plant establishment initially and after rehabilitation, and cleaning of underdrain pipes once every 10 years. Variation in present values is largely explained by differences in capital costs, as the maintenance and rehabilitation of the different scenarios was similar.

Table 3.2: Bioretention life cycle costs (130 m²)

	Full Infiltration	Partial Infiltration	No Infiltration
Input Parameters			
Life span	25 years	25 years	25 years
Capital cost	\$31,973	\$41,476	\$39,028
Rehabilitation cost at 25 years	\$7,504	\$7,504	\$7,504
Annual maintenance	\$945	\$952	\$952
Present Value including capital, maintenance and rehabilitation costs			
NPV at 50 years			
if i = 0 %	\$86,716	\$96,604	\$94,156
if i = 3 %	\$60,471	\$70,146	\$67,698
if i = 5 %	\$52,183	\$61,798	\$59,350
NPV at 25 years			
if i = 0 %	\$56,266	\$65,923	\$63,475
if i = 3 %	\$49,228	\$58,831	\$56,383
if i = 5 %	\$46,129	\$55,709	\$53,261

Note: i = discount rate

3.2 Permeable Pavement

Permeable pavements allow water to permeate through the surface or paver joints into a granular reservoir where water either infiltrates into the native soil and/or is released to a surface water body through a perforated underdrain. Various types of permeable pavements are available, including porous asphalt, pervious concrete, plastic grid pavers, and interlocking concrete permeable pavements (PICP). The PICP product was selected for costing in this project because it is currently the most common type used in Ontario, and the maintenance costs are well understood. As with bioretention, these pavements can be designed for full, partial or no infiltration and have been used to treat stormwater draining from an impervious pavement. In the scenarios described below, it is assumed that a 60 m x 16.7 m impermeable asphalt drains onto an equal sized area of permeable pavers. A concrete curb extending to the native soil separates the two types of pavements.

3.2.1 Model Scenarios and Designs

Full infiltration

The pavement can be designed for full infiltration if the underlying subsoil has a permeability of 15 mm/h or greater (after compaction). The base granular reservoir without underdrains is 350 mm deep, and is capable of storing runoff from a 61 mm rain event over the catchment area (Figure 3.4). Plastic edge restraints are used to prevent slumping of pavers along the edges and a monitoring well is included for inspection purposes.

Partial infiltration

A partial infiltration system is used where the post compaction permeability of the native subsoil is less than 15 mm/h. The system has the same depth as the full infiltration system, but an underdrain is included to ensure full drainage between rain events (Figure 3.5). The perforated pipe in this case is raised roughly 50 mm above the native subsoil to allow for some infiltration. Since the depth below the underdrain is only capable of storing runoff from a 9 mm event, a flow restrictor is sometimes included to retain water in the base above the perforated underdrain, and thereby promote greater infiltration. Since these restrictors are optional and relatively inexpensive, the cost of this feature has not been included.

No infiltration

No infiltration systems are applied when infiltration is not desirable. In this case, the pavement structure would help to filter contaminants but runoff would not be reduced. The primary additional feature is the impermeable liner that surrounds the pavement base and sides (Figure 3.6)

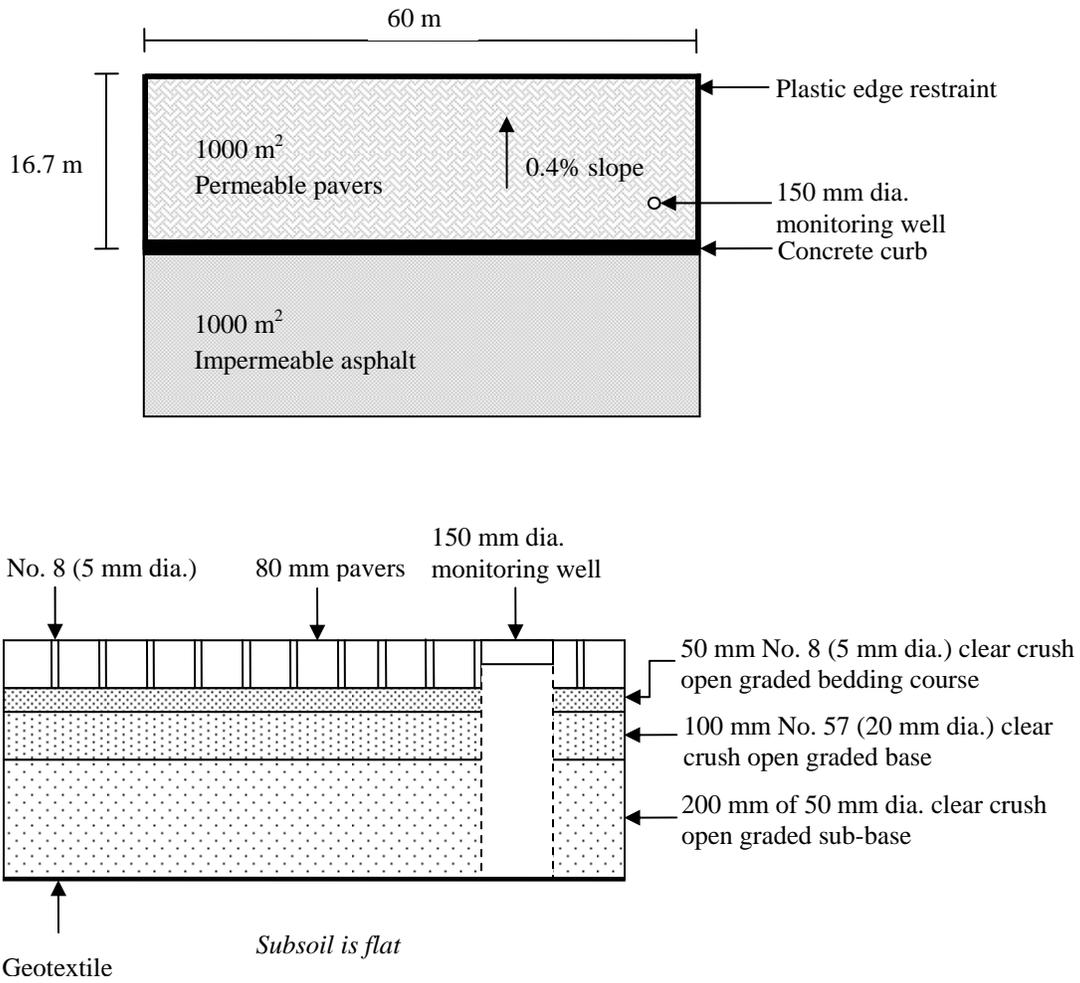


Figure 3.4: Permeable pavement full infiltration design. Plan view (top); cross section (bottom)

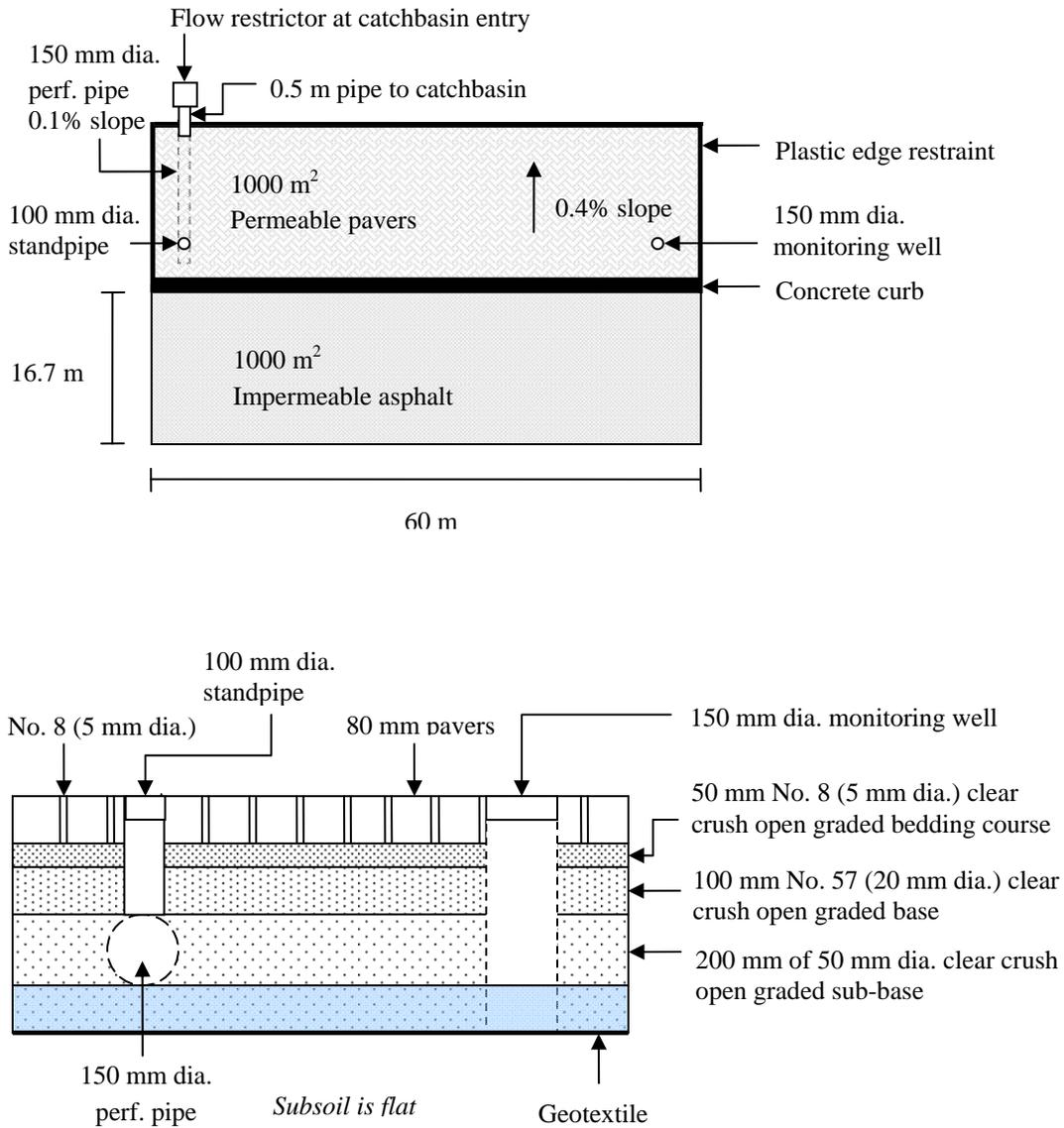


Figure 3.5: Permeable pavement partial infiltration design. Plan view (top); cross section (bottom)

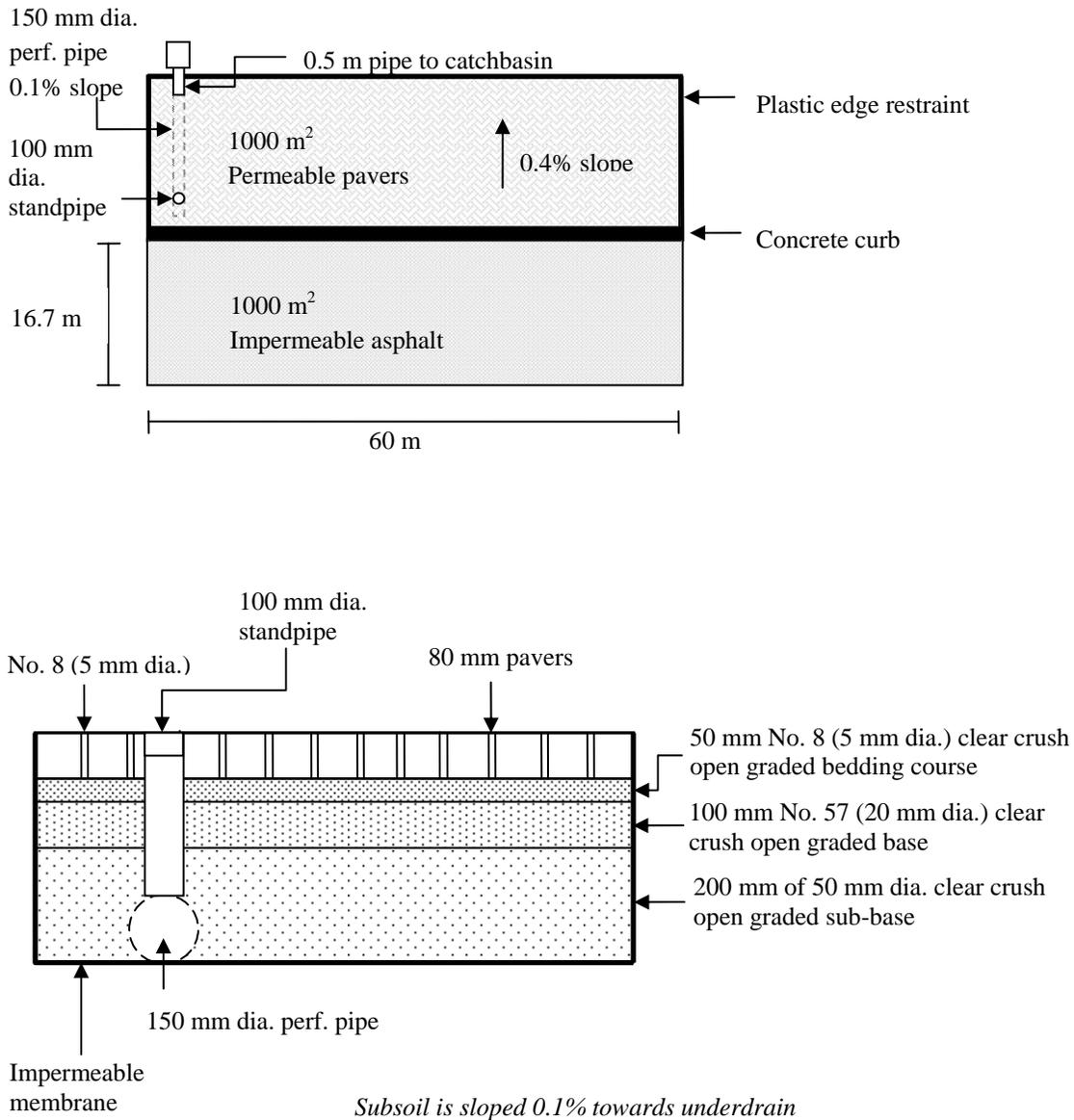


Figure 3.6: Permeable pavement no infiltration design. Plan view (top); cross section (bottom)

3.2.2 Capital Costs

General cost categories and totals for the three permeable pavement designs and a conventional asphalt (for comparison) are presented in Table 3.3. Detailed costs are provided in Appendix A. The presence of an underdrain made little difference in the overall cost. However, the addition of the impermeable liner in the ‘no infiltration’ scenario increased the cost considerably, even though test pits and infiltration measurements were not required (Table 3.3).

Table 3.3: Permeable pavement and conventional asphalt capital costs (1000 m²)

Input Parameters	Permeable Interlocking Concrete Pavements			Asphalt
	Full Infiltration	Partial Infiltration	No Infiltration	
Planning & Site Preparation	\$12,537	\$12,659	\$10,514	\$4,714
Excavation	\$5,584	\$5,584	\$5,584	\$4,870
Materials & Installation	\$80,192	\$81,409	\$94,055	\$36,769
Total	\$98,313	\$99,652	\$110,153	\$46,353

The asphalt was assumed to be 50 mm thick and constructed over a 300 mm crusher run granular base. The total cost of asphalt was just less than half the price of permeable pavement for an equivalent area.

At this cost, the entire parking lot with 1000 m² of asphalt draining onto 1000 m² of a partial infiltration permeable pavement would be roughly \$146,000. By comparison, the cost of a parking lot with a partial infiltration bioretention system and an asphalt drainage area would be \$134,182 (2000 m² of asphalt + 130 m² bioretention). Although the capital cost of the bioretention stormwater control system is lower, the system requires 130 m² of additional space.

3.2.3 Life Cycle Costs

The life cycle costs for permeable pavements and asphalt are presented in Table 3.4. The paver costs are based on the assumption that the pavers would need to be replaced in 30 years, and that annual inspections, replacement of selected pavers, and periodic cleaning would cost on average \$433 to \$436. The cost of replacement is less than the initial installation cost because the base granular materials can be largely re-used, and there are no excavation costs. The asphalt costs assume a 25 year life cycle assuming it is well maintained, with annual patching and crack sealing costs of \$1000 and seal coating every three years at a cost of \$3580. Asphalt pavements that are not maintained in this fashion would have a shorter life.

At the 25 year period of evaluation, neither the permeable pavement nor asphalt would have been replaced. Higher permeable pavement present value costs over this time period largely reflect the higher initial capital costs. The present value cost differences narrow considerably over the 50 year evaluation period as the higher asphalt maintenance costs accumulate, particularly at low discount rates.

Table 3.4: Permeable pavement and conventional asphalt life cycle costs (1000 m²)

Input Parameters	Permeable Interlocking Concrete Pavements			Asphalt
	Full Infiltration	Partial Infiltration	No Infiltration	
Life span	30 years	30 years	30 years	25 years
Capital cost	\$98,313	\$99,652	\$110,153	\$46,353
Replacement cost at 30 years (25 years for asphalt)	\$72,990	\$7,990	\$72,990	\$26,951
Annual maintenance	\$433	\$436	\$436	\$2,146
Present Value including capital, maintenance and rehabilitation costs				
NPV at 50 years				
if i = 0 %	\$192,970	\$194,462	\$204,963	\$180,584
if i = 3 %	\$139,552	\$140,968	\$151,469	\$113,887
if i = 5 %	\$123,081	\$124,472	\$134,973	\$92,812
NPV at 25 years				
if i = 0 %	\$109,146	\$110,562	\$121,063	\$99,993
if i = 3 %	\$105,796	\$107,185	\$117,686	\$83,382
if i = 5 %	\$104,325	\$105,703	\$116,204	\$76,117

Note: i = discount rate

3.3 Infiltration Trenches

Infiltration trenches consist of rectangular excavations filled with clean stone granular material. Runoff from the road or roof enters the system through a perforated pipe that conveys water to the trench where it can infiltrate into the subsoil. Pretreatment is required for road runoff. Unlike permeable pavement and bioretention, infiltration trenches and chambers are typically located under paved or landscaped areas. These practices are often used in tight spaces where surface areas are either not available or are designated for other uses.

3.3.1 Model Scenarios and Designs

Infiltration trenches are often designed similarly on low and high permeability soils because runoff is controlled at the entry point to the system, typically via a weir in a manhole or concrete chamber, allowing water to bypass the system when the trench or chamber system is full. Thus, the scenarios in this case do not include partial, full and no infiltration, but are instead divided according to the type of runoff received. Relatively clean runoff from roofs require considerably less pretreatment than runoff from roads. The addition of pre-treatment devices for road drainage can add considerably to the cost of the system.

Roof Runoff

In this scenario, runoff drains into a 2 x 51 m trench via a control manhole from a 2000 m² industrial or commercial roof (Figure 3.7). The footprint of the facility is approximately 1/20th the

size of the roof. The system is 1.62 m deep (Figure 3.8) with the capacity to store runoff from a 29 mm rain event. Additional storage is available in the contributing storm sewer pipes. The invert of the overflow is located at 1.2 m below the surface to protect against frost. Other than a sump in the manhole, which allows for some settling of larger solids, there is no pre-treatment. Monitoring wells are provided to facilitate inspections.

Road and Roof Runoff

This scenario is identical to the previous one, but the drainage area consists of roof (500 m²) and road runoff (1500 m²), with pretreatment via a hydrodynamic separator for the road runoff portion (Figure 3.9 and 3.10). The roof runoff portion flows directly to the control manhole without pre-treatment. If the road and roof runoff were combined in the same sewer, the hydrodynamic separator would need to be larger.

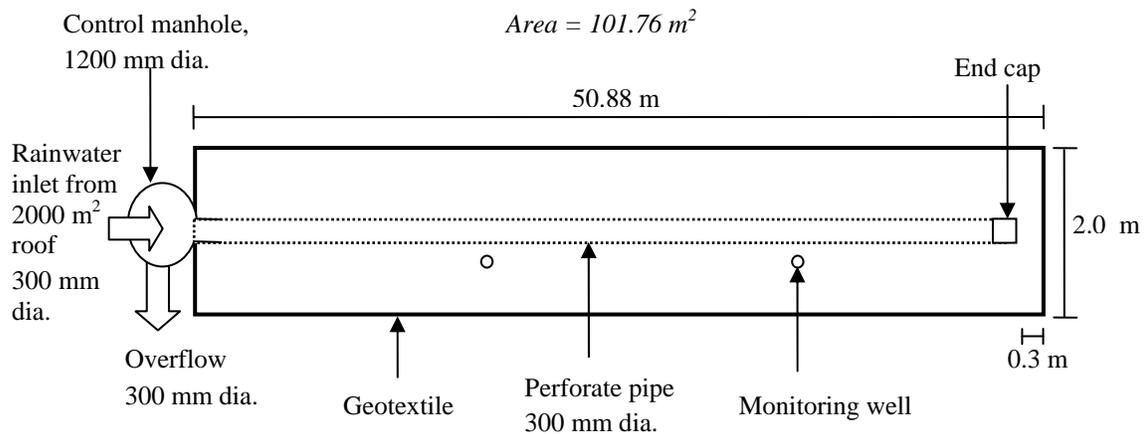


Figure 3.7: Plan view of the infiltration trench receiving roof runoff only

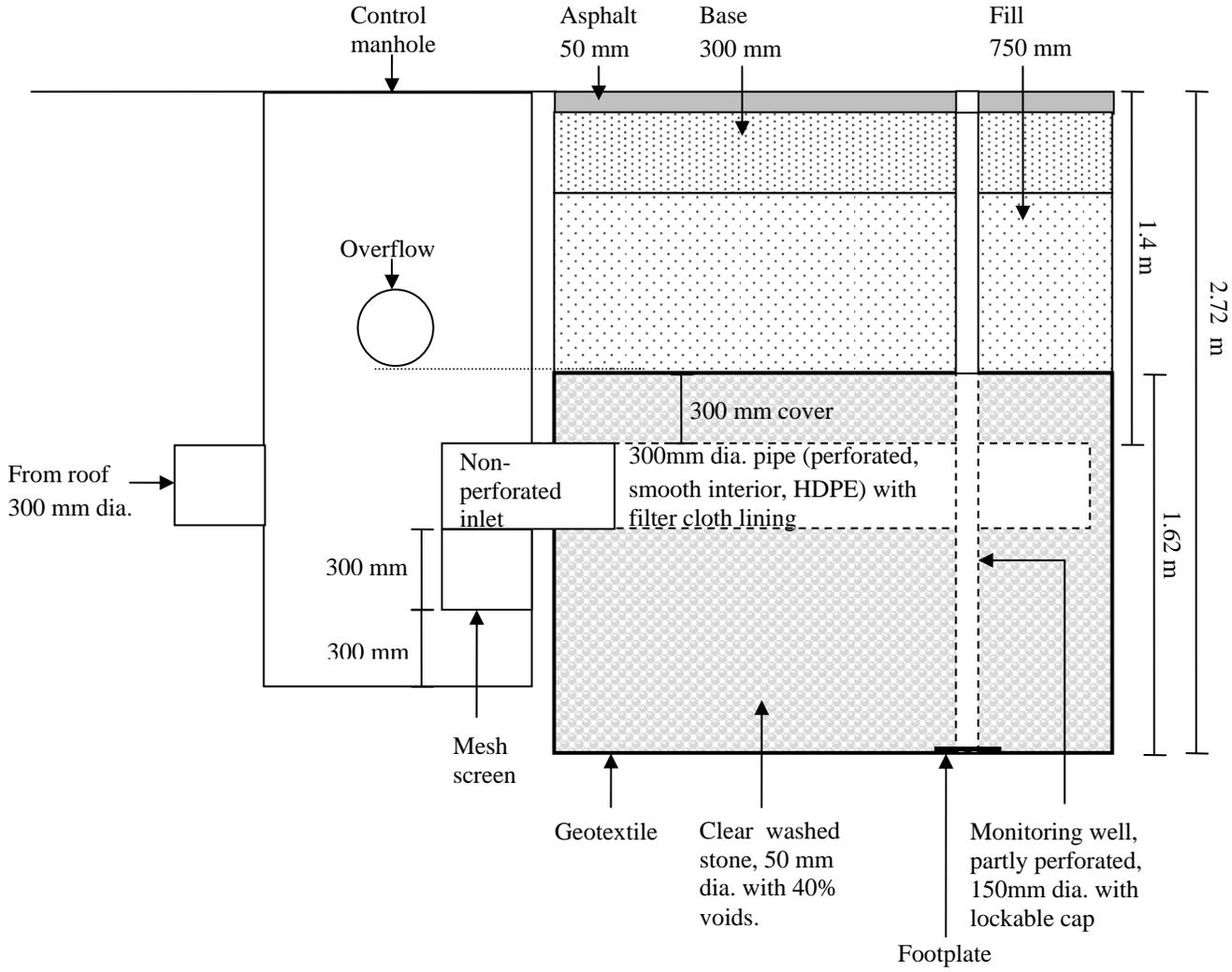


Figure 3.8: Cross section of infiltration trench receiving roof runoff only.

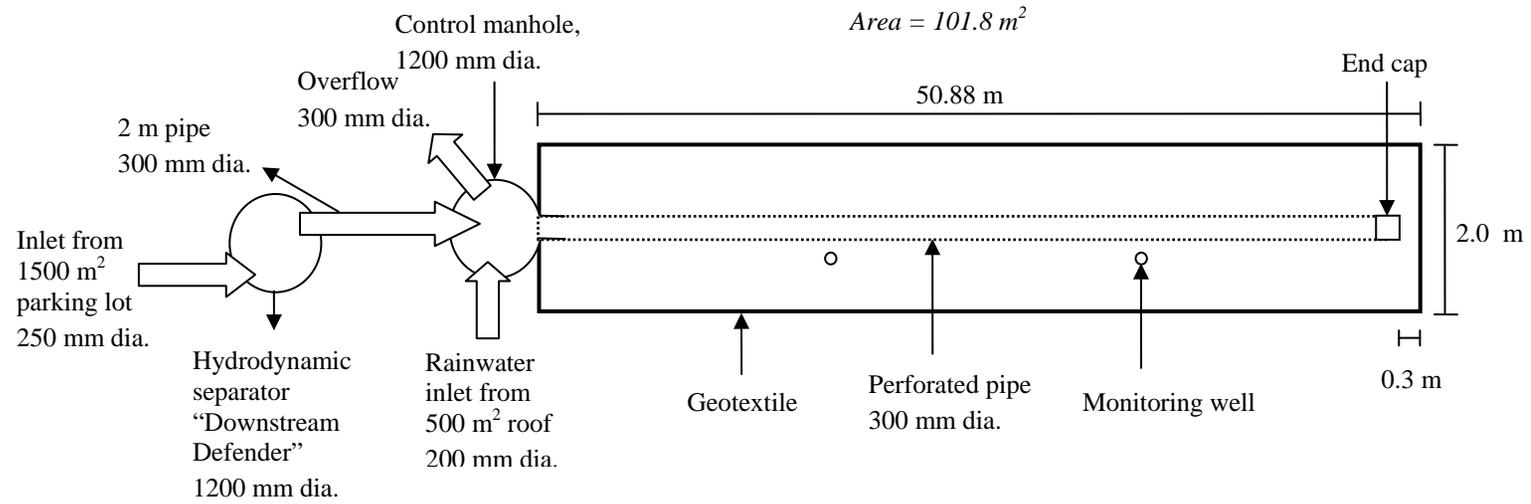


Figure 3.9: Plan view of the infiltration trench receiving road (1500 m²) and roof runoff (500 m²)

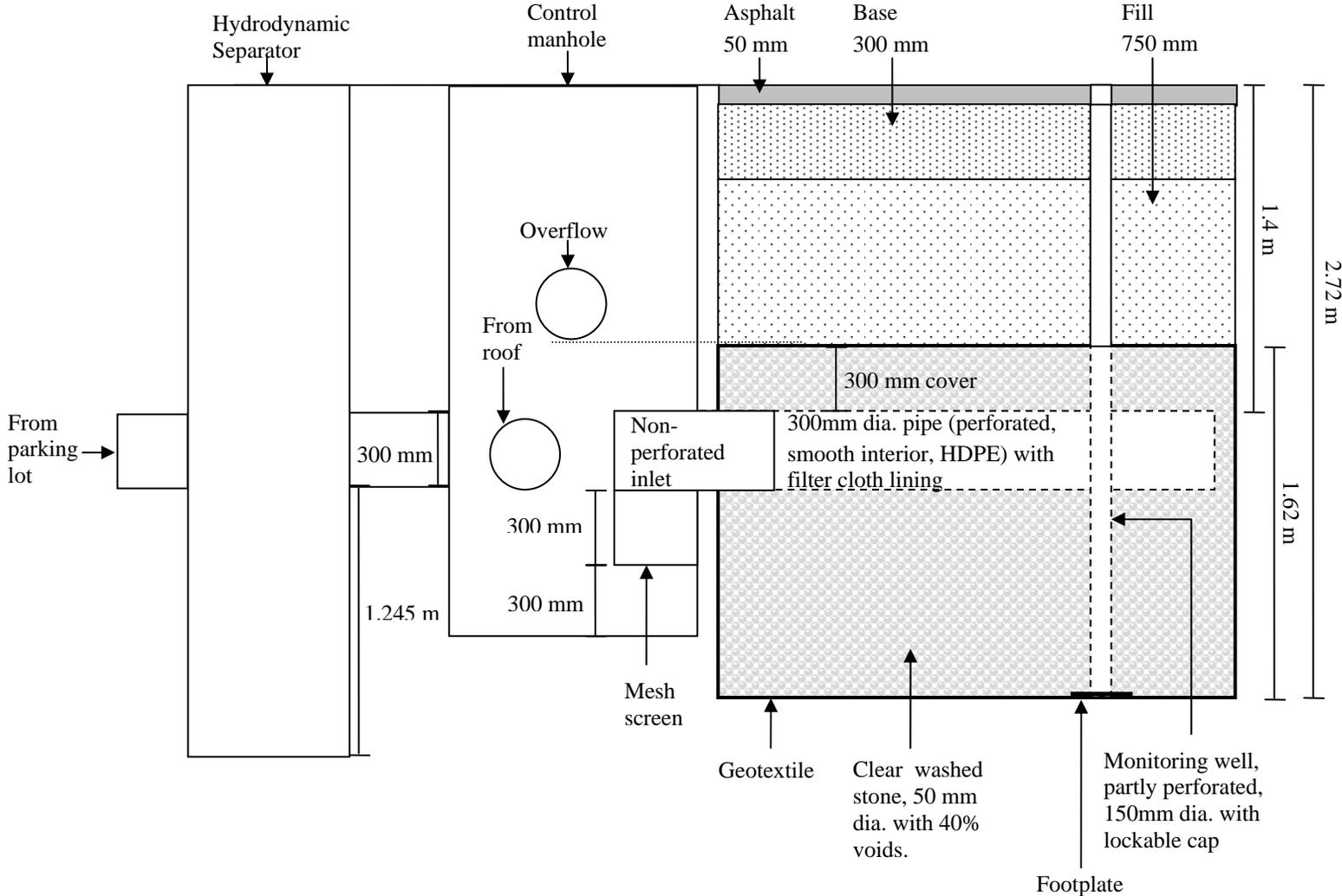


Figure 3.10: Cross section of the infiltration trench receiving road (1500 m²) and roof runoff (500 m²)

3.3.2 Capital Costs

The capital costs presented in Table 3.5 and Appendix A shows the road runoff scenario with pretreatment to be 63% more expensive than the roof runoff scenario due to the requirement for expensive pre-treatment via an Oil Grit Separator in the road runoff scenario. These results indicate that if only a portion of the runoff from a site is infiltrated, it is clearly cheaper to prioritize roof runoff for this purpose.

Table 3.5: Infiltration trench capital costs

Input Parameters	Roof Only	Road & Roof
Planning & Site Preparation	\$7,436	\$9,068
Excavation	\$2,642	\$2,642
Materials & Installation	\$17,498	\$33,824
Total	\$27,575	\$45,534

3.3.3 Life Cycle Costs

Studies have shown that infiltration trenches can continue to function well over long time periods (e.g. JF Sabourin and Associates, 2008). Hence it was assumed that, with adequate maintenance, replacement or major rehabilitation would not be required over the 50 year evaluation period. The road runoff scenario was considerably more expensive to maintain than the roof runoff scenario because the hydrodynamic separator requires regular inspections and vacuum removal of sediments. Also, the inner filter cloth held in place by expandable rings would need to be pulled out and changed every 8 years. Incorporating these higher maintenance costs increased the long term cost of the road runoff scenario to a 50 year present value equal to more than double that of the roof runoff scenario.

Table 3.6: Infiltration trench life cycle costs

Input Parameters	Roof Only	Road & Roof
Life span	50+ years	50+ years
Capital cost	\$27,575	\$45,534
Replacement cost	n/a	n/a
Annual maintenance	\$74	\$1,277
Present Value including capital, maintenance and rehabilitation costs		
NPV at 50 years		
if i = 0 %	\$31,250	\$109,384
if i = 3 %	\$29,432	\$77,810
if i = 5 %	\$28,873	\$68,090
NPV at 25 years		
if i = 0 %	\$29,375	\$77,134
if i = 3 %	\$28,808	\$67,127
if i = 5 %	\$28,561	\$62,760

Note: i = discount rate

3.4 Infiltration Chambers

A number of proprietary manufactured modular chambers are available as an alternative to infiltration trenches. These large open perforated structures create temporary storage of stormwater for infiltration (Figure 3.11). The chamber sections can be installed individually or in series in large trench formations. Since the chambers are empty, they are able to store more water than a stone filled trench over the same area.

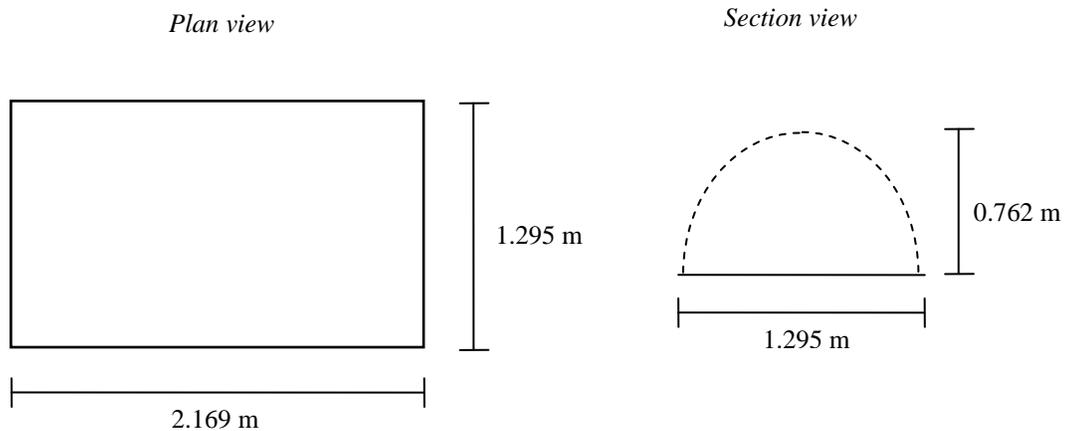


Figure 3.11: Sections of corrugated wall chambers

3.4.1 Model Scenarios and Designs

The two model scenarios are similar to those described earlier for infiltration trenches. The first scenario is for roof runoff, the second for a combination of roof and road runoff. As with trenches, the primary difference between the scenarios is the need for pretreatment in the road runoff scenario, which is accomplished by using an appropriately sized hydrodynamic separator.

Roof runoff

The footprint of the chamber is similar to the trench discussed in the previous section (1/20th of the drainage area), but the depth is 0.55 m shallower (Figure 3.12) because the empty chambers have the capacity to store a larger volume of water. Even with the shallower depth, however, the chambers have the capacity to store roughly one third more stormwater than the trench. The control manhole with weir is designed the same way as the trench to permit direct comparisons between the two practices.

Road and roof runoff

As with trenches, the drainage area is comprised of 25% roof and 75% road. A hydrodynamic separator is included to provide pre-treatment for the road runoff portion. Roof runoff is directed to the control manhole in the same manner as the previous scenario, since the cleaner roof water requires less pre-treatment.

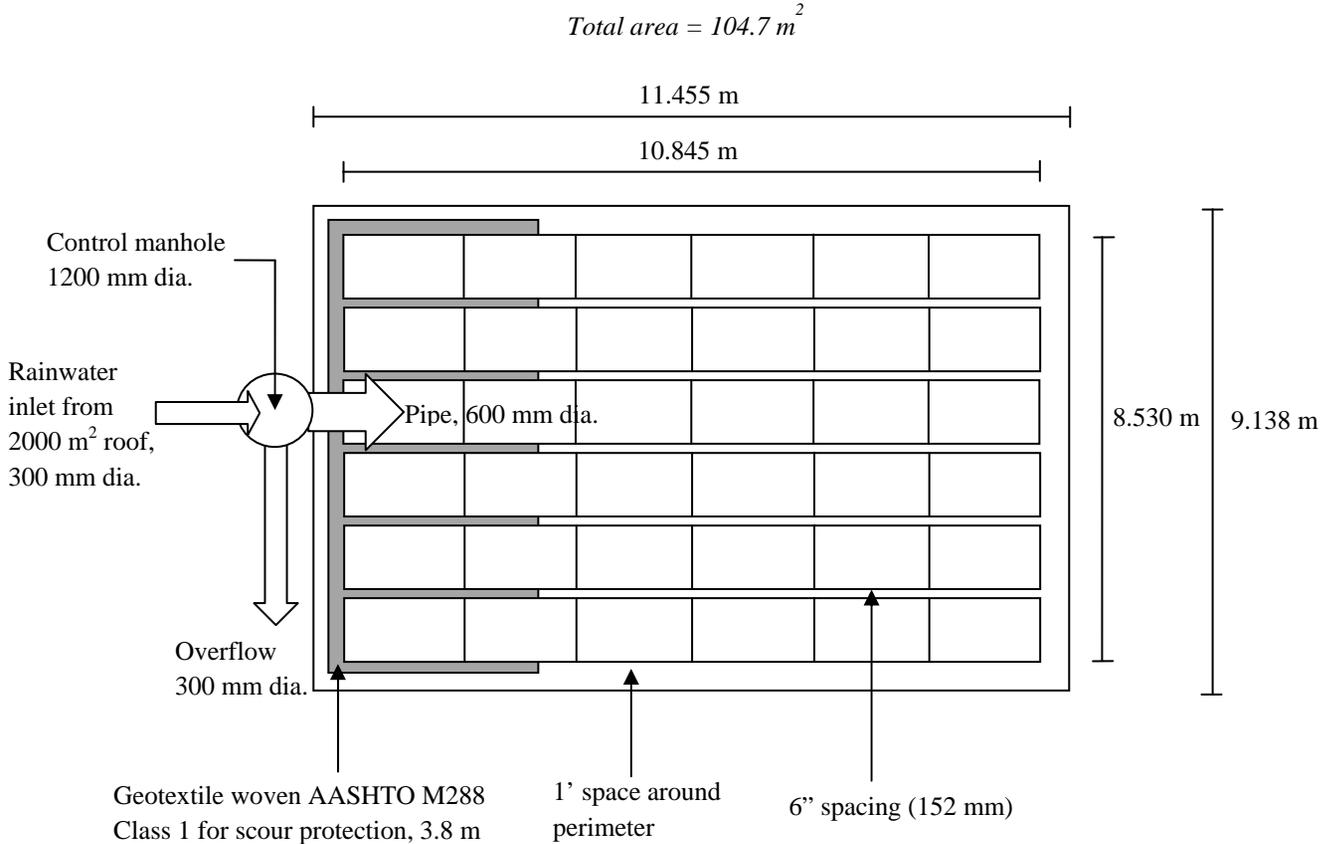


Figure 3.12: Plan view of infiltration chambers receiving roof runoff only

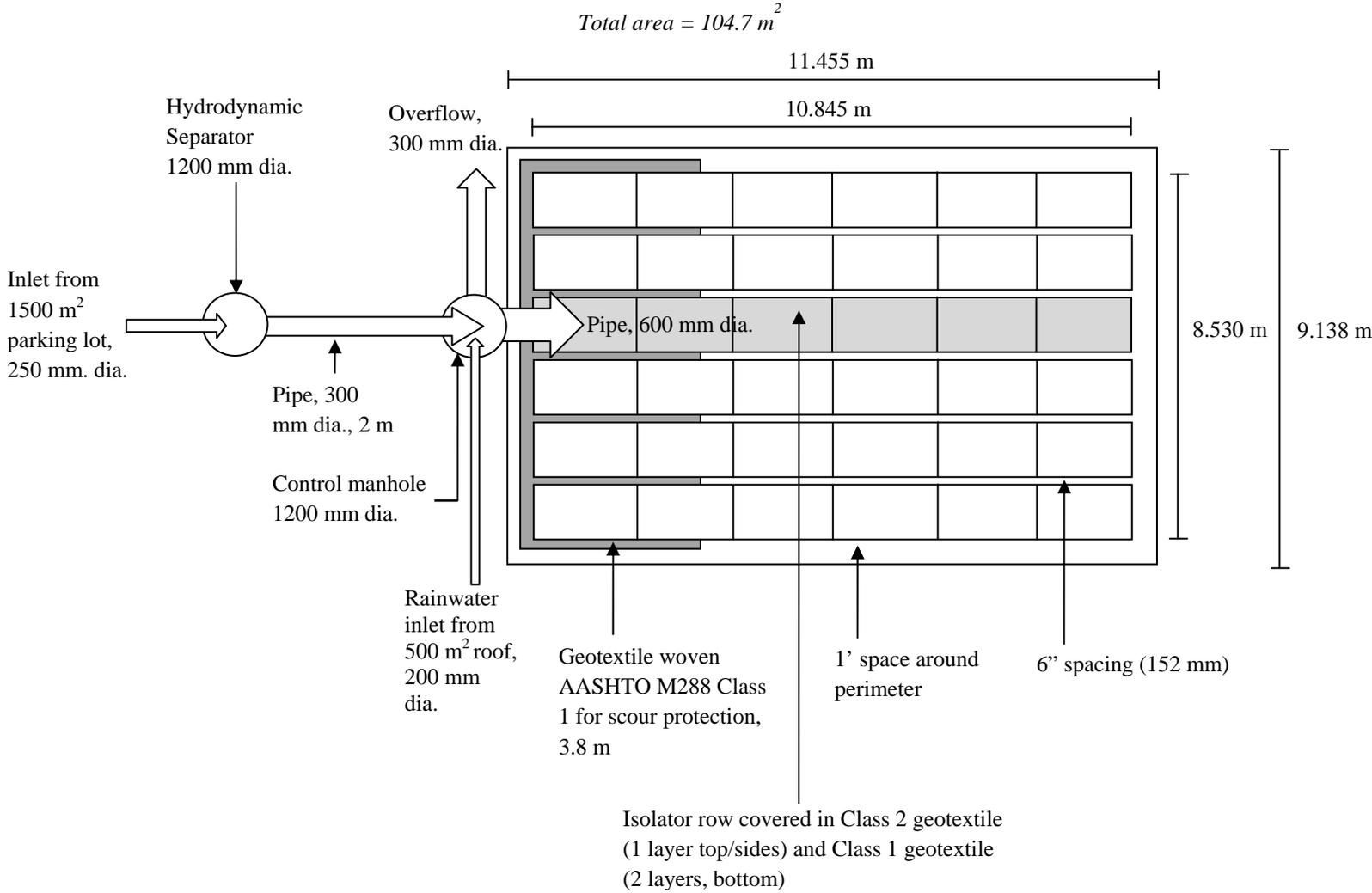


Figure 3.13: Plan view for infiltration chambers receiving road (1500 m²) and roof runoff (500 m²)

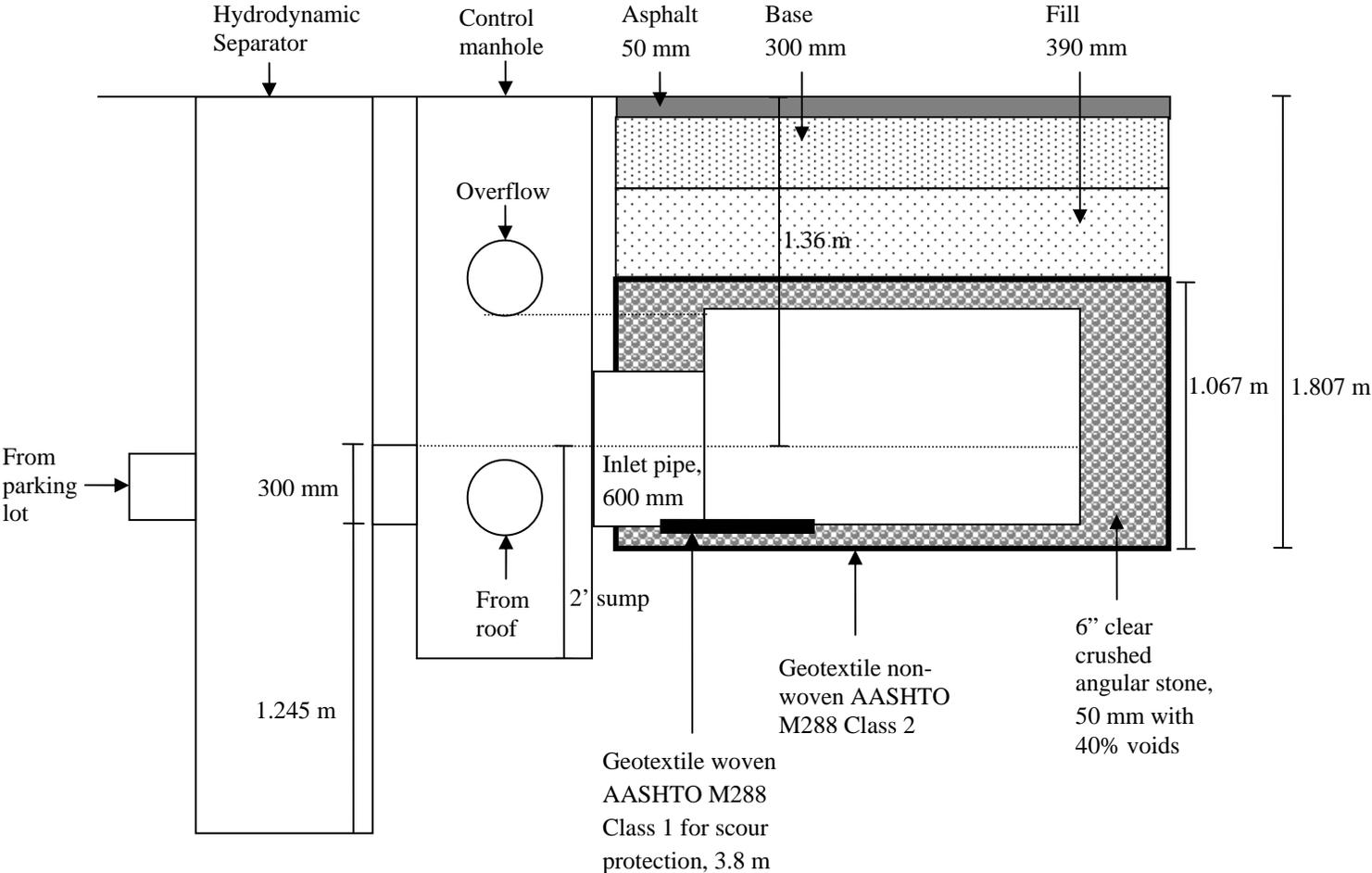


Figure 3.14: Cross section of infiltration chambers receiving road (1500 m²) and roof runoff (500 m²)

3.4.2 Capital Costs

The capital cost of the road/roof runoff scenario was 70% more than that of the roof runoff scenario because the former required expensive pre-treatment via a hydrodynamic separator (Figure 3.13). Chamber materials are more expensive than clear stone, but savings on perforated pipes and other installation expenses resulted in the two practices having very similar material and installation costs. Overall, the infiltration chamber costs were only slightly higher than the infiltration trench costs discussed in the previous section. The benefit of chambers, however, is that these provide considerably more storage per unit area than a simple gravel filled trench.

Table 3.7: Infiltration chambers capital costs

Input Parameters	Roof Only	Road & Roof
Planning & Site Preparation	\$5,723	\$7,373
Excavation	\$2,141	\$2,141
Materials and Installation	\$17,683	\$34,192
Total	\$25,547	\$43,706

3.4.3 Life Cycle Costs

The underground chambers were expected to last at least 50 years if they were adequately maintained, and therefore replacement costs were not applied. Costs of maintenance were very low for the roof runoff scenario because maintenance activities were limited to cleaning out the control manhole once per year. The hydrodynamic separators in the road/roof runoff scenario required inspection, cleanout and sediment disposal, which resulted in an average annual maintenance cost of \$1,212. Overall net present value costs for the road/roof runoff scenario were well over double that of the roof runoff scenario. Maintenance costs for the trenches and chamber system were expected to be the same, hence NPV differences between the two practices were a result of differences in the initial capital cost alone.

Table 3.8: Infiltration chambers life cycle costs

	Roof Runoff	Road and Roof Runoff
Input Parameters		
Life span	50+ years	50+ years
Capital cost	\$25,547	\$43,706
Replacement cost	na	na
Annual maintenance	\$74	\$1,212
Present Value including capital, maintenance and rehabilitation costs		
NPV at 50 years		
if i = 0 %	\$29,222	\$104,306
if i = 3 %	\$27,404	\$74,269
if i = 5 %	\$26,845	\$65,038
NPV at 25 years		
if i = 0 %	\$27,347	\$73,406
if i = 3 %	\$26,780	\$64,008
if i = 5 %	\$26,533	\$59,909

Note: i = discount rate

3.5 Enhanced Grass Swales

Enhanced swales are designed to detain, infiltrate and convey flows to the storm sewer system or directly to the receiving water. Check dams help slow and filter water to enhance sedimentation, soil infiltration and evapotranspiration by plants and/or grasses. Unlike dry swales, they do not incorporate an engineered soil media mix and optional underdrain. Therefore, this practice does not usually provide the same runoff reduction and water quality benefits as a dry swale or bioretention system.

Swales and open channels are often used adjacent to roadways. They can also be used along the perimeter of parking lots and other impervious drainage areas. Swales can be planted with grass or other herbaceous plants, with rainwater entering either through curb cuts or as sheet flows.

3.5.1 Model Design and Scenarios

In the model scenario, runoff enters the swale as sheetflow through curb cut inlets. The swale is planted with grass and check dams are provided at 30 m intervals. Check dams can be made of different materials. The cost of three options were evaluated – concrete curbs, compost filter socks, and rocks. The swale footprint is one tenth the size of the drainage area. Culverts are used to convey water below driveways or sidewalks, and a culvert at the downstream end of the swale conveys water to the conventional sewer system.

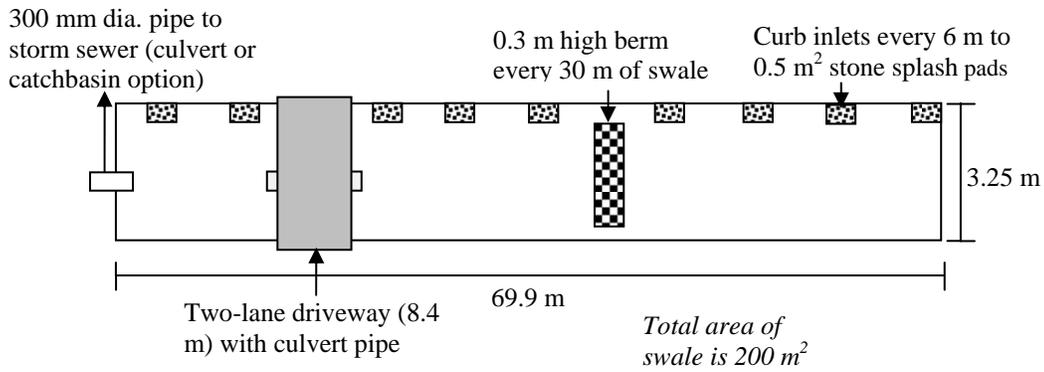


Figure 3.15: Plan view of enhanced grass swale. Drainage area is 2000 m²

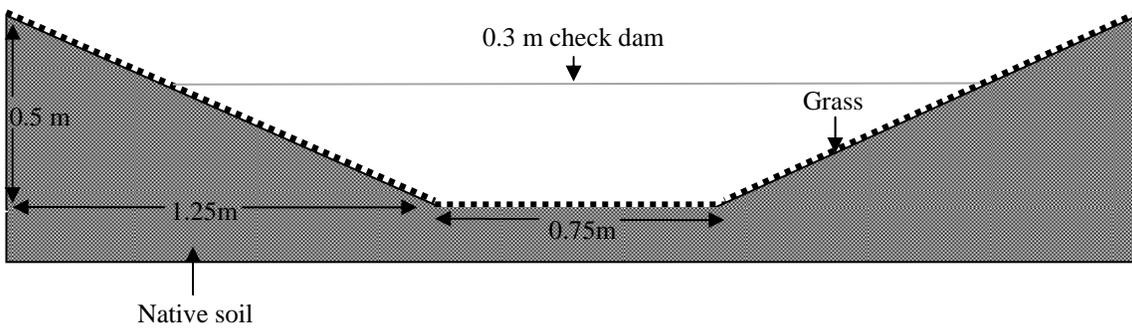


Figure 3.16: Cross section of enhanced grass swale

3.5.2 Capital Costs

Enhanced swales are one of the least expensive stormwater practices because they do not require significant excavation, and include pipes only at driveway or road crossings, and at the downstream connection to the storm sewer system. The curbs and curb cuts added significantly to the overall cost (see Appendix A). These are not necessary in swale designs where runoff enters the swale as sheet flow across its full length. Parking wheel stops or bollards can be used to prevent vehicle damage to the swale. Removal of the curbs and gutters from the model design would save approximately \$5500. There was only a minor difference in cost between the different check dam options.

Table 3.9: Enhanced grass swale capital costs

Input Parameters	Curb check dam	Filter sock check dam	Rock check dam
Planning & Site Preparation	\$5,726	\$5,694	\$5,705
Excavation	\$1,455	\$1,455	\$1,455
Materials and Installation	\$11,401	\$11,084	\$11,187
Total	\$18,582	\$18,233	\$18,347

3.5.3 Life Cycle Costs

Maintenance of enhanced swales consists of regular inspections, watering, litter and sediment removal, and mowing. Grass may also need to be restored periodically. These routine costs add significantly to the overall long term costs, but the practice remains one of the least expensive LID practices evaluated in this study.

Table 3.10: Enhanced grass swale life cycle costs

	Curb check dam	Filter sock check dam	Rock check dam
Input Parameters			
Life span	50+ years	50+ years	50+ year
Capital cost	\$18,582	\$18,233	\$18,347
Replacement cost	n/a	n/a	n/a
Annual maintenance	\$500	\$500	\$500
Present Value including capital, maintenance and rehabilitation costs			
NPV at 50 years			
if i = 0 %	\$43,567	\$43,218	\$43,333
if i = 3 %	\$32,351	\$32,003	\$32,117
if i = 5 %	\$28,874	\$28,525	\$28,639
NPV at 25 years			
if i = 0 %	\$32,011	\$31,662	\$31,777
if i = 3 %	\$28,505	\$28,156	\$28,270
if i = 5 %	\$26,947	\$26,598	\$26,712

Note: i = discount rate

3.6 Rainwater Harvesting

The term *Rainwater Harvesting* (RWH) refers to the ancient practice of collecting rainwater from roofs or other impermeable surfaces for future use in satisfying daily water needs. A RWH system typically consists of three basic elements: the collection system (such as a roof), the conveyance system (infrastructure that transports the water), and the storage system (above or below ground cistern); however in larger systems or ones designed to produce potable water, a pressurized or non-pressurized water discharge system and pre/post treatment unit is usually included. In most cases, a cistern overflow draining to an infiltration basin or municipal sewer system is necessary in order to prevent system backups.

3.6.1 Model Scenarios and Designs

The RWH scenarios selected for detailed costing are applicable to large commercial, industrial or institutional contexts, which are currently the most common type of system installed within the Greater Toronto Area. Both scenarios were developed using a RWH sizing and costing tool developed in 2009 by the University of Guelph, Connect the Drops and TRCA to facilitate wider

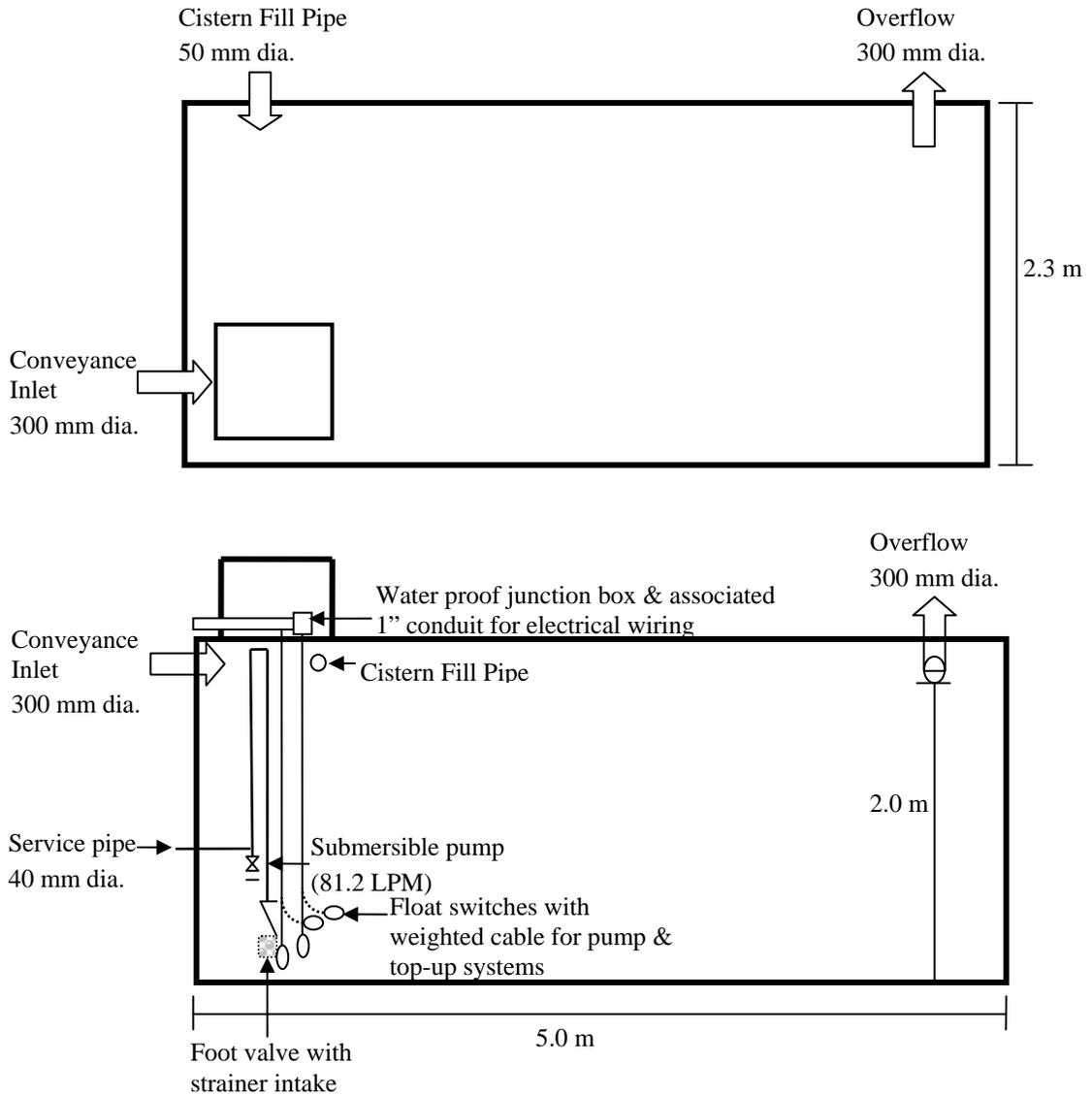
adoption of RWH systems in Ontario (STEP, 2011). The tool uses RS Means databases for costing and optimal cistern sizing based on local rainfall data for the GTA and recommendations provided in recent guidelines on RWH in Ontario.

Concrete cistern outside

The first scenario consists of a 23,000 L concrete cistern buried adjacent to the building with dual plumbing distribution, an 81.2 LPM submersible pump, and a 439 L expansion tank. The system also includes a float switch to prevent the pump from dry running, a top-up float switch and associated wiring, a solenoid valve, air gap to prevent backflow, as well as backflow preventer at the premise boundary, a water meter and water hammer arrestor. In the portion of the building using the rainwater cistern for toilet flushing there were 260 occupants and two hose bibs were used on average 14 minutes per day from April to September.

Plastic tank inside

The plastic tank is also 23,000 L, but is stored inside the building. It was costed out volumetrically and therefore could consist of one large unit or several smaller units, depending on space constraints. Many of the same features in the concrete cistern case would apply here as well, but since the cistern is inside, there would be no need for excavation.



Total Volume = 23,000 L

Figure 3.17: Pre-cast concrete tank design

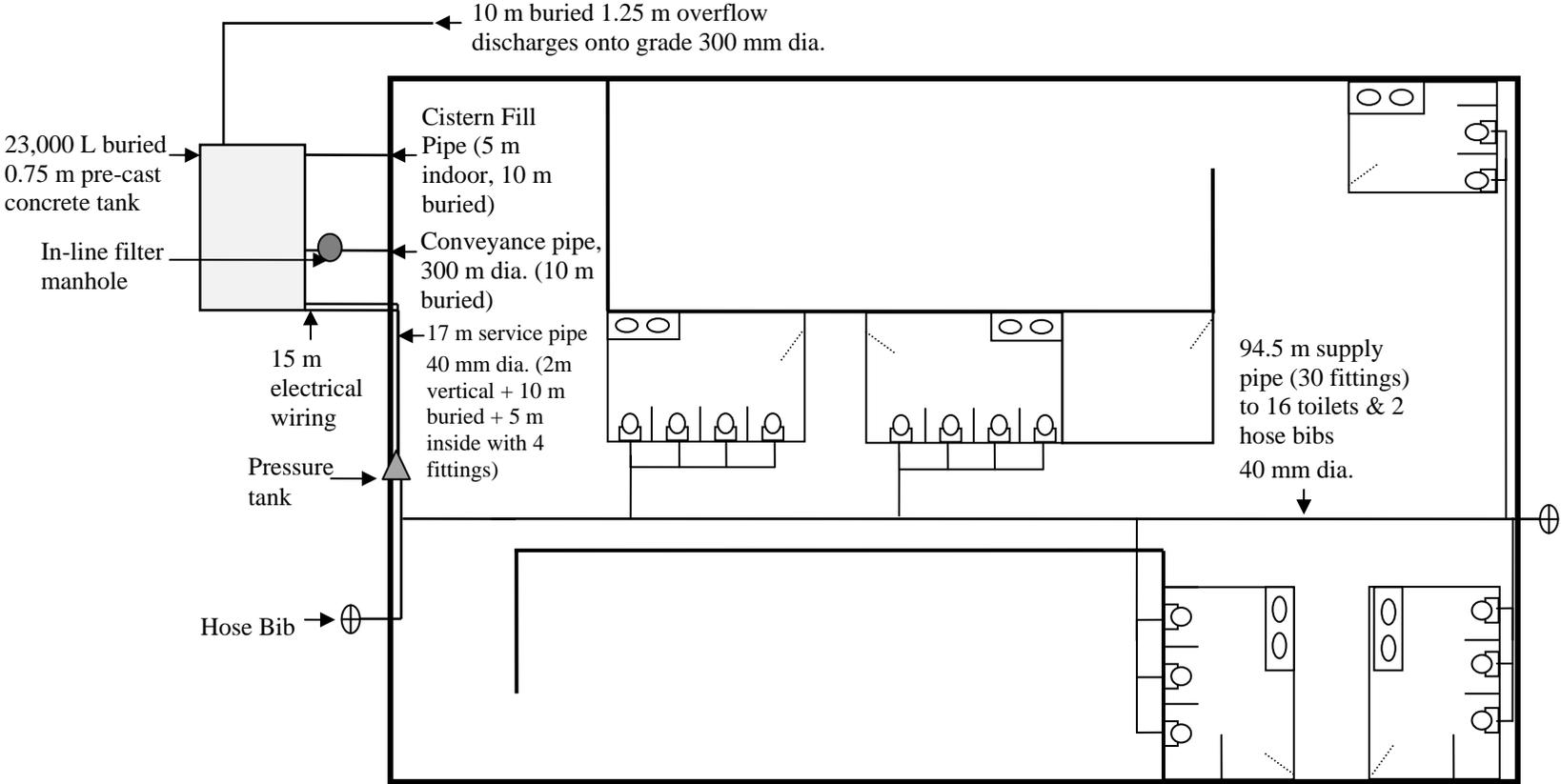


Figure 3.18: Site design for buried pre-cast concrete tank

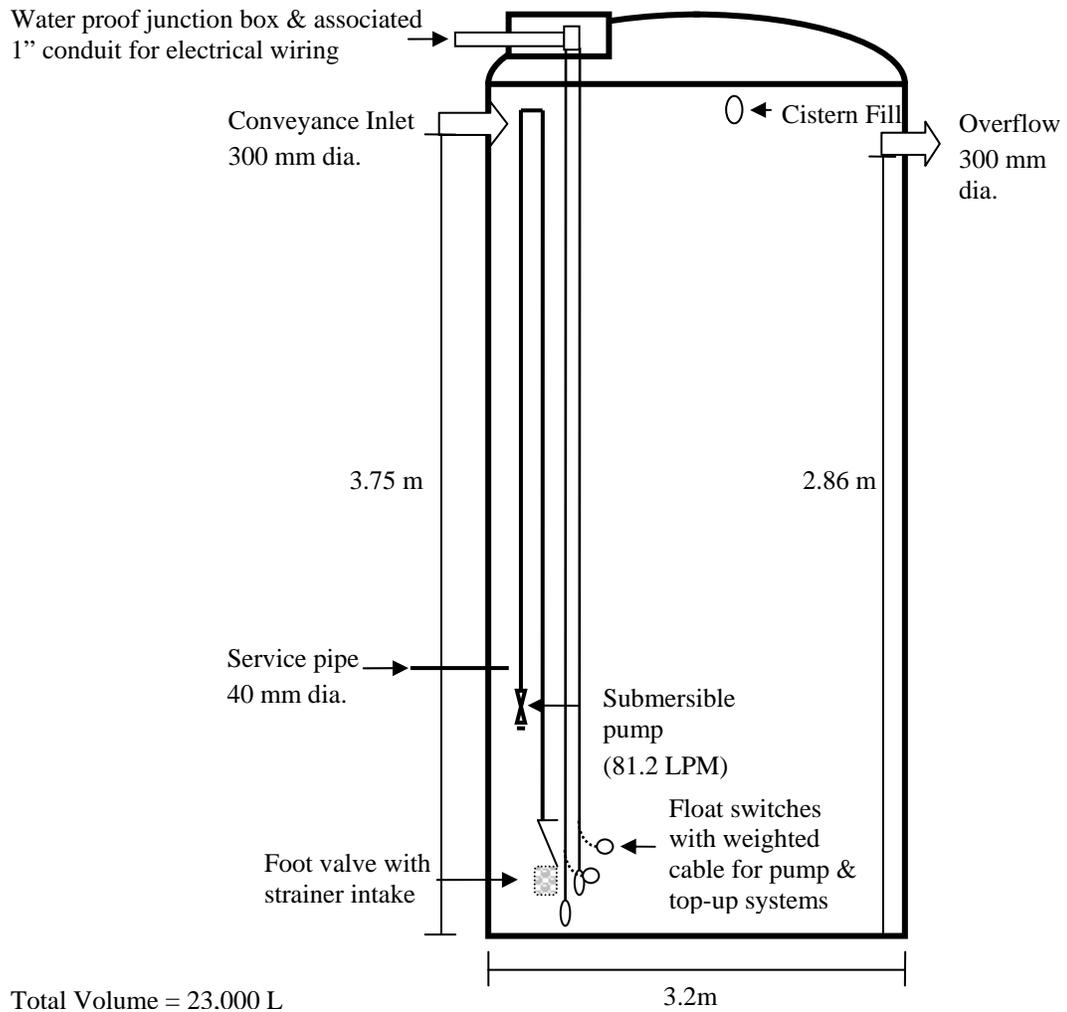


Figure 3.19: Plastic tank design

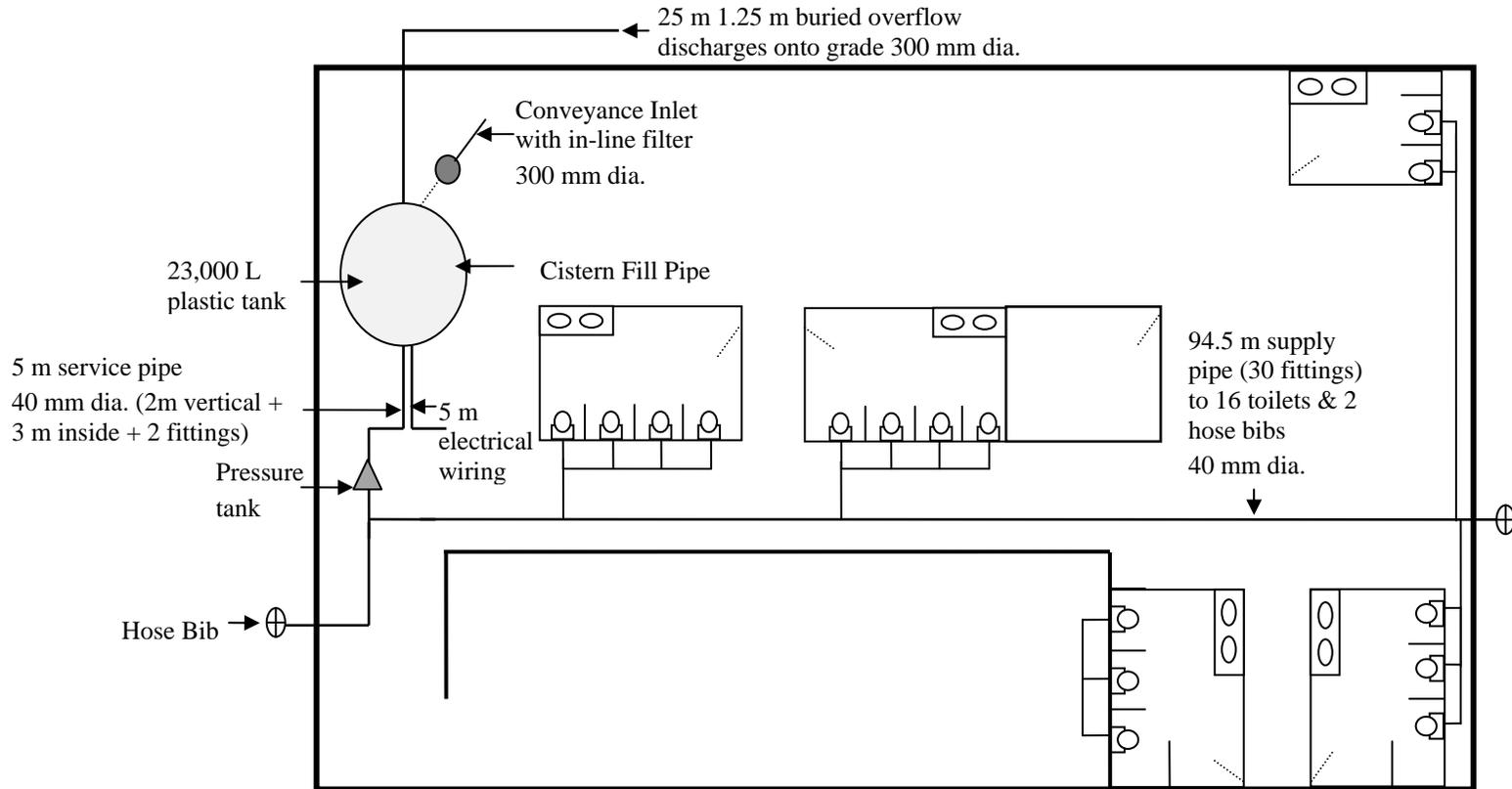


Figure 3.20: Site design for indoor plastic tank

3.6.2 Capital Costs

The concrete tank cost more than the plastic tank, primarily due to the added costs for excavation (Table 3.11). The trench and piping for the overflow cost more for the plastic tank because it was assumed that it would be further from the discharge point and therefore needed to be double the length. Most of the major costs for the tank, pump and piping were similar.

Table 3.11: Rainwater harvesting capital costs

Input Parameters	Concrete Tank Outdoor	Plastic Tank Indoor
Planning & Site Preparation	\$4,794	\$3,694
Excavation	\$1,244	\$0
Materials & Installation	\$41,199	\$36,943
Total	\$47,237	\$40,637

3.6.3 Life Cycle Costs

In the life cycle cost estimates shown in Table 3.12 the plastic tank is replaced in year 40, at a cost of \$7,170, whereas the concrete cistern is assumed to last longer. Average annual maintenance costs are the same in the two scenarios at roughly \$744. The requirement for replacing the plastic tank brings the net present values of the two scenarios closer together, with the plastic tank being only slightly less expensive at a 0% discount rate over the 50 year evaluation period.

Table 3.12: Rainwater harvesting life cycle costs

Input Parameters	Concrete Tank Outdoor	Plastic Tank Indoor
Life span	50+ years	40 years
Capital cost	\$47,237	\$40,637
Replacement cost at 40 years	na	\$5,970
Annual maintenance	\$744	\$744
Present Value including capital, maintenance and rehabilitation costs		
NPV at 50 years		
if i = 0 %	\$84,451	\$83,821
if i = 3 %	\$66,088	\$61,318
if i = 5 %	\$60,140	\$54,388
NPV at 25 years		
if i = 0 %	\$65,844	\$59,244
if i = 3 %	\$59,519	\$52,919
if i = 5 %	\$56,754	\$50,154

Note: i = discount rate

3.7 Extensive Greenroof

Greenroofs are typically classified as either extensive or intensive. Extensive greenroofs support low growing plants and have substrate depths ranging from 5 to 15 cm. A greenroof with a substrate deeper than 15 cm is normally defined as intensive. Extensive roofs are much more common and were therefore selected as the basis for detailed costing in this project.

A green roof assembly usually consists of the following components above the roofing membrane: a root-resistant layer to minimize root damage to the membrane; a drainage layer to remove excess water from the drainage medium; a filter fabric to prevent fine particles in the growing medium from clogging the drainage layer; a growing medium to support healthy plant growth, and plants selected for their adaptability to local climate conditions. An irrigation system may also be needed depending on the type of plants selected.

3.7.1 Model Scenarios and Designs

The scenarios selected for model costing include inexpensive and more expensive variations of an extensive green roof. Since green roofs are usually installed on gently sloping roofs, both scenarios assume a roof slope of 2%. As with other practices, it is assumed that the green roof is installed as part of the original new building design (i.e. not a retrofit).

'Cheap' system

The inexpensive system involves installing a sedum cutting system with a 10 cm growing medium on a building less than 5 stories high, which makes it easier to get the plants and green roof materials onto the roof. This scenario does not include pathways, fencing or other features that help improve accessibility. The water leakage test is a simpler, less expensive test than the more sophisticated methods. A TPO waterproof membrane is used.

Expensive system

In this scenario, the building is over 5 stories tall, the waterproof membrane is more expensive and a sophisticated water leakage test is performed. It also includes a root barrier, an irrigation system, more expensive edging and a 15 cm growing medium. Plants are in the form of sedum mats, which are much more expensive than sedum plugs or cuttings. A more expensive EPDM waterproof membrane was used in this scenario.

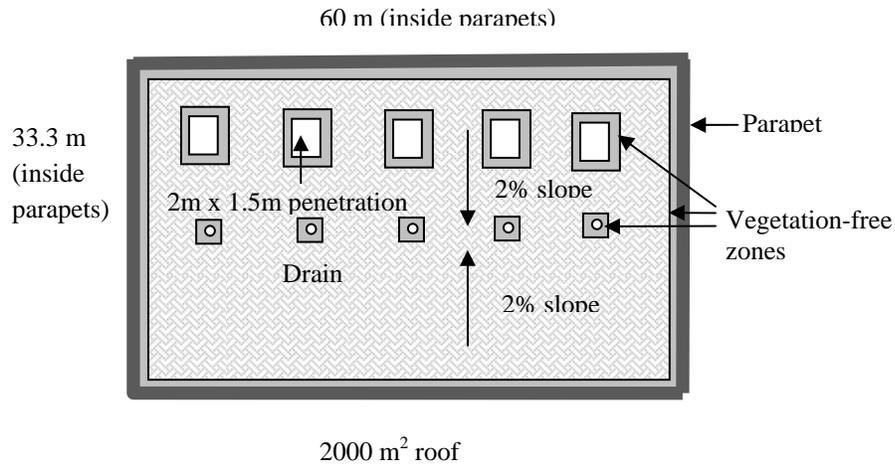


Figure 3.21: Plan view of greenroof design

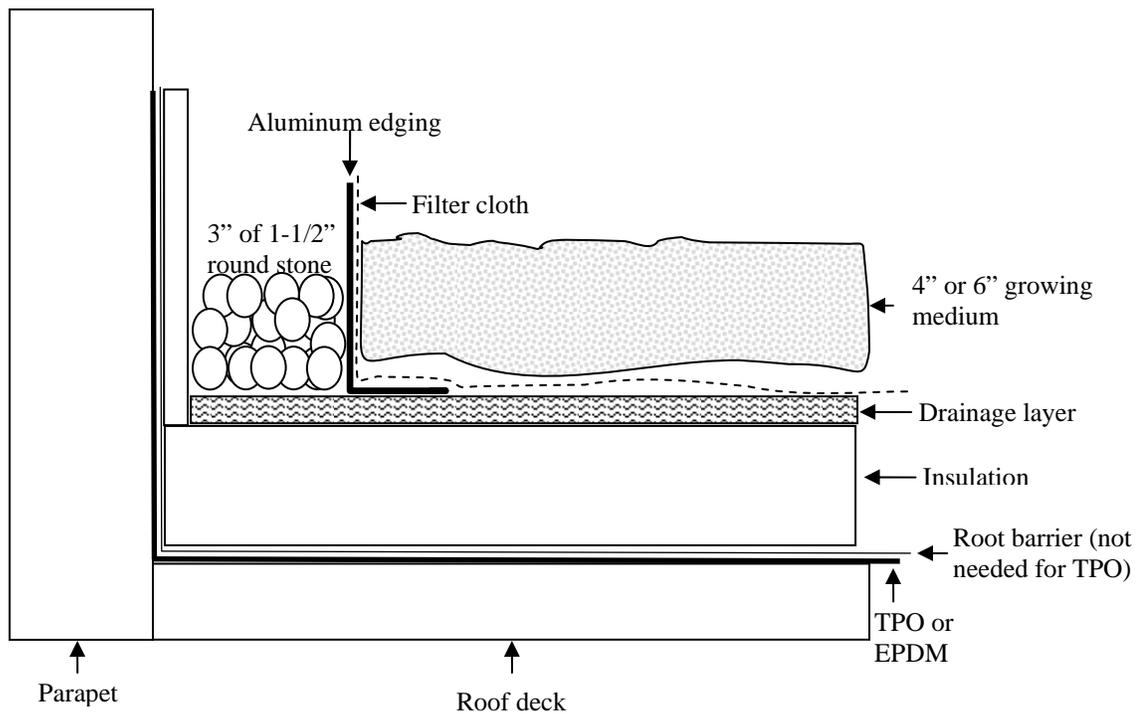


Figure 3.22: Cross section of greenroof design

3.7.2 Capital Costs

The capital cost breakdown shown in Table 3.13 and Appendix A shows how the differences between the two scenarios affect the overall price of the systems. The expensive system is more than twice the cost of the cheap system, mostly due to differences in the cost of materials. The membrane represents a significant component of the cost, but since all roofs require membranes,

it should not be regarded as a cost that is unique to green roofs. Only the 'expensive' green roof has a specialized membrane that would be more costly than a conventional roof.

Table 3.13: Extensive greenroof capital costs

Input Parameters	Cheap		Expensive	
	With Membrane	Without Membrane	With Membrane	Without Membrane
Planning & Site Preparation	\$21,341	\$10,163	\$44,804	\$31,693
Craning	\$13,897	\$9,265	\$56,676	\$51,524
Materials & Installation	\$196,040	\$90,631	\$371,430	\$255,441
Total	\$231,278	\$110,060	\$472,909	\$338,658

Detailed costs for conventional roofs were not calculated in this project. However, these were estimated in an earlier study (STEP, 2007) that compared the life cycle cost of green roofs to conventional roofs under various scenarios. In that study, the initial capital cost of a new conventional roof of an equivalent area (2000 m²) was estimated to be a minimum of \$172,000, not including the roof deck. Accordingly, a green roof would add at least \$59,278 to the initial capital cost of the roof. These extra initial costs would be recouped to some extent by the green roof's much longer life and other energy, stormwater and biodiversity benefits.

3.7.3 Life Cycle Costs

The life span of the green roofs was estimated to be 40 years regardless of the scenario. The less expensive scenario had much lower replacement costs, but the \$308 higher annual maintenance costs resulted in a similar net present value for overall maintenance and rehabilitation (assuming a 5% discount rate). The discount rate is a particularly important factor in these scenarios because the high replacement costs play a significant role in the overall Net Present Value calculations.

Table 3.14: Extensive greenroof life cycle costs

Input Parameters	Cheap		Expensive	
	With Membrane	Without Membrane	With Membrane	Without Membrane
Life span	40 years	40 years	40 years	40 years
Capital cost	\$231,278	\$110,060	\$472,909	\$338,658
Replacement cost at 40 years	\$373,628	\$209,187	\$613,542	\$436,068
Annual maintenance	\$2,022	\$1854	\$1,714	\$1546
Present Value including capital, maintenance and rehabilitation costs				
NPV at 50 years				
if i = 0 %	\$706,022	\$411,947	\$1,172,167	\$852,026
if i = 3 %	\$413,506	\$237,683	\$705,990	\$513,139
if i = 5 %	\$341,174	\$193,720	\$592,200	\$429,862
NPV at 25 years				
if i = 0 %	\$301,346	\$175,920	\$519,577	\$381,118
if i = 3 %	\$288,698	\$164,706	\$504,838	\$367,814
if i = 5 %	\$282,999	\$159,617	\$498,524	\$362,109

Note: i = discount rate

In the earlier STEP study (2007) that compared the cost of conventional roofs to green roofs, various cost and roof longevity scenarios were also evaluated. The scenario that assumed a discount rate of 3.5% and green roof longevity of 45 years showed that even moderately priced green roofs, with initial capital costs similar to this study, can cost less than conventional roofs (which were assumed to last 15 years) while providing other stormwater, biodiversity, energy and heat island mitigation benefits.

4.0 COMPARISON OF LID PRACTICE COSTS

Selecting the preferred combination of stormwater practices for a development site requires knowledge of environmental conditions, space constraints, the anticipated water balance and water quality impacts, as well as the type of practices that would help mitigate these impacts. Since different practices can achieve similar benefits, cost becomes an important criterion for selecting among available stormwater treatment options.

4.1 Capital Costs

The initial capital costs associated with planning, design and construction of the different practice scenarios and practice types are compared in Figure 4.1. The comparison shows that bioretention, rainwater harvesting and the road runoff variations of infiltration chambers and trenches fall within a similar range of costs. Permeable pavements are more expensive, mostly due to the higher cost of materials and installation. Enhanced swales are the least expensive in part because they are designed primarily for conveyance, rather than water balance control. Infiltration trenches and chambers that receive only roof runoff are also relatively cost effective because of the lower costs for pre-treatment. Green roofs are the most expensive but offer a range of benefits that are unique to this practice. They also displace the need to install a conventional roof, which none of the other practices do.

In interpreting these results, it is important to recognize that only the practice itself is assigned a cost. The savings that may be gained from implementing one practice over another are not captured. Thus, for instance, if the project involves building a new parking lot, there would be costs associated with paving the parking lot with asphalt and installing a practice that helps mitigate the water quantity and quality impacts of the runoff generated. Selecting permeable pavement would mean that only a portion of the parking lot would require paving (assuming some asphalt drains onto the pavement), resulting in cost savings over a practice such as bioretention, which cannot be used as a parking surface, and would therefore require more asphalt paving. In the case of bioretention, there may also be a cost associated with the larger area required to accommodate the practice, given that each practice has the same roof and/or paved drainage area. If instead, an underground chamber were used, the cost of asphalt above the chamber would be extra, but there would be no impact on buildable area. The specific context of the project, therefore, will play a critical role in the overall cost of the project

The cost data can also be viewed through the lens of the benefits the different practices provide with regards to stormwater treatment. Focusing specifically on volume reduction is perhaps the simplest means of accomplishing this task because reducing runoff volumes addresses multiple issues, including water quality, stream erosion, thermal impacts and groundwater recharge. The costs could then be expressed per unit volume of runoff reduced through infiltration and/or evapotranspiration. This approach works less well for building integrated practices such as green roofs and rainwater harvesting because the unique practice values associated with, for example,

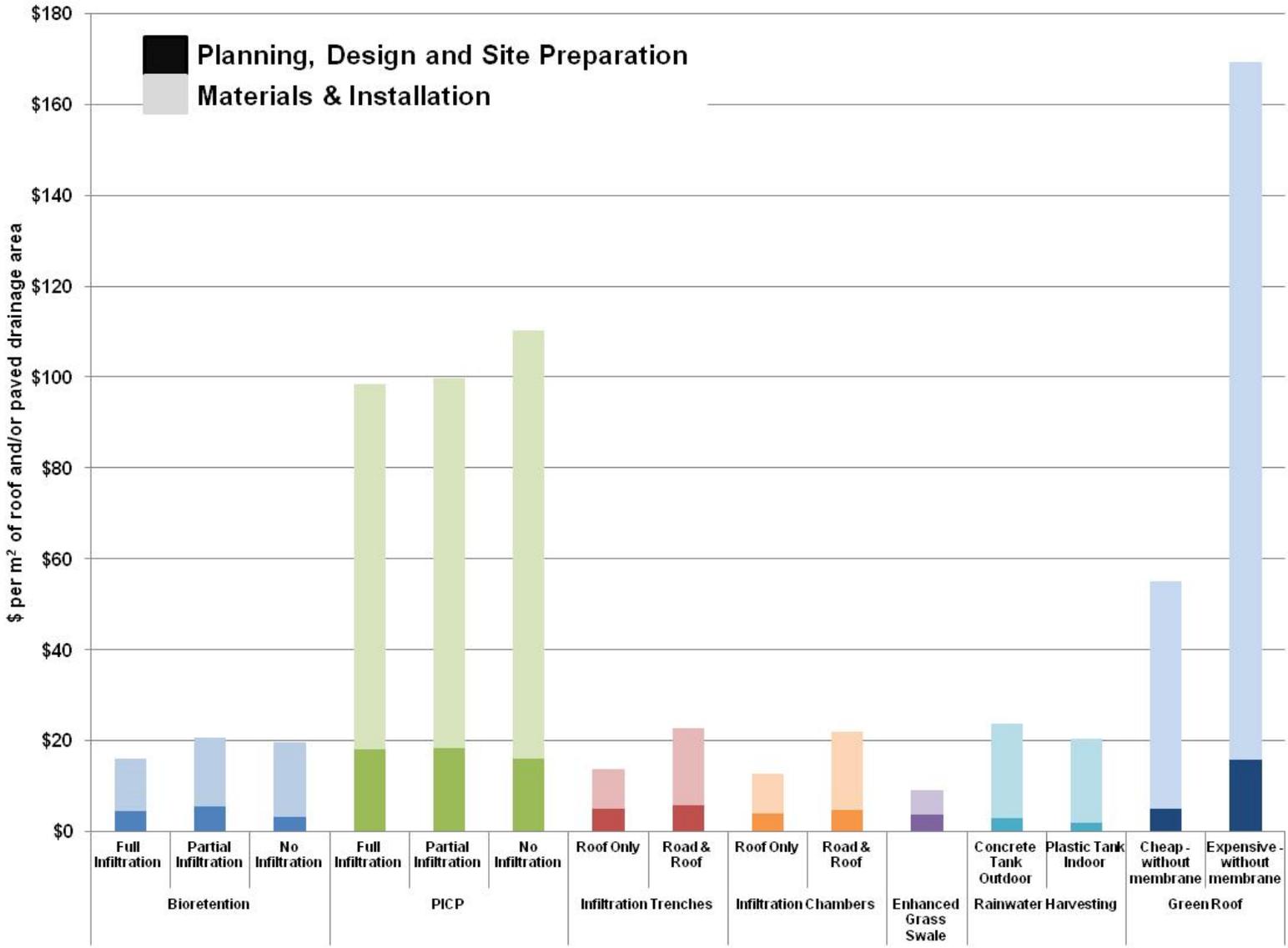


Figure 4.1: Capital costs for all practices per m² of roof and/or paved drainage area

energy reduction or potable water savings, are not accounted for in the overall cost/benefit. An example of costs expressed in relation to the load of suspended solids reduced for different treatment options is provided in section 4.3.

4.2 Life Cycle Costs

Table 4.1 compares LID practice costs for annual maintenance, rehabilitation and overall net present values at discount rates ranging from 0 to 5%. Figure 4.2 shows net present values for the 25 and 50 year time periods. Annual maintenance costs averaged over 50 years ranged from \$74 for infiltration chambers and trenches treating roof runoff to \$2,022 for green roofs. In general, maintenance costs were higher for practices requiring plant maintenance, such as bioretention and green roofs, or extensive pre-treatment, such as infiltration chambers and trenches treating road runoff. Rainwater harvesting systems require relatively little maintenance but pumps and pressure tanks need to be replaced at 10 year intervals.

All practices except rainwater harvesting (concrete cistern), underground chambers or trenches and enhanced grass swales required major rehabilitation at some point in the 50 year time period. These expensive rehabilitation costs weigh heavily in the net present value calculations, particularly at low discount rates, making the practices not requiring rehabilitation comparably less expensive over the long term.

Table 4.1: Life cycle costs for all practices

	Bioretention			Permeable Pavement			Infiltration Trenches		Infiltration Chambers		Enhanced Grass Swales	Rainwater Harvesting		Green Roofs			
	Full Infiltration	Partial Infiltration	No Infiltration	Full Infiltration	Partial Infiltration	No Infiltration	Roof Only	Road & Roof	Roof Only	Road & Roof	Rock Check Dam	Concrete Tank Outdoor	Plastic Tank Indoor	Cheap		Expensive	
														With membrane	Without membrane	With membrane	Without membrane
Construction	\$31,973	\$41,476	\$39,028	\$98,313	\$99,652	\$110,153	\$27,575	\$45,534	\$25,547	\$43,706	\$18,347	\$47,237	\$40,637	\$231,278	\$110,060	\$472,909	\$338,658
50 year evaluation period																	
Ave. annual maintenance	\$945	\$952	\$952	\$433	\$436	\$436	\$74	\$1,277	\$74	\$1,212	\$500	\$744	\$744	\$2,022	\$1,854	\$1,714	\$1,546
Rehabilitation	\$7,504	\$7,504	\$7,504	\$72,990	\$72,990	\$72,990	na	na	na	na	na	na	\$5,970	\$373,628	\$209,187	\$613,542	\$436,068
Year rehabilitation required	25	25	25	30	30	30	na	na	na	na	na	na	40	40	40	40	40
Present Value of maintenance & rehabilitation only																	
if i=0%	\$54,743	\$55,128	\$55,128	\$94,657	\$94,810	\$94,810	\$3,675	\$63,850	\$3,675	\$60,600	\$24,985	\$37,214	\$43,184	\$474,744	\$301,887	\$699,258	\$513,368
if i=3%	\$28,498	\$28,670	\$28,670	\$41,239	\$41,316	\$41,316	\$1,857	\$32,276	\$1,857	\$30,563	\$13,769	\$18,851	\$20,681	\$182,228	\$127,623	\$233,081	\$174,481
if i=5%	\$20,210	\$20,322	\$20,322	\$24,768	\$24,820	\$24,820	\$1,298	\$22,556	\$1,298	\$21,332	\$10,292	\$12,903	\$13,751	\$109,896	\$83,660	\$119,291	\$91,204
Present value of all (capital cost, maintenance & rehabilitation)																	
if i=0%	\$86,716	\$96,604	\$94,156	\$192,970	\$194,462	\$204,963	\$31,250	\$109,384	\$29,222	\$104,306	\$43,333	\$84,451	\$83,821	\$706,022	\$411,947	\$1,172,167	\$852,026
if i=3%	\$60,471	\$70,146	\$67,698	\$139,552	\$140,968	\$151,469	\$29,432	\$77,810	\$27,404	\$74,269	\$32,117	\$66,088	\$61,318	\$413,506	\$237,683	\$705,990	\$513,139
if i=5%	\$52,183	\$61,798	\$59,350	\$123,081	\$124,472	\$134,973	\$28,873	\$68,090	\$26,845	\$65,038	\$28,639	\$60,140	\$54,388	\$341,174	\$193,720	\$592,200	\$429,862
25 year evaluation period																	
Ave. annual maintenance	\$972	\$978	\$978	\$433	\$436	\$436	\$72	\$1,264	\$72	\$1,188	\$537	\$744	\$744	\$2,802	\$2,634	\$1,867	\$1,698
Rehabilitation	na	na	na	na	na	na	na	na	na	na	na	na	na	na	na	na	na
Year rehabilitation required	na	na	na	na	na	na	na	na	na	na	na	na	na	na	na	na	na
Present Value of maintenance & rehabilitation only																	
if i=0%	\$24,293	\$24,447	\$24,447	\$10,833	\$10,910	\$10,910	\$1,800	\$31,600	\$1,800	\$29,700	\$13,429	\$18,607	\$18,607	\$70,068	\$65,860	\$46,668	\$42,460
if i=3%	\$17,255	\$17,355	\$17,355	\$7,483	\$7,533	\$7,533	\$1,233	\$21,593	\$1,233	\$20,302	\$9,922	\$12,282	\$12,282	\$57,420	\$54,646	\$31,929	\$29,156
if i=5%	\$14,156	\$14,233	\$14,233	\$6,012	\$6,051	\$6,051	\$986	\$17,226	\$986	\$16,203	\$8,365	\$9,517	\$9,517	\$51,721	\$49,557	\$25,615	\$23,451
Present value of all (capital cost, maintenance & rehabilitation)																	
if i=0%	\$56,266	\$65,923	\$63,475	\$109,146	\$110,562	\$121,063	\$29,375	\$77,134	\$27,347	\$73,406	\$31,777	\$65,844	\$59,244	\$301,346	\$175,920	\$519,577	\$381,118
if i=3%	\$49,228	\$58,831	\$56,383	\$105,796	\$107,185	\$117,686	\$28,808	\$67,127	\$26,780	\$64,008	\$28,270	\$59,519	\$52,919	\$288,698	\$164,706	\$504,838	\$367,814
if i=5%	\$46,129	\$55,709	\$53,261	\$104,325	\$105,703	\$116,204	\$28,561	\$62,760	\$26,533	\$59,909	\$26,712	\$56,754	\$50,154	\$282,999	\$159,617	\$498,524	\$362,109

Note: i = discount rate

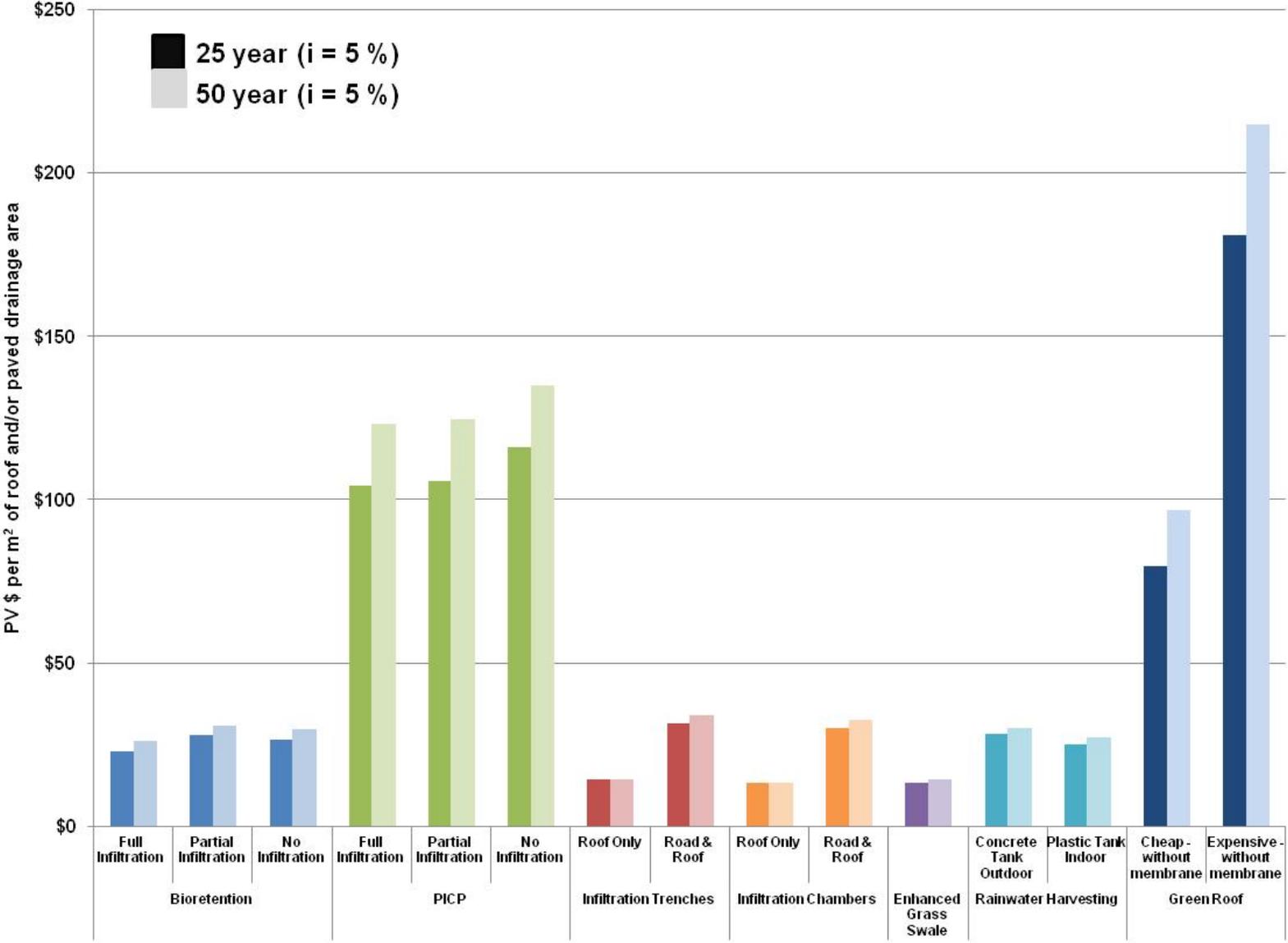


Figure 4.2: Present values for 25 year and 50 year evaluation periods for all practices per m² of roof and/or paved drainage area

4.3 Comparisons to Conventional Grey Infrastructure

The previous section compared the cost of LID practices to one another. In this section, the costs of selected practices are compared to conventional grey infrastructure based on the cost both of the practice and the contributing drainage area. The practice type used in the analysis is for a partial infiltration system.

The scenarios selected for comparative analysis have an asphalt drainage area of 2000 m² with treatment provided by different types of LID practices and a conventional oil grit separator. The scenarios were as follows:

1. Asphalt (2000 m²) draining to a storm sewer with treatment provided by an appropriately sized **oil grit separator**
2. Asphalt (2000 m²) draining to a 130 m² partial infiltration **bioretention cell** (see design and costing in section 3.1)
3. Asphalt (1000 m²) draining to 1000 m² partial infiltration **permeable interlocking concrete pavement** (see design and costing in section 3.2).
4. Asphalt (2000 m²) draining to a 100 m² infiltration **trench with pre-treatment provided through a 20 m² gravel inlet** (substituted OGS in the trench design provided in section 3.3 for a much less expensive gravel filter inlet)
5. Asphalt (2000 m²) draining to a 100 m² underground **infiltration trench with pre-treatment provided by an Oil Grit Separator** (similar to model provided in section 3.3, but the OGS is larger to accommodate the larger asphalt drainage area).
6. Asphalt (2000 m²) draining to a 200 m² **enhanced swale** (see design and costing in section 3.5)

It should be noted that the bioretention cell, infiltration trench with gravel filter, and enhanced swale take up 130, 20 and 200 m² more space than the other scenarios, respectively. The conventional scenario with OGS treatment also differs significantly from the others as it is the only practice that does not reduce runoff volumes and contaminant loads through infiltration and/or evapotranspiration. The enhanced swale would also be expected to infiltrate less runoff than the other LID practices since it is designed to convey runoff.

Figure 4.3 presents the initial capital and present value costs of the scenarios (asphalt + treatment option) over the 50 year evaluation period at a discount rate of 5%. The initial capital costs of the different treatment scenarios are relatively similar, ranging from \$54 to \$73 per square meter of paved drainage area. The conventional OGS treatment scenario had the second lowest initial cost, at \$57 per m² of paved drainage. When routine maintenance and

rehabilitation/replacement costs are added, and expressed as net present value, the conventional OGS treatment scenario has the third highest cost, at \$114 per m² of paved drainage.

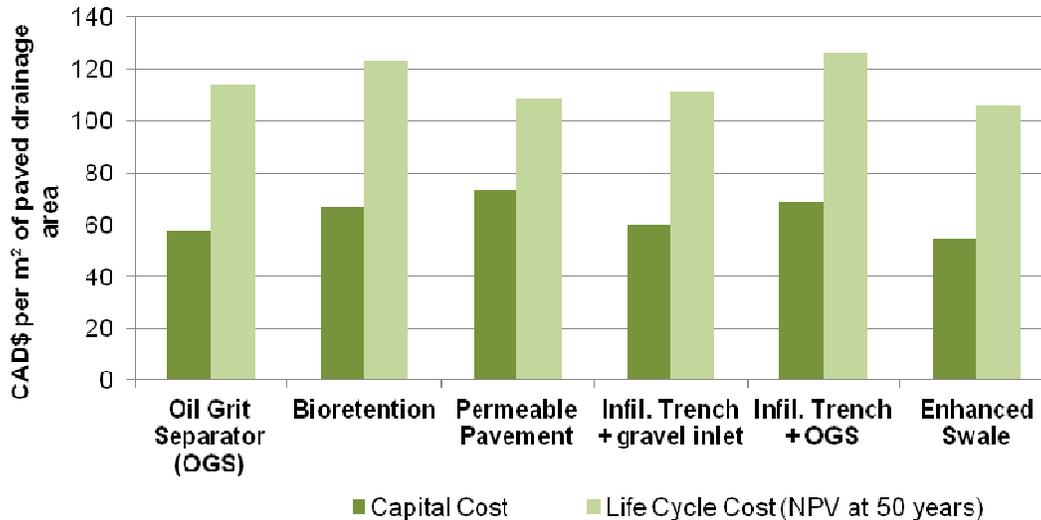


Figure 4.3: Capital and life cycle costs for different asphalt runoff treatment scenarios

The costs presented in Figure 4.3 do not consider differences in the stormwater management benefits of the practices. These differences are shown in Table 4.2 in relation to estimated reductions in runoff, total suspended solids (TSS) concentrations and TSS loads. The runoff reduction estimates are based on literature values provided in the *LID Guide*. TSS concentration reductions are based on literature reviews and local studies (e.g. STEP, 2008; Drake et al, 2012). To show the value of these benefits, the capital and life cycle costs of the scenarios are expressed in Figure 4.4 as dollars per kilograms of TSS reduced annually, assuming an average influent concentration of 200 mg/L, annual precipitation of 800 mm (based on climate ‘normals’ in Ontario) and an asphalt runoff coefficient of 0.98.

Table 4.2: Estimated reductions in runoff, TSS concentrations and loads for six asphalt treatment scenarios

Treatment Scenario	Runoff Reduction (%)	TSS Concentration Reduction (%)	Load Reduction (%)
Oil Grit Separator	0	50	50
Partial Infiltration Bioretention	45	80	89
Partial Infiltration Permeable Pavement	45	80	89
Infiltration Trench with gravel inlet	45	50	73
Infiltration Trench with Oil Grit Separator	45	50	73
Enhanced Swale	20	60	68

As expected, LID practices are less expensive than traditional OGS treatment when costs are denominated in terms of their water quality benefits. The capital cost of LID scenarios was between 24 and 44% lower than conventional OGS treatment. On a life cycle cost basis, these savings increase to between 35 and 77%. The cost differences relative to conventional OGS treatment would be even greater if the native soils were sandy, as this would significantly increase the volume of runoff reduced and the practices would be cheaper to construct because of lower material and installation costs (see full infiltration scenario models in chapter 3).

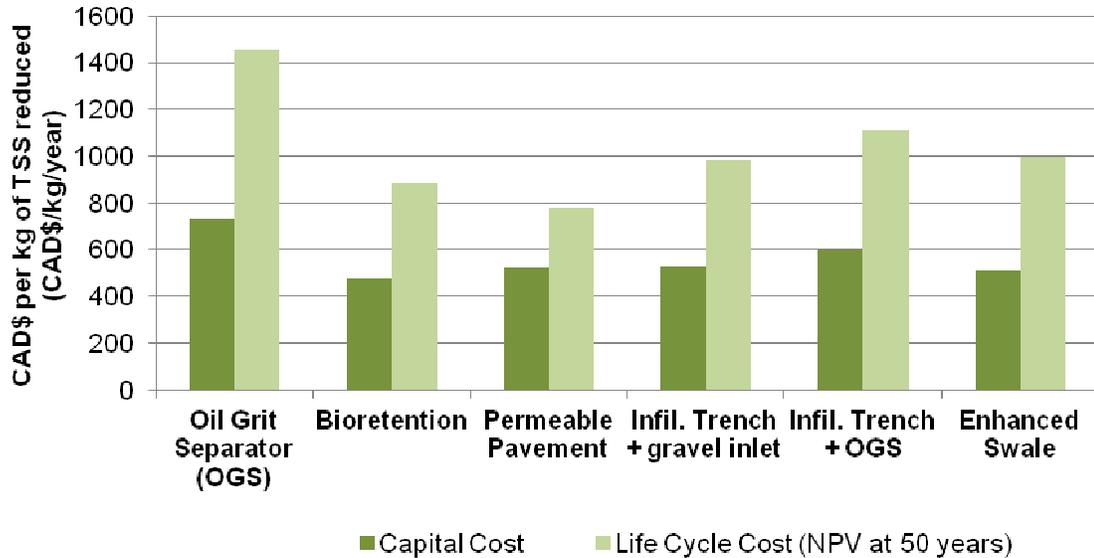


Figure 4.4: Capital and life cycle costs expressed per kilogram of total suspended solids (TSS) load reduced (see text for assumptions)

4.4 Comparison of Study Findings to Other Literature

A literature review was conducted to compare construction, maintenance and rehabilitation costs estimated in this study with those found in other studies or estimated by other models. The literature review was intended to provide an overview of key sources of LID cost information rather than a comprehensive summary of all the literature available on the topic.

Capital cost literature values are compared in the next section, followed by a comparison of maintenance and rehabilitation costs. Most literature values were converted to 2010 \$CAD and to the Toronto location using RSMMeans year and location conversion factors.

In reviewing the literature, every attempt was made to select a design similar to the model designs in this study in order to provide more robust comparisons. However, in several instances this was not possible as each study/model incorporated its own unique design assumptions, and included or excluded different components from the construction cost. These differences in methodologies contributed to some of the variation in costs among studies.

4.4.1 Review of literature on LID construction costs

Capital costs of LID practices included in this study are compared with other comparable literature in Table 4.3. The comparison indicates that cost estimates of bioretention and permeable pavers from this study fall on the low end of the range suggested by literature and other models when converted to 2010 \$CAD. Differences in design and costing assumptions account for some of the discrepancy. For instance, WERF added 20% contingency to its permeable paver cost, which was not included in this study. The Olson et al (2010) estimate for permeable pavers provided in Table 4.3 is interpolated from detailed costing of 3 projects in Denver Colorado ranging in size from 324 m² to 2,671 m².

The cost of infiltration trenches in this study (without the hydrodynamic separator) appears to be on the high end of the range suggested by literature. There was no literature available for Infiltration Chambers. However, the cost of the Infiltration Chamber system (without the hydrodynamic separator) matched relatively closely to that suggested by the chamber manufacturer. The cost of the hydrodynamic separator used as a pretreatment device for the chambers and trenches was lower than the Olson et al (2010) estimate.

Cost estimates in this study for rainwater harvesting were higher than the two sources reviewed, likely due to differences in the design and/or range of costs included. Costs from the STEP (2010a) study were provided by the design engineer and did not include indoor piping. WERF had higher tank and installation costs, but lower costs for the pump and piping. Filter and top-up system costs, which accounted for over 10K in the present study did not appear to be included.

Green roof costs in this study were lower than green roof industry sources, possibly due to a lower assumed mark-up. Our costs, however, did line up with those of WERF, which takes a unit costing approach similar to ours. The TRCA survey of local green roofs installed in the GTA reported slightly lower costs, in part because many of the buildings in this survey were low rises that did not require expensive equipment to move the green roof materials (STEP, 2007).

Table 4.3: LID capital cost comparison to literature/other models. Italicized text indicates literature values are lower; bolded text indicates literature costs are higher, and normal text indicates costs are similar

LID	Model design	Capital cost in this study	Literature values			
Bioretention	Full Infiltration	\$32K	\$26K WERF 2009	\$32K Weiss et al 2007	\$40K Brown & Schueler 1997	\$44K Olson et al 2010
	Partial Infiltration	\$42K	\$49K Weiss et al 2007	\$55K WERF 2009	\$59K Brown & Schueler 1997	\$60K Olson et al 2010
	No Infiltration	\$39K	\$41K Weiss et al 2007	\$49K Brown & Schueler 1997	\$52K Olson et al 2010	\$55K WERF 2009
Permeable Pavers	Full Infiltration	\$98K	\$55K (This only includes paver, bedding, base & sub-base) ICPI 2011	\$99K - \$198K (This includes 20% contingency, not included in our estimate) WERF 2009	\$188K Olson et al 2010	
	Partial Infiltration	\$100K				
	No Infiltration	\$110K				
Infiltration Trench	Full Infiltration – 2000m ² roof runoff	\$28K	\$14K Brown & Schueler 1997	\$25K Weiss et al 2007		
	Full Infiltration – 1500m ² parking lot + 500m ² roof runoff	\$45K	This design includes a hydrodynamic separator, which is assessed below.			
Infiltration Chamber	Full Infiltration – 2000m ² roof runoff	\$26K	\$17K (This is lower because Stormtech is only including items directly related to the chambers) Stormtech 2012			
	Full Infiltration – 1500m ² parking lot + 500m ² roof runoff	\$43K	Lit review not done. This design includes a hydrodynamic separator, which is assessed below.			
	Hydrodynamic separator	\$15K	\$4K - \$72K USEPA 1999	\$38K Olson et al 2010		
Rainwater Harvesting	Buried concrete tank	\$47K	\$23K <i>General rule of thumb \$1/L</i>	\$24K		
	Indoor plastic tank	\$41K		\$21K WERF 2009		

LID	Model design	Capital cost in this study	Literature values		
Green Roof (extensive)	Cheap Case (4" growing medium, sedum plugs, lower building)	\$247K	\$189K (costs are similar when design differences between studies are considered)	\$344K - \$430K	\$646K-\$754K
			WERF 2009	Bass 2012	Wylie 2012
	Expensive Case (6" growing medium, sedum mats, higher building)	\$473K	\$316K (costs are similar when design differences between studies are considered)		\$279 - \$397 (based on supplier estimates for the green roof system including the base roof)
			WERF 2009		STEP 2007
	Expensive Case (no membrane, green roof only)	\$339K			\$238-256K based on actual green roof costs of projects in the GTA, not including the membrane. \$228K-\$360K based on supplier estimates not including the base roof
					STEP 2007

4.4.2 Review of literature on LID maintenance and rehabilitation costs

Table 4.4 presents LID maintenance cost comparisons from various literature sources. Overall, cost estimates for maintenance provided in this study align reasonably well with literature values.

The maintenance costs for bioretention cells estimated in this study are in good agreement with most of the values from the literature or other models. None of the literature sources indicated a periodic rehabilitation cost. Incorporating the rehabilitation cost at 25 years into the annual maintenance cost would only increase it to \$1,103 per year. A 134 m² bioretention facility installed in Vaughan, Ontario in 2010 and monitored through the TRCA's Sustainable Technologies Evaluation Program had annual maintenance costs of approximately \$1200, mostly for weeding and plant maintenance (STEP, 2010b).

The annual cost estimates in this study for maintaining permeable pavers were consistent with those suggested by other literature. Higher end ranges in the literature assumed that more frequent maintenance may be required in some circumstances. Major rehabilitation costs varied among literature sources. WERF recommended removing, washing and replacing all the aggregate at a cost equal to the initial cost of installation. In the present study, it was assumed that the rehabilitation would cost only about two thirds of the initial cost because fines would be largely removed from the surface through regular cleaning, thereby preserving the integrity of the open graded base. The 2010 Olson et al study assumed rehabilitation would cost 80% of the initial installation cost, and that it would be required after only 18 years. The present study suggested a 30 year timeline for rehabilitation based on observations of the structural condition of older permeable pavement sites in the Greater Toronto Area.

Infiltration trenches and chambers were assumed to require very little maintenance if adequate pretreatment was provided based on installations monitored in Ontario (e.g. SWAMP, 2004; JF Sabourin and Associates, 2008). Hence maintenance costs for cleanout of the hydrodynamic device providing pretreatment to the infiltration trench and chamber was the primary maintenance cost for these practices. Costs for clean-out of this device agreed well with other models. The Olson et al (2010) model assumed an additional cost for replacing the hydrodynamic separator after 25 years which was deemed unnecessary in this study.

Rainwater harvesting annual maintenance costs were slightly lower than the lower end estimate by WERF. However, WERF assumed a longer replacement cost for the plastic tank. This extra replacement cost contributes significantly to the 50 year maintenance burden.

Green roof annual maintenance costs in the first two years agree well with other references (CMHC, 2003; GRHC, 2006) that consider this initial period of plant establishment to be a period of more intense maintenance. Higher end maintenance costs may be required on accessible green roofs, or roofs visible from the building windows. However, most green roofs in Ontario are not of this type. Two green roofs monitored by TRCA/STEP in the Toronto area have required very little maintenance after the first year, as plants brought to the rooftop garden by wind or animals have been allowed to thrive or replace the original plant stock.

Table 4.4: LID maintenance and rehabilitation cost comparison to literature/other models. Italicized text indicates literature values are lower; bolded text indicates literature costs are higher, and normal text indicates costs are similar

LID	Maintenance or rehabilitation	Cost in this study	Literature values			
Bioretention	Annual maintenance	\$945 - \$952 (2%-3% of construction cost)	0.7% - 10.9% of construction cost Weiss et al 2005	\$454 - \$6425 WERF 2009	\$1182 Olson et al 2010	5% - 7% of construction cost USEPA, 1999
	Rehabilitation	\$7,504 after 25 years	None of the above references mention separate rehab costs.			
Permeable Pavers	Annual maintenance	\$433 - \$436	\$64	\$130 - \$3,550 (medium is \$330)	\$450 - \$3,400 (median of \$1,900) for sediment removal only	
	Rehabilitation	\$75K (63% - 71% of initial construction cost) after 30 years	Olson et al 2010 100% of initial construction cost after 25, 35 or 45 years WERF 2009	WERF 2009 80% of initial construction cost every 18 years Olson et al 2010	Erickson et al 2010	
Infiltration Trench 2000m ² roof runoff	Annual maintenance	\$74 (all for sediment removal) (0.3% of construction cost)	\$371 - \$742 for sediment removal Erickson et al 2010	5% - 20% of construction cost USEPA 1999	5.1% - 126.0% of construction cost Weiss et al 2005	10% - 15% of construction cost Alberta 1999
1500m ² parking lot + 500m ² roof runoff	Annual maintenance	\$1277 (all for sediment removal) (2.8% of construction cost)	\$370 - \$740 for sediment removal Erickson et al 2010	5% - 20% of construction cost USEPA 1999	5.1% - 126.0% of construction cost Weiss et al 2005	10% - 15% of construction cost Alberta 1999
Infiltration Chamber 2000m ² roof runoff	Annual maintenance	\$74	Lit review not done.			
1500m ² parking lot + 500m ² roof runoff	Annual maintenance	\$1212	This cost is mainly to clean out the hydrodynamic separator. See lit review for hydrodynamic separator below.			
Hydrodynamic separator	Annual maintenance	\$1200	\$420 - \$1,400 WERF 2005	\$1,027 Olson et al 2010	\$1,800 USEPA 1999	
	Rehabilitation	assumed to last 50	120% of initial cost after 25 years			

LID	Maintenance or rehabilitation	Cost in this study	Literature values	
		years	BMP-REALCOST 2010	
Rainwater Harvesting	Annual maintenance	\$744	\$815 - \$13K	
	Rehabilitation	\$5970 plastic tank replacement at year 40	<i>WERF does not replace tank.</i>	
Green Roof	Annual maintenance	\$1,714 - \$2,022	WERF 2009	
	First 2 years	\$640 - \$26,620	\$2.7 - \$44/m2 (\$5.4K - \$88K) GRHC 2006	\$13-\$21/m2 (\$26K - \$42K) CMHC 2003
	Rehabilitation	\$342K - \$617K	No lit review done on rehab costs.	

5.0 CONCLUSIONS

This project has employed a robust and replicable methodology to compile capital and life cycle costs for a number of the most common low impact development practices. Results show the costs associated with construction, maintenance and rehabilitation of each practice. The broader public benefits and avoided infrastructure costs associated with applying the practices are not documented as these will vary depending on a range of factors specific to each site. Combining the cost data collected in this project with these more site specific considerations will help guide decisions on the type and combination of low impact development practices best suited to each site.

Model LID practice design costs documented in this study indicate that bioretention, infiltration chambers, infiltration trenches and enhanced swales are some of the least expensive practices to implement when only the practice cost itself is considered. The practice of rainwater harvesting provides added savings by reducing the cost of potable water supplies. Permeable pavements are comparably more expensive than most other practices, but in many instances these costs would be offset to some extent by a reduction in the need to pave the drainage area, since the pavements serve both as a parking surface and stormwater treatment practice. The practice also does not require as much land as some other practices, making it particularly well suited to retrofit contexts. Green roofs are by far the most expensive practice as they are installed in less accessible locations and need to be carefully engineered to protect the integrity of the building envelope. This practice is often selected because of its aesthetic, biodiversity and energy saving benefits, as well as its overall contribution to green building rating schemes, the value of which were not considered in our cost assessment. The costs and benefits of green roofs are best assessed in relation to those of conventional roofs over long time periods to capture the cost savings associated with the longer life of green roofs (see, for example, STEP, 2007).

Just as green roofs replace conventional roofs, other LID practices supplant more conventional treatment practices. An analysis of different treatment scenarios for an asphalt parking lot revealed that LID practices had comparable life cycle costs to conventional treatment using an oil grit separator (OGS). When the treatment costs of the scenarios were expressed in relation to the superior water quality benefits of LID, the life cycle costs of the LID practices were between 35 and 77% less expensive than conventional OGS treatment.

Capital and life cycle costs generated through this project have been scaled and programmed into a spreadsheet decision support tool for each practice that allows users to input site design information (e.g drainage area size and type) and alter unit costs in order to generate estimates of overall practice costs based on site specific data and considerations. This tool is available on the Sustainable Technologies Evaluation Program website at www.sustainabletechnologies.ca.

6.0 REFERENCES

Alberta Environmental Protection (Municipal Program Development Branch, Environmental Sciences Division, Environmental Service). Jan. 1999. "Stormwater Management Guidelines for the Province of Alberta." <<http://environment.gov.ab.ca/info/library/6786.pdf>>

Atlanta Regional Commission. Georgia Stormwater Management Manual Vol. 2: Technical Handbook. Aug. 2001.

Bass, Brad (Researcher). 2012. Environment Canada, Adaptation and Impacts Research, Personal Communication, April 5, 2012

Brown, Whitney & Thomas Schueler. 1997 "The Economics of Stormwater BMPs in the Mid-Atlantic Region. Center for Watershed Protection for Chesapeake Research Consortium. Silverspring, Maryland.

Buckley, M. Souhlas, T. and Hollingshead, A. 2012a. Economic Benefits of Green Infrastructure: Great Lakes Region, ECONorthwest.

Buckley, M. Souhlas, T. and Hollingshead, A. 2012b. Economic Benefits of Green Infrastructure: Chesapeake Bay Region, ECONorthwest.

Clayton, R. A. and T.R. Schueler (The Center for Watershed Protection). 1996. Design of Stormwater Filtering Systems. prepared for Chesapeake Research Consortium, Inc. Maryland.

CMHC (Peck, Steven and Monica Kuhn). 2003. "Design Guidelines for Green Roofs." <<http://www.cmhc.ca/en/inpr/bude/himu/coedar/loader.cfm?url=/commonspot/security/getfile.cfm&PageID=70146>>

Drake, J., Bradford, A., Van Seters, T. 2012. Evaluation of Permeable Pavements in Cold Climates – Kortright Centre, Vaughan. A project conducted under the Sustainable Technologies Evaluation Program, Toronto and Region Conservation.

Erickson, A.J., J.S. Gulliver, J. Kang, P. Weiss, and C.B. Wilson. 2010. Maintenance for Stormwater Treatment Practices. *Journal of Contemporary Water Research & Education*. Issue 146: pp 75-82.

Green Roofs for Healthy Cities (GRHC). 2006. "Green Roof Design 101, Introductory Course, Participant's Manual." Second Edition.

Interlocking Concrete Pavement Institute (ICPI). 2011. Life Cycle Cost Analysis Spreadsheet

J.F. Sabourin and Associates Incorporated. 2008a. *20 Year Performance Evaluation of Grassed Swale and Perforated Pipe Drainage Systems*. Project No. 524(02). Prepared for the Infrastructure Management Division of the City of Ottawa. Ottawa, ON.

Lawson, Sarah. A Planning Framework for Low Impact Development (LID) in Stormwater Management – an Ontario Perspective." thesis for Ryerson University. Toronto. 2010.

Marbek, 2010. Assessing the Economic Value of Protecting the Great Lakes: Rouge River Case Study for Nutrient Reduction and Nearshore Health Protection. Ottawa, Ontario.

Minnesota Stormwater Steering Committee. Minnesota Stormwater Manual Version 2. Minnesota Pollution Control Agency. Jan. 2008.

Odefey, J. Detwillery, S., Rousseau, K., Trice, A., Blackwell, R. O'Hara, K., Buckley, M., Souhlas, T., Brown, S. Raviprakash, P. 2012. *Banking on Green: A look at how green infrastructure can save municipalities money and provide economic benefits community-wide*. American Rivers, American Society of Landscape Architects, EcoNorthwest, Water Environment Federation.

Olson, C., L.A.Roesner, B. Urbonas, K. Mackenzie. Aug. 2010. "BMP-REALCOST Best Management Practices – Rational Estimation of Actual Likely Costs of Stormwater Treatment - A Spreadsheet Tool for Evaluating BMP Effectiveness and Life Cycle Costs - User's Manual and Documentation Version 1.0." <http://www.udfcd.org/downloads/down_software.htm>. accessed Oct. 2011.

Pennsylvania Department of Environmental Protection. 2006. Pennsylvania Stormwater Best Management Practices Manual.

Prince George's County, Maryland (Dept. of Environmental Services). Bioretention Manual. Revised Dec. 2007.

RSMMeans. RSMMeans Cost Data 2010. Kingston, Massachusetts.

Stormtech, 2011. personal communication with Jacob Horvath, Nov/Dec. 2011.

Sustainable Technologies Evaluation Program. 2011. Rainwater Harvesting Design and Costing Tool. Toronto and Region Conservation Authority.

Sustainable Technologies Evaluation Program, 2010a. Performance Evaluation of Rainwater Harvesting Systems, Toronto, Ontario. Toronto and Region Conservation Authority.

Sustainable Technologies Evaluation Program, 2010b. Performance Evaluation of a Bioretention System, Earth Rangers, Vaughan. Toronto and Region Conservation Authority.

Sustainable Technologies Evaluation Program, 2008. Review of the Science and Practice of Stormwater Infiltration. Toronto and Region Conservation Authority

Sustainable Technologies Evaluation Program. 2007. An Economic Analysis of Green Roofs. Evaluating the costs and savings to building owners in Toronto and surrounding regions. Toronto and Region Conservation Authority.

SWAMP. 2004. Performance Assessment of a Perforated Pipe Stormwater Exfiltration System, Toronto, Ontario. Toronto and Region Conservation Authority.

Toronto and Region Conservation Authority and Credit Valley Conservation Authority. 2010 Low Impact Development Stormwater Management Planning and Design Guide. Version 1.0. Toronto.

Toronto and Region Conservation Authority, 2009. Don River Watershed Management Plan, TRCA, Toronto.

Toronto and Region Conservation Authority, 2008. Humber River Watershed Management Plan, TRCA, Toronto.

Toronto and Region Conservation Authority, 2007. Rouge River Watershed Management Plan, TRCA, Toronto.

U.S. Environmental Protection Agency (USEPA). 1999. "Preliminary data summary of urban stormwater best management practices." EPA-821-R-99-012. Washington, D.C.

Water Environment Research Foundation (WERF). 2009. BMP and LID Whole Life Cost Models Version 2.0. (spreadsheets)

Water Environment Research Foundation (WERF). Critical Assessment of Stormwater Treatment and Control Selection Issues – Final Report. 2005. Co-published by IWA Publishing.

Weiss, P.T., J.S. Gulliver, and A.J. Erickson. 2005. "The Cost and Effectiveness of Stormwater Management Practices." St. Paul, Minnesota: Minnesota Dept. of Transportation.

Weiss, P.T., J.S. Gulliver, A.J. Erickson. 2007. Cost and Pollutant Removal of Storm-Water Treatment Practices. *Journal of Water Resources Planning and Management*. Volume 133, No. 2: pp 218-228.

Wisconsin Department of Natural Resources. Conservation Practice Standard 1004. Nov. 2010.

Wylie, Scott. 2012. Wytech Building Envelope Solutions, personal communication, Apr. 9, 2012.

APPENDIX A:

Detailed Costing Tables

Table A.1: Bioretention

Item	Item detail	RSMMeans Unit Cost (2010\$CND)	Unit cost (\$CND) - other source	Units	Full Infiltration	Partial Infiltration	No Infiltration	Assumptions/Notes
Site Investigation								
Dig test pit 1	80 HP backhoe (equipment)	\$24.88		\$/m ³	\$29.86	\$99.52	\$0.00	Test pit is 1 m x 1 m x 1.275 m, volume is 1.2 m ³ (For Full) Test pit is 1m x 1m x 2m and sloped 1:1 above 1.2 m depth, volume is ~ 4 m ³ (For Partial) RSMMeans costs for light & heavy soil were averaged.
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$59.50	\$198.32	\$0.00	
Dig test pit 2	80 HP backhoe (equipment)	\$24.88		\$/m ³	\$29.86	\$99.52	\$0.00	Test pit is 1 m x 1 m x 1.275 m, volume is 1.2 m ³ (For Full) Test pit is 1m x 1m x 2m and sloped 1:1 above 1.2 m depth, volume is ~ 4 m ³ (For Partial) RSMMeans costs for light & heavy soil were averaged.
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$59.50	\$198.52	\$0.00	
Infiltration tests	Double-ring infiltrometer	\$608.85		\$/test	\$2,435.40	\$2,435.40	\$0.00	2 infiltration tests per test pit
Site Preparation								
Preconstruction meeting	Part of overhead				\$0.00	\$0.00	\$0.00	
Stakeout of utilities			\$500.00	lump sum	\$500.00	\$500.00	\$500.00	Assume no interfering utilities are found as a result.
Tree & plant protection					\$0.00	\$0.00	\$0.00	Assume no trees
Traffic control					\$0.00	\$0.00	\$0.00	Assume not required
Install erosion & sediment control and control drainage	2" submersible gas pump for 3 days (incl. gas)		\$81.15	\$/day	\$243.45	\$243.45	\$0.00	
	Silk sack in catchbasin		\$65.00	\$/unit	\$65.00	\$65.00	\$0.00	
	Silt fence 1m around perimeter of excavation		\$2.21	\$/m	\$179.01	\$191.47	\$0.00	
	Silt fence labour		\$1.77	\$/m	\$143.37	\$153.35	\$0.00	
Mobilization/demobilization					\$0.00	\$0.00	\$0.00	Active construction site, so all equipment on site
Excavation								
Vegetation removal	Clearing, grubbing, haul away material				\$0.00	\$0.00	\$0.00	Active construction site, assume already done
Topsoil salvage - Stockpile & stabilize					\$0.00	\$0.00	\$0.00	Assume 6" of topsoil is already removed
Excavation	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.24		\$/Bm ³	\$181.35	\$330.46	\$245.94	Assume excavation rate of 100 Bm ³ /hr (188 Lm ³ /hr) Excavation is sloped 1:1 above 1.2 m depth
	1.5 m ³ bucket excavator (equipment)	\$1.89		\$/Bm ³	\$276.41	\$503.69	\$374.86	
	Loading - 15 % of excavation cost			%	\$68.66	\$125.12	\$93.12	
	Hauling in a 13.76 m ³ truck (including truck & driver)	\$172.92		\$/hr/truck	\$760.85	\$1,400.65	\$1,037.52	
Safety fencing	4' high fencing, 6 m around perimeter of excavation (124 m). Assume 1 week rental.	\$800.00		\$/week	\$800.00	\$800.00	\$800.00	Includes setup & takedown
Material and Installation								
Impermeable membrane	0.762 mm HDPE liner (materials)	\$4.96		\$/m ²	\$0.00	\$0.00	\$1,602.08	Assume membrane extends 1 m beyond edges
	0.762 mm HDPE liner (labour)	\$9.32		\$/m ²	\$0.00	\$0.00	\$3,010.36	Adjusted RSMMeans labour cost of \$8.88 by +5% to \$9.32 because of the smaller quantity.

Item	Item detail	RSMans Unit Cost (2010\$CND)	Unit cost (\$CND) - other source	Units	Full Infiltration	Partial Infiltration	No Infiltration	Assumptions/Notes
HDPE Piping	Underdrain 200 mm diameter, perforated (material)		\$16.87	\$/m	\$0.00	\$548.28	\$548.28	Pipe materials used: Armtec Boss 2000, corrugated with smooth inner wall
	Underdrain 200 mm diameter, perforated (labour)	\$11.40		\$/m	\$0.00	\$370.50	\$370.50	
	Clean out pipes, 150 mm diameter (material)		\$9.84	\$/m	\$0.00	\$27.06	\$27.06	
	Clean out pipes, 150 mm diameter (labour)	\$10.86		\$/m	\$0.00	\$29.87	\$29.87	
	Overflow pipe 200 mm diameter (material)		\$15.37	\$/m	\$19.60	\$31.59	\$25.36	
	Overflow pipe 200 mm diameter (labour)	\$11.40		\$/m	\$14.54	\$23.43	\$18.81	
	Pipe to sewer - 200 mm diameter (trenching not incl.) (material)		\$15.37	\$/m	\$76.85	\$76.85	\$76.85	
	Pipe to sewer - 200 mm diameter (trenching not incl.) (labour)	\$11.40		\$/m	\$57.00	\$57.00	\$57.00	
	Monitoring pipes - 150 mm diameter, perforated (material)		\$10.94	\$/m	\$27.90	\$44.96	\$0.00	
	Monitoring pipes - 150 mm diameter, perforated (labour)	\$10.86		\$/m	\$27.69	\$44.63	\$0.00	
	Delivery for all pipes		\$50 to \$100	\$/delivery	\$50.00	\$100.00	\$100.00	
	HDPE Pipe Fittings	Underdrain: 3 end caps (200 mm)	\$5.40		\$/unit	\$0.00	\$16.20	
Underdrain: 3 Tees (200 mm)		\$107.58		\$/unit	\$0.00	\$322.74	\$322.74	
Cleanouts: 2 caps at surface (150 mm)		\$65.00		\$/unit	\$0.00	\$130.00	\$130.00	
Cleanouts: 2 Tees (150 mm to 200 mm)		\$107.58		\$/unit	\$0.00	\$215.16	\$215.16	
Overflow pipe: 1 inlet guard (200 mm)		\$25.00		\$/unit	\$25.00	\$25.00	\$25.00	
Overflow pipe: 1 Tee		\$107.58		\$/unit	\$107.58	\$107.58	\$107.58	
Overflow Pipe: with 1 endcap/footplate (200 mm)		\$5.40		\$/unit	\$5.40	\$5.40	\$5.40	
Pipe to sewer: 1 manhole adaptor (200 mm)		\$36.38		\$/unit	\$36.38	\$36.38	\$36.38	
Monitoring pipes: 2 caps at surface (150 mm)		\$65.00		\$/unit	\$130.00	\$130.00	\$0.00	
Monitoring pipes: 2 footplates		\$5.40		\$/unit	\$10.80	\$10.80	\$0.00	
Pipe fittings: Labour	Underdrain: 3 end caps (200 mm)	\$50.00		\$/unit	\$0.00	\$150.00	\$150.00	
	Underdrain: 3 Tees (200 mm)	\$100.00		\$/unit	\$0.00	\$300.00	\$300.00	
	Cleanouts: 2 caps at surface (150 mm)	\$50.00		\$/unit	\$0.00	\$100.00	\$100.00	
	Cleanouts: 2 Tees (150 mm to 200 mm)	\$100.00		\$/unit	\$0.00	\$200.00	\$200.00	
	Overflow pipe: 1 inlet guard (200 mm)	\$50.00		\$/unit	\$50.00	\$50.00	\$50.00	
	Overflow pipe: 1 Tee	\$100.00		\$/unit	\$100.00	\$100.00	\$100.00	
Overflow Pipe: with 1 endcap/footplate (200 mm)	\$50.00		\$/unit	\$50.00	\$50.00	\$50.00		

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost (\$CND) - other source	Units	Full Infiltration	Partial Infiltration	No Infiltration	Assumptions/Notes
	Pipe to sewer: 1 manhole adapter (200 mm)	\$200.00		\$/unit	\$200.00	\$200.00	\$200.00	
	Monitoring pipes: 2 caps at surface (150 mm)	\$50.00		\$/unit	\$100.00	\$100.00	\$0.00	
	Monitoring pipes: 2 footplates	\$50.00		\$/unit	\$100.00	\$100.00	\$0.00	
Pipe to sewer trenching	Pipe trenching & backfill, 0.6 m wide, 1.2 m deep, no slope	\$15.59		\$/m	\$77.95	\$77.95	\$77.95	This trench depth was chosen assuming the site is excavated already a certain amount for asphalt paving
	Pipe bedding, 0.6 m wide	\$13.18		\$/m	\$65.90	\$65.90	\$65.90	
Stone	50 mm clear		\$36.00	\$/Cm ³	\$0.00	\$3,182.40	\$1,287.00	
	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.24		\$/m ³	\$0.00	\$109.62	\$44.33	Assumed cost is similar to cost of excavation (100 m ³ /hr)
	1.5 m ³ bucket excavator (equipment)	\$1.89		\$/m ³	\$0.00	\$167.08	\$67.57	
Pea Gravel	Material	\$56.10		\$/m ³	\$0.00	\$729.30	\$729.30	
	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.24		\$/m ³	\$0.00	\$16.12	\$16.12	
	1.5 m ³ bucket excavator (equipment)	\$1.89		\$/m ³	\$0.00	\$24.57	\$24.57	
	Level by hand	\$1.56		\$/m ²	\$0.00	\$202.80	\$202.80	
Geotextile	Material	\$3.10		\$/m ²	\$24.80	\$145.70	\$139.50	
	Labour	\$0.40		\$/m ²	\$3.20	\$18.80	\$18.00	
Filter media	Gro-Bark (material)		\$41.40	\$/Lm ³	\$6,727.50	\$6,727.50	\$6,727.50	
	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.24		\$/Cm ³	\$161.20	\$161.20	\$161.20	Assumed cost is similar to cost of excavation (100 m ³ /hr)
	1.5 m ³ bucket excavator (equipment)	\$1.89		\$/Cm ³	\$245.70	\$245.70	\$245.70	
	Delivery	\$900.00		\$/delivery	\$900.00	\$900.00	\$900.00	
Backfill sides of excavation	80 HP dozer + 0.5 labourer (labour)	\$1.02		\$/Lm ³	\$0.00	\$26.72	\$4.69	Assume swell factor of 25% and compaction factor of 0.9 (US Army 2000*)
	80 HP dozer (equipment)	\$0.66		\$/Lm ³	\$0.00	\$17.29	\$3.04	
Curbs & gutter with curb inlets	150 mm high curb, 150 mm thick gutter & 600 mm wide	\$88.35		\$/m	\$6,449.55	\$6,449.55	\$6,449.55	Assume cost is same with or without inlets
	Labour	\$26.26		\$/m	\$1,916.98	\$1,916.98	\$1,916.98	
Vegetation	Mixture of Shrubs, grasses & broadleaf/herb. Includes delivery and labour	\$50.20		\$/m ²	\$4,367.40	\$4,367.40	\$4,367.40	Assume vegetation covers 2/3 of cell area = 87 m ² Assume 50% shrub, 40% grasses, 10% broadleaf/herb.
Wood mulch	75 mm deep (material)	\$2.72		\$/m ²	\$337.28	\$337.28	\$337.28	130 m ² area minus 6 m ² stone inlets
	Labour	\$5.18		\$/m ²	\$642.32	\$642.32	\$642.32	
Stone inlets	50 mm clear (material)		\$36.00	\$/Cm ³	\$21.60	\$21.60	\$21.60	Assume 50 mm clear, 100 mm deep. Area = 0.5 m ² x 12 inlets = 6 m ² . Vol = 6 m ² x 100 mm = 0.6 m ³ .
	Spread by hand (labour)	\$173.11		\$/m ³	\$103.87	\$103.87	\$103.87	
SUBTOTAL					\$29,066	\$37,706	\$35,480	
Fees								
Project Overhead		10%		% of subtotal	\$2,906.62	\$3,770.55	\$3,548.02	
TOTAL					\$31,973	\$41,476	\$39,028	

Table A2. Permeable pavement

Item	Item detail	RSMean Unit Cost (2010\$CND)	Unit cost (2010\$CND) - other source	Units	Full Infiltration	Partial Infiltration	No Infiltration	Assumptions/Notes
Site Investigation								
Dig test pit 1	80 HP backhoe (equipment)	\$24.88		\$/m ³	\$10.70	\$10.70	\$0.00	Assume test pit is 1 m x 1 m x 430 mm. RSMean costs for light & heavy soil were averaged.
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$21.32	\$21.32	\$0.00	
Dig test pit 2	80 HP backhoe (equipment)	\$24.88		\$/m ³	\$10.70	\$10.70	\$0.00	
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$21.32	\$21.32	\$0.00	
Infiltration tests		\$608.85		\$/test	\$2,435.40	\$2,435.40	\$0.00	2 infiltration tests per test pit.
Soil strength testing					\$0.00	\$0.00	\$0.00	Not costed; assumed geotech tests done previously
Soil quality testing					\$0.00	\$0.00	\$0.00	Not costed; assumed soil dumped elsewhere on site
Site Preparation								
Pre-construction meeting	Part of overhead				\$0.00	\$0.00	\$0.00	
Stakeout of utilities			\$500.00	lump sum	\$500.00	\$500.00	\$500.00	
Erosion and sediment controls	8" dia. FilterSoxx along 60 m edge along asphalt		\$10.00	\$/m	\$600.00	\$600.00	\$0.00	Assume items already on site, labour negligible
Mobilization/demobilization					\$0.00	\$0.00	\$0.00	Active construction site, so all equipment on site
Excavation								
Vegetation removal	Clearing, grubbing, haul away material				\$0.00	\$0.00	\$0.00	Active construction site, assume already done
Topsoil salvage, haul to stockpile	6" removed, 60 m travel to stockpile, 200 HP dozer + 0.5 labourer (labour)	\$1.15		\$/m ³	\$175.26	\$175.26	\$175.26	
	6" removed, 60 m travel to stockpile, 200 HP dozer (equipment)	\$2.06		\$/m ³	\$313.94	\$313.94	\$313.94	Assumed this equipment is not too heavy
Excavate	1.5 m ³ bucket excavator + 1 labourer, productivity 100 Bm ³ /hr (labour)	\$1.24		\$/Bm ³	\$347.20	\$347.20	\$347.20	Assumed 100 Bm ³ /hr as for Bioretention excavation. Assumed 6" of topsoil has already been removed, so do not need to excavate full depth.
	1.5 m ³ bucket excavator, productivity 100 Bm ³ /hr (equipment)	\$1.89		\$/Bm ³	\$529.20	\$529.20	\$529.20	
	Loading - 15% of excavation cost			%	\$131.46	\$131.46	\$131.46	
	Hauling in a 13.76 m ³ truck (including truck & driver)	\$172.92		\$/hr	\$1,466.36	\$1,466.19	1466.36	Assumed swell factor of 25%, cycle time of 20 min.
Compaction of native soil	30,000 lb grader + 25T vibratory roller + 1 labourer (labour)	\$1.16		\$/m ²	\$1,160.00	\$1,160.00	\$1,160.00	Assumed this equipment is not too heavy
	30,000 lb grader + 25T vibratory roller (equipment)	\$1.14		\$/m ²	\$1,140.00	\$1,140.00	\$1,140.00	
	Proctor test	\$149.45		\$/test	\$149.45	\$149.45	\$149.45	1 test required
	Nuclear density test	\$42.81		\$/test	\$171.24	\$171.24	\$171.24	Average of 4 tests required - test is done to check compaction.
Materials and Installation								
Impermeable membrane	0.762 mm HDPE liner (materials)	\$4.96		\$/m ²	\$0.00	\$0.00	\$5,720.86	Assume membrane extends 0.57 m beyond edges
	0.762 mm HDPE liner - 3 skilled workers (labour)	\$9.32		\$/m ²	\$0.00	\$0.00	\$10,745.96	
Geotextile	Polypropylene filtration fabric (materials)	\$3.10		\$/m ²	\$3,100.00	\$3,100.00	\$0.00	
	Polypropylene filtration fabric - 2 labourers	\$0.40		\$/m ²	\$400.00	\$400.00	\$0.00	

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost (2010\$CND) - other source	Units	Full Infiltration	Partial Infiltration	No Infiltration	Assumptions/Notes
	(labour)							
HDPE Piping	Underdrain, 150 mm diameter perforated pipe (materials)		\$10.94	\$/m	\$0.00	\$176.13	\$176.13	Piping materials used: HDPE Armtec Boss 2000 pipe
	Underdrain, 150 mm diameter perforated pipe (labour)	\$10.86		\$/m	\$0.00	\$174.85	\$174.85	
	Clean out pipes, 100 mm diameter pipe (materials)		\$4.79	\$/m	\$0.00	\$1.10	\$1.34	
	Clean out pipes, 100 mm diameter pipe (labour)	\$10.19		\$/m	\$0.00	\$2.34	\$2.85	
	Pipe to catchbasin, 150 mm diameter pipe (materials)		\$9.84	\$/m	\$0.00	\$4.92	\$4.92	
	Pipe to catchbasin, 150 mm diameter pipe (labour)	\$10.86		\$/m	\$0.00	\$5.43	\$5.43	
	Monitoring pipes, 150 mm diameter, perforated pipe (materials)		\$10.94	\$/m	\$4.70	\$4.70	\$0.00	
	Monitoring pipes, 150 mm diameter, perforated pipe (labour)	\$10.86		\$/m	\$4.67	\$4.67	\$0.00	
	Delivery (total for all pipe)		\$50.00	n/a	\$0.00	\$50.00	\$50.00	Assume delivery negligible
Pipe fittings	Underdrain 150 mm end cap (materials)		\$2.92	\$/ea.	\$0.00	\$2.92	\$2.92	
	Underdrain 150 mm end cap (labour)	\$50.00		\$/ea.	\$0.00	\$50.00	\$50.00	From looking at RSMeans, assumed \$50 for simplicity
	Underdrain 150 mm coupler (materials)		\$1.50	\$/ea.	\$0.00	\$1.50	\$1.50	
	Underdrain 150 mm coupler (labour)	\$50.00		\$/ea.	\$0.00	\$50.00	\$50.00	From looking at RSMeans, assumed \$50 for simplicity
	Clean-out 100 mm surface cap (materials)		\$65.00	\$/ea.	\$0.00	\$65.00	\$65.00	Cast iron cap, assumed to be suitable as a surface cap.
	Clean-out 100 mm surface cap (labour)	\$50.00		\$/ea.	\$0.00	\$50.00	\$50.00	From looking at RSMeans, assumed \$50 for simplicity
	Clean-out tee, 100 - 150 mm (materials)		\$111.65	\$/ea.	\$0.00	\$111.65	\$111.65	
	Clean-out tee, 100 - 150 mm (labour)	\$100.00		\$/ea.	\$0.00	\$100.00	\$100.00	Assumed \$100 from RSMeans cost of \$189 for a 300 mm tee.
	Manhole adapter for pipe to catchbasin, 150 mm (materials)		\$27.58	\$/ea.	\$0.00	\$27.58	\$27.58	
	Manhole adapter for pipe to catchbasin, 150 mm (labour)		\$200.00	\$/ea.	\$0.00	\$200.00	\$200.00	
	Monitoring pipe 150 mm surface cap (materials)		\$65.00	\$/ea.	\$65.00	\$65.00	\$0.00	
	Monitoring pipe 150 mm surface cap (labour)	\$50.00		\$/ea.	\$50.00	\$50.00	\$0.00	From looking at RSMeans, assumed \$50 for simplicity
	Monitoring pipe footplate (materials)		\$2.92	\$/ea.	\$2.92	\$2.92	\$0.00	Assumed same cost as an end-cap
	Monitoring pipe footplate (labour)	\$50.00		\$/ea.	\$50.00	\$50.00	\$0.00	From looking at RSMeans, assumed \$50 for simplicity
Flow restrictor	Canpipe 4" ball valve (materials)		\$93.90	\$/ea.	\$0.00	\$93.90	\$0.00	
	Canpipe 4" ball valve (labour)	\$50.00		\$/ea.	\$0.00	\$50.00	\$0.00	From looking at RSMeans, assumed \$50 for simplicity
Sub-base, 200 mm deep	50 mm clear limestone (materials)		\$36.00	\$/m ³	\$7,200.00	\$7,200.00	\$7,200.00	
	30,000 lb grader, 1.5 cy front-end loader, 300 HP dozer, 25 T vibratory roller, truck tractor & water tank trailer + 1 labour foreman + 2 labourers (labour)	\$1.00		\$/m ²	\$1,000.00	\$1,000.00	\$1,000.00	Assume equip not too heavy. These RSMeans costs are for 40 mm stone. Assume costs for 50 mm stone is same.
	30,000 lb grader, 1.5 cy front-end loader, 300	\$1.29		\$/m ²	\$1,290.00	\$1,290.00	\$1,290.00	

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost (2010\$CND) - other source	Units	Full Infiltration	Partial Infiltration	No Infiltration	Assumptions/Notes
	HP dozer, 25 T vibratory roller, truck tractor & water tank trailer (equipment)							
Base, 100 mm deep	19 mm clear limestone (materials)		\$37.00	\$/m ³	\$3,700.00	\$3,700.00	\$3,700.00	
	30,000 lb grader, 300 HP dozer, 25 T vibratory roller, truck tractor & water tank trailer + 1 labour foreman (labour)	\$0.58		\$/m ²	\$580.00	\$580.00	\$580.00	Assume equipment not too heavy
	30,000 lb grader, 300 HP dozer, 25 T vibratory roller, truck tractor & water tank trailer (equipment)	\$1.01		\$/m ²	\$1,010.00	\$1,010.00	\$1,010.00	Assume equipment not too heavy
Compaction test	Nuclear density test	\$42.81		\$/test	\$85.62	\$85.62	\$85.62	Assume 2 tests
Concrete curb along 1 edge	150 mm wide, 450 mm deep, cast-in-place (materials)	\$40.64		\$/m	\$2,438.40	\$2,438.40	\$2,438.40	Assume 450 mm is sufficient to extend to the sub-base of adjacent asphalt pavement.
	150 mm wide, 450 mm deep, cast-in-place - 1 carpenter foreman, 4 carpenters, 1 labourer (labour)	\$19.69		\$/m	\$1,181.40	\$1,181.40	\$1,181.40	
Plastic edge restraints along 3 edges	Snapedge® (materials)		\$4.76	\$/m	\$444.58	\$444.58	\$444.58	Delivery negligible because the product is very light and can be transported with the pavers.
	Snapedge® spikes (materials)		\$1.03	\$/m	\$96.20	\$96.20	\$96.20	Need 5 spikes per 8 ft (2.44 m) of Snapedge. Spikes are \$0.50 each.
	Snapedge® (labour)		\$1.80	\$/m	\$168.12	\$168.12	\$168.12	\$52.73/hr wage, 29.3 m can be installed per hour. Installation speed (96 ft/hr)
Bedding & Pavers	80 mm interlocking pavers (materials)		\$31.56	\$/m ²	\$31,560.00	\$31,560.00	\$31,560.00	Source is average of five prices
	Pavers (labour + equip only) and bedding including 50 mm layer and enough for void filling (materials, labour, equipment)		\$25.30	\$/m ²	\$25,300.00	\$25,300.00	\$25,300.00	Machine installation of pavers. Price is an average of range provided (\$21.30-\$29.30) for union labour.
Striping		\$0.46		\$/m	\$460.00	\$460.00	\$460.00	
SUBTOTAL					\$89,375	\$90,592	\$100,139	
Fees								
Project overhead		10.00%		% of sub total	\$8,937.52	\$9,059.23	\$10,013.94	
TOTAL					\$98,313	\$99,652	\$110,153	

Table A3: Infiltration trenches

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost - other source (\$CND)	Units	Roof Only	Road & Roof	Assumptions/Notes	
Site Investigation								
Dig test pit 1	80 HP backhoe (equipment)	\$24.88		\$/m ³	\$299.13	\$299.13	Assume test pit is 1m x 1m, 2.72m deep, & sloped 1:1 above 1.2m depth. RSMMeans costs for light & heavy soil were averaged.	
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$596.10	\$596.10		
Dig test pit 2	80 HP backhoe (equipment)	\$24.88		\$/m ³	\$299.13	\$299.13		
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$596.10	\$596.10		
Infiltration tests	Double-ring infiltrometer	\$608.85		\$/test	\$2,435.40	\$2,435.40	2 infiltration tests per test pit	
Soil strength testing							Not costed; assumed geotech tests done previously	
Soil quality testing							Not costed; assumed soil dumped elsewhere on site.	
Site Preparation								
Pre-construction meeting	Part of overhead							
Stakeout of utilities	Assume no interfering utilities found as a result		\$500.00	lump sum	\$500.00	\$500.00		
Erosion and sediment controls	Silt fence (material)	\$2.21		\$/m	\$112.71	\$112.71	Silt fence along one edge ~ 51 m (decided by Mariko Uda, similar to approach with Infiltration Chamber)	
	2 labourers	\$1.77		\$/m	\$90.27	\$90.27		
Mobilization/demobilization					\$0.00	\$0.00	Active construction site, so all equipment on site	
Excavation								
Vegetation removal	Clearing, grubbing, haul away material				\$0.00	\$0.00	Active construction site, assume already done	
Topsoil salvage, haul to stockpile	200 HP dozer (equipment)	\$2.06		\$/m ³	\$31.93	\$31.93	6" removed, 60 m travel to stockpile. 101.76 m ² x 152 mm = 15.5 m ³	
	200 HP dozer + 0.5 labourer (labour)	\$1.15		\$/m ³	\$17.83	\$17.83		
Excavate trench with trench box	Trench 2-3 m deep with trench box; 1.9 m ³ bucket excavator (equipment)	\$3.01		\$/Bm ³	\$787.12	\$787.12	Use a trench box. Assumed common earth. 1.9 m ³ bucket was used. Depth is 2.57m (2.72 m - 0.15 m topsoil already removed). Therefore vol = 2.57m x 101.76 m ² = 261.5 m ³	
	Trench 2-3 m deep with trench box; 1.9 m ³ bucket excavator + 1 labourer (labour)	\$1.39		\$/Bm ³	\$363.49	\$363.49		
	Loading - 15% of excavation cost			%	\$172.59	\$172.59		
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		\$/hr	\$1,017.39	\$1,017.39		Assumed some soil not hauled away (used later for backfill). Assumed swell factor of 25% and 20 min. cycle time to dump elsewhere on site. Assumed 13.76 m ³ truck size. Backfill (trench) = 660 mm x 2 m x 50.88 m = 67.162 Bm ³ ; Therefore vol to haul away = 261.5 Bm ³ - 67.2 Bm ³ = 194.3 Bm ³ .
Level subgrade	Assume this can be done as part of excavation step, as assumed with Infiltration Chambers							
Safety fencing	4' high, 1 week rental, including setup & takedown		\$251.75	lump sum	\$251.75	\$251.75	Assume 1m length of excavation left open (hole of 1 m x 2 m). Put a safety fence 1 m around it, total 14 m.	
Materials and Installation								
Manhole	Precast concrete, 4' dia., 8' deep (includes excavation of 34.2 Bm ³ , formed concrete footing, frame & cover, steps, compacted backfill) (material)	\$2,837.00		lump sum	\$2,837.00	\$2,837.00		
	Labour & equipment	\$2,354.00		lump sum	\$2,354.00	\$2,354.00		
	Loading excavated soil	15%		% of excavation cost	\$53.10	\$53.10		Used 15% of RSMeans cost for 4' dia., 8' deep manhole includes an excavation cost of \$354 to excavate 34.2 Bm ³
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		\$/hr	\$179.08	\$179.08		Assumed swell factor of 25% ("US Army 2000) and 20 min. cycle time to dump elsewhere on site. Assume all 34.2 Bm ³ excavated is hauled away
Geotextile - bottom, sides & top	Polypropylene filtration fabric (materials)	\$3.10		\$/m ²	\$1,162.50	\$1,162.50	Assumed this RSMeans line is suitable. Area = 101.8 m ² (top) + 101.8 m ² (bottom) + [2 sides x (50.88 m x 1.62 m deep)] + [2	

Item	Item detail	RSMMeans Unit Cost (2010\$CND)	Unit cost - other source (\$CND)	Units	Roof Only	Road & Roof	Assumptions/Notes
							ends x (2 m x 1.62 m deep)] = 375 m ² total area.
	2 labourers	\$0.40		\$/m ²	\$150.00	\$150.00	
Hydrodynamic Separator	Downstream Defender - 4' wide (material + delivery)		\$12,000.00	lump sum	\$0.00	\$12,000.00	
	Assume installation cost is roughly similar to that of 4' dia., 10' deep precast manhole in RSMMeans that includes 47.9 Bm ³ excavation (labour & equipment)	\$2,883.00		lump sum	\$0.00	\$2,883.00	
	Loading excavated soil	15%		% of excavation cost	\$0.00	\$432.45	
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		\$/hr	\$0.00	\$250.81	Assumed swell factor of 25% ("US Army 2000) and 20 min. cycle time to dump elsewhere on site. Assume all 47.9 Bm ³ excavated is hauled away
Pipe from separator to manhole - 2 m long trench	Trenching, common earth, no slope, 2' wide, 6' deep; 3/8 cy bucket hydraulic track-mounted backhoe, backfill, compaction, removal of spoil by truck; the backfill has been reduced to take into account pipe of suitable size & bedding (labour)	\$15.12		m	\$0.00	\$30.24	
	Equipment	\$5.58		m	\$0.00	\$11.16	
Pipe from separator to manhole - attach 300 mm pipe to both separator & manhole	Armtec Boss 2000 solid pipe, 300 mm dia. (material)		\$22.81	m	\$0.00	\$45.62	
	Armtec Boss 2000 solid pipe, 300 mm manhole adaptor (material)		\$46.00	ea.	\$0.00	\$46.00	
	Labour		\$200.00	ea.	\$0.00	\$400.00	Assume cost is similar to attaching pipe to separator
Pipe from separator to manhole - pipe bedding	Pipe bedding, side slope 0 to 1, 2' wide, pipe size 10" dia., compacted sand for bedding & 12" above pipe; pipe, trench, backfill not included (material)	\$8.22		m	\$0.00	\$16.44	
	Labour & equipment	\$5.10		m	\$0.00	\$10.20	
Attachment for pipe from parking lot to separator (do not cost pipe/trench, just cost out the attachment)	Labour		\$200.00	ea	\$0.00	\$200.00	Assume cost is similar to attaching pipe to a separator
Attachment for pipe from roof to control manhole (do not cost pipe/trench, just cost out the attachment)	Armtec Boss 2000 solid pipe, 200 or 300 mm manhole adaptor (material)		\$42.08	ea	\$42.08	\$42.08	Assume close to cost of 250 mm adaptor (\$42.08).
	Labour		\$200.00	ea	\$200.00	\$200.00	
Attach overflow pipe to control manhole (do not cost pipe/trench, just cost out the attachment)	Armtec Boss 2000 solid pipe, 300 mm manhole adaptor (material)		\$46.00	ea	\$46.00	\$46.00	
	Labour		\$200.00	ea	\$200.00	\$200.00	
Inlet pipe - attach pipe to both control manhole & perforated pipe	Armtec Boss 2000 solid pipe, 300 mm dia. (material)		\$22.81	m	\$6.84	\$6.84	
	Armtec Boss 2000 300 mm manhole adaptor (material)		\$46.00	ea	\$46.00	\$46.00	
	Labour to attach pipe to manhole		\$200.00	ea	\$200.00	\$200.00	
	Labour to attach pipe to perforated pipe		\$50.00	ea	\$50.00	\$50.00	
Perforated pipe	Armtec Boss 2000 perforated pipe, 300 mm dia. (material)		\$23.75	m	\$1,194.15	\$1,194.15	Assumed length of pipe is 2' (0.6m) less than length of trench.
	Installation of storm drainage piping, HDPE, 300 mm dia.; 1 foreman, 1 skilled labourer, 1 labourer (labour)	\$12.52		m	\$629.51	\$629.51	
	Armtec Boss 2000 end cap, belled (300 mm) (material)		\$11.46	ea	\$11.46	\$11.46	

Life Cycle Cost Assessment of Low Impact Development Practices

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost - other source (\$CND)	Units	Roof Only	Road & Roof	Assumptions/Notes
	Labour		\$50.00	ea	\$50.00	\$50.00	
Line inside of pipe with filter cloth & expandable rings	Assume polypropylene filtration fabric (materials)	\$3.10		\$/m ²	\$148.80	\$148.80	Assumed this RSMeans line is suitable. Area = 50.58 m (length) x 3.14 x 0.3 m = 48 m ² .
	Labour	\$50.00		hr	\$200.00	\$200.00	Assume 2 labourers, 2 hours
Monitoring wells	Armtec Boss 2000 perforated pipe, 150 mm dia. (material)		\$10.94	m	\$59.51	\$59.51	
	Installation of storm drainage piping, HDPE, 150 mm dia.; 1 foreman, 1 skilled labourer, 1 labourer (labour)	\$10.86		m	\$59.08	\$59.08	Each monitoring well is 2.72 m deep. There are 2 wells.
	Cast iron cap (assume these can be used for caps at surface)		\$65.00	ea	\$130.00	\$130.00	
	Labour		\$50.00	ea	\$100.00	\$100.00	
	Footplate - assume we can use Armtec Boss 2000 endcap, 150 mm dia., although this may be an underestimate.		\$2.92	ea	\$5.84	\$5.84	
	Labour		\$50.00	ea	\$100.00	\$100.00	
50 mm stone	50 mm clear limestone (materials)		\$36.00	\$/Cm ³	\$5,806.80	\$5,806.80	Calculated volume as follows: total vol = 101.8 m ² area x 1.62 m depth = 164.9 m ³ ; vol of pipe = π(0.15 m)(0.15 m) x 50.28 m = 3.6 m ³ ; thus, vol of stone req'd = 164.9 m ³ - 3.6 m ³ = 161.3 m ³ .
	1.5 m ³ excavator + 1 labourer (labour)	\$1.23		\$/Bm ³	\$198.40	\$198.40	Assumed cost to place stone is similar to that of excavating soil (100 Bm ³ /hr).
	1.5 m ³ excavator	\$1.88		\$/Bm ³	\$303.24	\$303.24	
Compact stone	21" wide walk-behind vibrating plate compactor, 2 passes, 12" lifts (equipment)	\$0.11		\$/Em ³	\$17.74	\$17.74	2 passes fine for light compaction and that with this stone hardly need compaction.
	21" wide walk-behind vibrating plate compactor, 2 passes, 12" lifts; 1 labourer (labour)	\$1.01		\$/Em ³	\$162.91	\$162.91	
Place fill	1.5 m ³ excavator + 1 labourer (labour)	\$1.23		\$/Bm ³	\$93.97	\$93.97	Assumed cost to place fill is similar to that of excavating soil (100 Bm ³ /hr). volume = 101.8 m ² x 0.750 m = 76.4 m ³ .
	1.5 m ³ excavator	\$1.88		\$/Bm ³	\$143.63	\$143.63	
Compact fill in 6" lifts to 95% Proctor density	Compact in 200 mm lifts with vibrating plate compactor (labour)	\$2.83		\$/Cm ³	\$216.21	\$216.21	Assume same cost whether compacted in 200 or 150 mm lifts.
	Equipment	\$0.25		\$/Cm ³	\$19.10	\$19.10	
	Proctor test	\$149.45		\$/test	\$149.45	\$149.45	1 test required
	Nuclear density test	\$42.81		\$/test	\$171.24	\$171.24	
SUBTOTAL					\$25,069	\$41,395	
Fees							
Project Overhead		10.00%		% of sub total	\$2,506.86	\$4,139.45	
TOTAL					\$27,575	\$45,534	

Table A4: Infiltration chambers

Item	Item detail	RSMean Unit Cost (2010\$CND)	Unit cost (CND) - other source	Units	Roof Only	Road & Roof	Assumptions/Notes
Site Investigation							
Dig test pit 1	80 HP backhoe (equipment)	\$24.88		\$/m ³	\$69.66	\$69.66	Assume test pit is 1 m x 1 m, 1.8 m deep, & sloped 1:1 above 1.2 m depth. RSMean costs for light & heavy soil were averaged.
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$138.82	\$138.82	
Dig test pit 2	80 HP backhoe (equipment)	\$24.88		\$/m ³	\$69.66	\$69.66	
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$138.82	\$138.82	
Infiltration tests	Double-ring infiltrometer	\$608.85		\$/test	\$2,435.40	\$2,435.40	2 infiltration tests per test pit
Soil strength testing							Not costed; assumed geotech tests done previously
Soil quality testing							Not costed; assumed soil dumped elsewhere on site
Site Preparation							
Pre-construction meeting	Part of Overhead				\$0.00	\$0.00	
Stakeout of utilities	Assume no interfering utilities found as a result		\$500.00	lump sum	\$500.00	\$500.00	
Erosion and sediment controls		\$2.21		\$/m	\$26.52	\$26.52	Silt fence along one edge ~ 12 m (decided by Mariko Uda & Lisa Rocha)
	2 labourers	\$1.77		\$/m	\$21.24	\$21.24	
Mobilization/demobilization	Active construction site, so all equipment on site				\$0.00	\$0.00	
Excavation							
Vegetation removal	Clearing, grubbing, haul away material				\$0.00	\$0.00	Active construction site, assume already done
Topsoil salvage, haul to stockpile	200 HP dozer + 0.5 labourer (labour)	\$1.15		\$/m ³	\$18.29	\$18.29	6" removed, 60 m travel to stockpile
	200 HP dozer (equipment)	\$2.06		\$/m ³	\$32.75	\$32.75	6" removed, 60 m travel to stockpile
Excavate	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.23		\$/Bm ³	\$223.86	\$223.86	Assumed common earth, 100 Bm ³ /hr (interpolated in RSMean), that depth is 1.65 m (1.8 m - 0.15 m topsoil already removed), that excavation is sloped 1:1 above 1.2 m depth.
	1.5 m ³ bucket excavator (equipment)	\$1.88		\$/Bm ³	\$342.16	\$342.16	
	Loading - 15% of excavation cost			%	\$84.90	\$84.90	
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		\$/hr	\$788.57	\$788.57	
Level subgrade	Can be done as part of excavation step.				\$0.00	\$0.00	
Safety fencing	4' high, 1 week rental, including setup & takedown).	\$650.00		lumpsum	\$650.00	\$650.00	Assume 6 m around perimeter of excavation -> total 89 m
Materials and Installation							
Manhole	Precast concrete, 4' dia., 8' deep (includes excavation of 34.2 Bm ³ , formed concrete footing, frame & cover, steps, compacted backfill) (material)	\$2,837.00		lump sum	\$2,837.00	\$2,837.00	This RSMean cost does not include hauling away excavated soil or pipe connections
	Labour and equipment	\$2,354.00		lump sum	\$2,354.00	\$2,354.00	
	Loading excavated soil - 15% of excavation cost			%	\$53.10	\$53.10	RSMean cost for 4' dia., 8' deep manhole includes an excavation cost of \$354 to excavate 34.2 Bm ³
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		\$/hr	\$179.08	\$179.08	Assumed swell factor of 25% (*US Army 2000), assumed 20 min. cycle time to dump elsewhere on site. Assume all 34.2 Bm ³ excavated is hauled away.
Hydrodynamic Separator	Downstream Defender - 4' wide (material + delivery)		\$12,000.00	lump sum	\$0.00	\$12,000.00	
	Assume installation cost is roughly similar to that of 4' dia., 10' deep precast manhole in RSMean that includes 47.9 Bm ³ excavation (labour & equipment)	\$2,883.00		lump sum	\$0.00	\$2,883.00	

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost (CND) - other source	Units	Roof Only	Road & Roof	Assumptions/Notes
	Loading excavated soil - 15% of excavation cost			%	\$0.00	\$432.35	
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		\$/hr	\$0.00	\$250.81	Assumed swell factor of 25% (*US Army), assumed 20 min. cycle time to dump elsewhere on site. Assume all 47.9 Bm ³ excavated is hauled away.
Pipe from separator to manhole - 2 m long trench	Trenching, common earth, no slope, 2' wide, 6' deep; 3/8 cy bucket hydraulic track-mounted backhoe, backfill, compaction, removal of spoil by truck; the backfill has been reduced to take into account pipe of suitable size & bedding (labour)	\$15.12		m	\$0.00	\$30.24	
	Equipment	\$5.58		m	\$0.00	\$11.16	
Pipe from separator to manhole - attach 300 mm pipe to both separator & manhole	Armtec Boss 2000 solid pipe, 300 mm dia. (material)		\$22.81	m	\$0.00	\$45.62	
	Armtec Boss 2000 solid pipe, 300 mm manhole adaptor (material)		\$46.00	ea.	\$0.00	\$46.00	
	Labour		\$200.00	ea.	\$0.00	\$400.00	
Pipe from separator to manhole - pipe bedding	Pipe bedding, side slope 0 to 1, 2' wide, pipe size 10" dia., compacted sand for bedding & 12" above pipe; pipe, trench, backfill not included (material)	\$8.22		m	\$0.00	\$16.44	
	Labour and equipment	\$5.10		m	\$0.00	\$10.20	
Attach pipe from parking lot to separator (do not cost pipe/trench, just cost out the attachment)	Labour		\$200.00	ea	\$0.00	\$200.00	
Attach pipe from roof to control manhole (do not cost pipe/trench, just cost out the attachment)	Armtec Boss 2000 solid pipe, 200 or 300 mm manhole adaptor (material)		\$42.08	ea	\$42.08	\$42.08	Assume close to cost of 250mm adaptor (\$42.08).
	Labour		\$200.00	ea	\$200.00	\$200.00	
Attach overflow pipe to control manhole (do not cost pipe/trench, just cost out the attachment)	Armtec Boss 2000 solid pipe, 300 mm manhole adaptor (material)		\$46.00	ea	\$46.00	\$46.00	
	Labour		\$200.00	ea	\$200.00	\$200.00	
Inlet pipe to chamber - attach pipe to both control manhole & chamber	Armtec Boss 2000 solid pipe, 600 mm manhole adaptor (material)		\$135.98	ea	\$135.98	\$135.98	
	Labour to attach pipe to manhole		\$200.00	ea	\$200.00	\$200.00	
	Labour to attach pipe to chamber		\$50.00	ea	\$50.00	\$50.00	
Geotextile surrounding stone	Polypropylene filtration fabric (materials)	\$3.10		\$/m ²	\$784.30	\$784.30	Assumed this RSMeans line is suitable for the Class 2 non-woven geotextile. Area = 104.7 m ² (top) + 104.7 m ² (bottom) + (41.186 m perimeter x 1.067 m deep) = 253 m ² total area. account.
	2 labourers	\$0.40		\$/m ²	\$101.20	\$101.20	
50 mm stone, 6" (152mm) deep, place	50 mm clear limestone (materials)		\$36.00	\$/Cm ³	\$572.76	\$572.76	
	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.23		\$/Bm ³	\$19.57	\$19.57	Assumed cost is similar to cost of excavation
	1.5 m ³ bucket excavator (equipment)	\$1.88		\$/Bm ³	\$29.91	\$29.91	
Level the stone	Hand grade select gravel (2 labourers)	\$1.56		\$/m ²	\$163.33	\$163.33	
Geotextile for scour protection	Polypropylene filtration fabric (materials)	\$3.10		\$/m ²	\$107.57	\$107.57	Assumed this RSMeans line is suitable for the Class 1 woven geotextile. Area = 3.8 m x 9.138 m = 34.7 m ² total area.
	2 labourers	\$0.40		\$/m ²	\$13.88	\$13.88	
Infiltration chambers & end caps	Stormtech SC-740 chambers		\$96.00	\$/m ³ storage	\$6,535.68	\$6,535.68	Material + delivery cost for chambers, endcaps, fittings, couplers, geotextile is approximately \$100/m ³ of storage. Exclude geotextile at approximately \$1/m ² . Estimated cost is \$96/m ³ of storage.
	2 labourers	\$50.00		\$/person-hr	\$100.00	\$100.00	Installation rate is 30 chambers per hour by 2 labourers
Geotextile for isolator row	Polypropylene filtration fabric (materials)	\$3.10		\$/m ²	\$0.00	\$162.13	Assumed this RSMeans line is suitable for the Class 1 & 2 woven geotextile. Class 1 geotextile on bottom: area = 1.4 m x 10.9 m x 2 layers = 30.5 m ² . Class 2 geotextile on top/sides: area = perimeter of half-circle of 2 m x 10.9 m = 21.8 m ² thus,

Item	Item detail	RSMans Unit Cost (2010\$CND)	Unit cost (CND) - other source	Units	Roof Only	Road & Roof	Assumptions/Notes
							total area of geotextile = 52.3 m ² .
	2 labourers	\$0.40		\$/m ²	\$0.00	\$20.92	
50mm stone, fill around chambers and 6" (152 mm) over top	50 mm clear limestone (materials)		\$36.00	\$/Cm ³	\$2,044.80	\$2,044.80	Calculated volume as follows: total vol = 104.7 m ² area x (1.067 m depth - 152 mm stone depth below) = 95.8 m ³ ; vol inside chambers = 30 chambers x 1.3 m ³ /chamber = 39 m ³ ; thus, vol of stone req'd = 95.8m ³ - 39 m ³ = 56.8 m ³ .
	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.23		\$/Bm ³	\$69.86	\$69.86	Assumed cost is similar to cost of excavation
	1.5 m ³ bucket excavator (equipment)	\$1.88		\$/Bm ³	\$106.78	\$106.78	
Level the stone	Hand grade select gravel (2 labourers)	\$1.56		\$/m ²	\$163.33	\$163.33	
Well-graded soil, 390 mm depth, compacted	Assume native soil on-site is suitable (so no material cost)				\$0.00	\$0.00	
	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.23		\$/Bm ³	\$50.18	\$50.18	Assumed cost is similar to cost of excavation. Compacted volume = 390 mm x 104.7 m = 40.8 Cm ³
	1.5 m ³ bucket excavator (equipment)	\$1.88		\$/Bm ³	\$76.70	\$76.70	
	Compact in 200 mm lifts with vibrating plate compactor (labour)	\$2.83		\$/Cm ³	\$115.46	\$115.46	Assume same cost whether compacted in 200 or 300mm lifts.
	Equipment	\$0.25		\$/Cm ³	\$10.20	\$10.20	
	Proctor test	\$149.45		\$/test	\$149.45	\$149.45	1 test required
	Nuclear density test	\$42.81		\$/test	\$171.24	\$171.24	Average of 4 tests required - test is done to check compaction.
SUBTOTAL					\$23,224	\$39,733.10	
Fees							
Project Overhead		10.00%		% of sub total	\$2,322.41	\$3,973.31	
TOTAL					\$25,547	\$43,706	

Table A5: Enhanced grass swale

Item	Item detail	RSMMeans Unit Cost (2010\$CND)	Unit cost - other source (\$CND)	Units	Curb check dam	Filter sock check dam	Rock check dam	Assumptions/Notes
Site Investigation								
Dig test pit 1	80 HP backhoe (equip)	\$24.88		\$/m ³	\$29.86	\$29.86	\$29.86	Test pit is 1m x 1m x 1.275m, therefore volume is ~ 1.2m ³ RSMMeans costs for light & heavy soil were averaged.
Dig test pit 2	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$59.50	\$59.50	\$59.50	
	80 HP backhoe (equip)	\$24.88		\$/m ³	\$29.86	\$29.86	\$29.86	Test pit is 1m x 1m x 1.275m, therefore volume is ~ 1.2m ³ RSMMeans costs for light & heavy soil were averaged.
	80 HP backhoe + 1 labourer (labour)	\$49.58		\$/m ³	\$59.50	\$59.50	\$59.50	
Infiltration tests	Double-ring infiltrometer	\$608.85		\$/test	\$2,435.40	\$2,435.40	\$2,435.40	2 infiltration tests per test pit
Soil quality testing	Not costed; assumed soil dumped elsewhere on site.							
Site Preparation								
Preconstruction meeting	Part of overhead							
Stakeout of utilities	Assume no interfering utilities are found as a result.		\$500.00	lump sum	\$500.00	\$500.00	\$500.00	
Tree & plant protection	Assume no trees							
Traffic control	Assume not required							
Install erosion & sediment control and control drainage	2" submersible gas pump (incl. gas)	\$81.15		\$/day	\$243.45	\$243.45	\$243.45	Assume 3 days
	Silt sack in catchbasin		\$65.00	\$/unit	\$65.00	\$65.00	\$65.00	
	Silt fence 1m around excavation (material)	\$2.21		\$/m	\$341.00	\$341.00	\$341.00	Assume distance is 2x(69.9 m + 2m) + 2x(3.25 m + 2m), total is 154.3 m. Swale 61.5 m + driveway 8.4 m = 69.9 m.
	Silt fence 1m around excavation (labour)	\$1.77		\$/m	\$273.11	\$273.11	\$273.11	
Mobilization/demobilization	Active construction site so all equipment on site					\$0.00	\$0.00	
Excavation								
Vegetation removal	Clearing, grubbing, haul away material				\$0.00	\$0.00	\$0.00	Active construction site, assume already done
Topsoil salvage, haul to stockpile	200 HP dozer (equip)	\$2.06		\$/m ³	\$0.00	\$0.00	\$0.00	Assumed already done as part of regular construction
	200 HP dozer + 0.5 labourer (labour)	\$1.15		\$/m ³	\$0.00	\$0.00	\$0.00	
Excavation	1.5 m ³ bucket excavator + 1 labourer (labour)	\$1.24		\$/Bm ³	\$88.68	\$88.68	\$88.68	Assume excavation rate of 100 Bm ³ /hr (Mark Preston). Excavation is sloped 2.5:1 along edges 0.5 m depth. Excavation of swale (L 61.5 m, V 62.49 m ³) and driveway with additional 0.15 m depth non-sloped (L 8.4m, V 10.33 m ³), total Volume = 72.82 m ³
	1.5 m ³ bucket excavator (equipment)	\$1.89		\$/Bm ³	\$135.17	\$135.17	\$135.17	Swale bottom = 23.06, sides = 38.91, Corners - 0.52, swale total = 62.49 m ³ , Driveway bottom = 4.1, slopes = 5.72, corners - 0.52, Driveway total - 10.33 m ³ , TOTAL EXCAVATION = 72.82 m ³
Loading excavated soil		15%		% of excavation cost	\$33.58	\$33.58	\$33.58	
Hauling excavated soil	13.76m ³ truck (incl. driver)	\$172.92		\$/hr/truck	\$397.72	\$397.72	\$397.72	71.52 m ³ x 1.25 (swell factor, US Army 2000*) = 89.4 Lm ³ ; thus, 6.5, so 7 truckloads. Assume 20 min. cycle time to dump elsewhere on site; thus, 2 hours and 20 minutes /truck
Safety fencing	4' high fencing, 6m around perimeter of excavation. Assume 1 week rental (incl. setup & takedown).		\$800.00	lump sum for 124m	\$800.00	\$800.00	\$800.00	
Materials and Installation								
HDPE Pipe	Pipe to sewer - 200 mm diameter Armtec Boss 2000, corrugated with smooth inner wall (material)		\$15.37	\$/m	\$76.85	\$76.85	\$76.85	
	Pipe to sewer (labour)	\$11.40		\$/m	\$57.00	\$57.00	\$57.00	
	Delivery for all pipes		\$50.00	lumpsum	\$50.00	\$50.00	\$50.00	
HDPE Pipe Fittings	Pipe to sewer: manhole adapter (200 mm)	\$36.38		\$/unit	\$36.38	\$36.38	\$36.38	

Life Cycle Cost Assessment of Low Impact Development Practices

Item	Item detail	RSMMeans Unit Cost (2010\$CND)	Unit cost - other source (\$CND)	Units	Curb check dam	Filter sock check dam	Rock check dam	Assumptions/Notes
Pipe Fittings: Labour	Pipe to sewer:manhole adapter (200 mm)	\$200.00		\$/unit	\$200.00	\$200.00	\$200.00	
Pipe to sewer trenching	Pipe trenching & backfill, 0.6m wide, 1.2m deep, no slope	\$15.59		\$/m	\$77.95	\$77.95	\$77.95	
	Pipe bedding, 0.6m wide	\$13.18		\$/m	\$65.90	\$65.90	\$65.90	
Pipe for culvert	300 mm, 1.6 mm thickness		\$40.00	\$/m	\$336.00	\$336.00	\$336.00	Smaller than recommended however small depth, 8.4 m width
	Labour	\$11.40		\$/m	\$95.76	\$95.76	\$95.76	Assumed to be the same as HDPE pipe
	Delivery		\$50.00	lumpsum	\$50.00	\$50.00	\$50.00	Assumed to be the same as HDPE pipe
Curbs & gutter with curb inlets	150 mm high curb, 150 mm thick gutter, 600 mm wide (not sure if reinforced) (material)	\$88.35		\$/m	\$5,433.53	\$5,433.53	\$5,433.53	Assumed perimeter along one side, not including driveway
	Labour	\$26.26		\$/m	\$1,614.99	\$1,614.99	\$1,614.99	
Catchbasins	Frame and cover		\$500.00	each	\$500.00	\$500.00	\$500.00	
	Catchbasin	\$367.00		m	\$279.65	\$279.65	\$279.65	Minimum size is 0.762 m
	Installation	\$500.00		each	\$500.00	\$500.00	\$500.00	
Sod	Material		\$2.00	\$/m ²	\$441.00	\$441.00	\$441.00	Sod covers bottom (46.15 m ²) and sides (174.34 m ²), total 220.5 m ²
	Labour		\$1.00	\$/m ²	\$220.50	\$220.50	\$220.50	
Check dams	0.3 m curb		\$150.00	\$/m ²	\$337.50	\$0.00	\$0.00	1 m long, 0.3 m high curbs, length 0.75 m bottom, then up sides 0.75 in length to reach 0.3 m height, so 0.75*3 = 2.25 m for each check dam
	Biofilter sock		\$15.00	\$/m	\$0.00	\$20.25	\$0.00	1 m long, 0.3 m high for each check dam
	Rocks 50 mm clear (material)		\$36.00	\$/m ³	\$0.00	\$0.00	\$19.44	Main section 0.75 m wide by 0.3 m high by minimum 0.6 m length (0.135 m ³), plus front and back slopes at 2:1 ratio (0.068 m ³ per slope), plus sides of main section (0.068 m ³ per side), plus sides of front and back slopes (0.034 m ³ per slope side), TOTAL 0.54 m ³
	Rocks 50 mm clear (labour)	\$173.11		\$/m ³	\$0.00	\$0.00	\$93.48	
	Geotextile Material	\$3.10		\$/m ²	\$0.00	\$0.00	\$9.77	Geotextile required under rock check dam
	Geotextile Labour	\$0.40		\$/m ²	\$0.00	\$0.00	\$1.26	
Stone inlets	50 mm clear (material)		\$36.00	\$/Cm ³	\$16.20	\$16.20	\$16.20	Assume 50 mm clear, 100mm deep, 1 every 6 m, Area = 0.5m ² x 9 inlets = 4.5 m ² . Vol = 4.5 m ² x 0.1 m = 0.45 m ³ .
	Spread by hand (labour)	\$173.11			\$77.90	\$77.90	\$77.90	
	Geotextile Material	\$3.10		\$/m ²	\$13.95	\$13.95	\$13.95	Assume 50 mm clear, 100mm deep. Area = 0.5m ² x 9 inlets = 4.5 m ² .
	Geotextile Labour	\$0.40		\$/m ²	\$1.80	\$1.80	\$1.80	
Headwalls for culvert	Headwall on either side of driveway		\$200.00	each	\$400.00	\$400.00	\$400.00	
50mm stone	50 mm clear limestone (materials)		\$36.00	\$/Cm ³	\$291.60	\$291.60	\$291.60	Free draining backfill around pipe in culvert, surrounding pipe, 0.15 m below (rectangle trench) and 0.15 m above
	1.5 m ³ excavator + 1 labourer (labour)	\$1.23		\$/Bm ³	\$9.96	\$9.96	\$9.96	Volume of pipe = 0.59 m ³ , volume of backfill: flat - 3.645, slopes 5.06, total 8.708 m ³ . Backfill - pipe volume - 8.1 m ³
	1.5 m ³ excavator	\$1.88		\$/Bm ³	\$15.23	\$15.23	\$15.23	
Compact stone	21" wide walk-behind vibrating plate compactor, 2 passes, 6" lifts (equip)	\$0.11		\$/Em ³	\$0.89	\$0.89	\$0.89	Assumed cost is the same whether compacted in 6" or 12" lifts
	21" wide walk-behind vibrating plate compactor, 2 passes, 6" lifts; 1 labourer (labour)	\$1.01		\$/Em ³	\$8.18	\$8.18	\$8.18	
	Proctor test	\$149.45		\$/test	\$149.45	\$149.45	\$149.45	
	Nuclear density test	\$42.81		\$/test	\$42.81	\$42.81	\$42.81	Assume 1 would be sufficient with smaller area
SUBTOTAL					\$16,893	\$16,576	\$16,679	
Fees								
Project Overhead		10.00%		% of subtotal	\$1,689.28	\$1,657.56	\$1,667.92	
TOTAL					\$18,582.08	\$18,233.11	\$18,347.17	

Table A6: Rainwater harvesting

Item	Item detail	RSMears Unit Cost (2010\$CND)	Unit cost (CND) - other source	Units	Concrete Tank Outdoor	Plastic Tank Indoor	Assumptions/Notes
Site Investigation							
Soil strength testing					\$0.00	\$0.00	Not costed; assumed geotech tests done previously
Soil quality testing					\$0.00	\$0.00	Not costed; assumed soil dumped elsewhere on site
Site Preparation							
Preconstruction meeting	Part of overhead				\$0.00	\$0.00	
Stakeout of utilities	Assume no interfering utilities are found as a result.		\$500.00	lump sum	\$500.00	\$0.00	
Mobilization/demobilization	Active construction site, so all equipment on site				\$0.00	\$0.00	
Excavation							
Conveyance Pipe trenching & backfill	0.6 m wide, 1.2 m deep, no slope	\$15.59		m	\$155.90	\$0.00	
Conveyance Pipe Excavation	1.5 m ³ bucket excavator + 1 labourer (labour)	\$2.48		Bm ³	\$62.25	\$0.00	Assumed common earth, 50 Bm ³ /hr (guesstimate by Mariko Uda because this is a small excavation), excavation is 18" around tank (assume same clearance as for rainwater harvesting tank; see below) and sloped 1:1 about 1.2 m depth, and that tank is buried 0.5 m with 6" of bedding underneath
	1.5 m ³ bucket excavator (equipment)	\$3.78		Bm ³	\$94.88	\$0.00	
	Loading - 15% of excavation cost			%	\$5.85	\$0.00	Assume only 6.3 Bm ³ of soil (vol of tank & bedding) is hauled away. Excavation cost of just 6.3 Bm ³ is (\$62 + \$95) * 6.3 Bm ³ / 25.1 Bm ³ = \$39
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		hr	\$32.85	\$0.00	Assume only 6.3 Bm ³ of soil (vol of tank & bedding) is hauled away. 6.3 Bm ³ x 1.25 (swell factor, US Army 2000*) = 7.88 Lm ³ ; thus, 0.57 truckload. Assume 20 min. cycle time to dump elsewhere on site. Thus, 0.19 truck-hours
Tank Excavation	1.5 m ³ bucket excavator + 1 labourer (labour)	\$2.48		Bm ³	\$277.51	\$0.00	Assumed common earth, 50 Bm ³ /hr (guesstimate by Mariko Uda because this is a small excavation), excavation is 18" around tank (according to Technical Advisory Council for Onsite Wastewater Treatment 2006*, put min 18" clearance on all sides of precast concrete septic tank) and sloped 1:1 above 1.2 m depth, and that tank is buried 0.75 m with 6" of bedding underneath.
	1.5 m ³ bucket excavator (equipment)	\$3.78		Bm ³	\$422.98	\$0.00	
	Loading - 15% of excavation cost			%	\$29.10	\$0.00	Assume only 31.0 Bm ³ of soil (vol of tank & bedding) is hauled away. Excavation cost of just 31.0 Bm ³ is (\$278 + \$423) * 31.0 Bm ³ / 111.9 Bm ³ = \$194.
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		hr	\$162.54	\$0.00	Assume only 31.0 Bm ³ of soil (vol of tank & bedding) is hauled away. 31.0 Bm ³ x 1.25 (swell factor, US Army 2000*) = 38.75 Lm ³ ; thus, 2.82 truckload. Assume 20 min. cycle time to dump elsewhere on site. Thus, 0.94 truck-hours.
Service pipe: burying				\$0.00	\$0.00	Buried with conveyance pipe	
Top-up pipe: burying					\$0.00	\$0.00	Assume for simplicity that top-up pipe is buried in same trench as conveyance & service pipes
Overflow pipe trenching & backfill	0.6 m wide, 1.2 m deep, no slope	\$15.59		m	\$0.00	\$0.00	Do not cost as would be needed even without rainwater harvesting
Materials and Installation							
Conveyance Pipe							
PVC SDR 35	300 mm diameter (material)	\$75.31		m	\$753.10	\$0.00	Just costed length from exterior of building to tank
	300 mm diameter (labour)	\$16.20		m	\$162.00	\$0.00	
	300 mm diameter	\$1.62		m	\$16.20	\$0.00	
Pipe bedding	0.6 m wide	\$13.18		m	\$131.80	\$0.00	
Inline German-style filter	3P VF4 by 3P Technik, which is suitable for a		\$5,825.00	ea	\$5,825.00	\$5,825.00	

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost (CND) - other source	Units	Concrete Tank Outdoor	Plastic Tank Indoor	Assumptions/Notes
	2000 m ² catchment						
Precast concrete tank to put filter in	2.1 m long x 1.5 m wide x 1.7 m deep		\$3,000.00	ea	\$3,000.00	\$3,000.00	
Installation of filter into tank and delivery of combined tank/filter	RH20 provides service to install 3P VF4 filter into tank		\$2,000.00	ea	\$2,000.00	\$2,000.00	
Bedding	20 mm clear (material)		\$37.00	m ³	\$33.74	\$0.00	Concrete tanks usually have bedding of 6" of 20-25 mm clean stone. Vol = 6" (152 mm) x 3 m x 2 m = 0.912 m ³
	1.5 m ³ bucket excavator + 1 labourer (labour)	\$2.11		m ³	\$1.92	\$0.00	Assumed cost is similar to cost of excavation but for gravel. Used RSMeans cost for excavation but deducted 15% as suggested by RSMeans for soft soil or sand. Assumed similar for gravel
	1.5 m ³ bucket excavator (equipment)	\$3.21		m ³	\$2.93	\$0.00	
Attach inflow, outflow and overflow pipes to tank			\$100.00	ea	\$300.00	\$300.00	
Backfill	80 HP dozer + 0.5 labourer (labour)	\$1.02		Lm ³	\$26.62	\$0.00	Vol of backfill = vol of excavated (25.06 Bm ³) - vol of tank (5.355 m ³) - vol of bedding (0.912 Cm ³) = 18.8 Cm ³ . Assuming swell factor of 25% and compaction factor of 0.9 (US Army 2000*), 18.8 Cm ³ would equal 26.1 Lm ³ (18.8 Cm ³ * 1.25 / 0.9).
	80 HP dozer (equipment)	\$0.66		Lm ³	\$17.23	\$0.00	
Compact backfill	Walk-behind vibrating plate (labour)	\$3.07		Cm ³	\$57.72	\$0.00	
	Walk-behind vibrating plate (equipment)	\$0.28		Cm ³	\$5.26	\$0.00	
Tank							
Pre-cast concrete (below ground)			\$0.30	L	\$6,900.00	\$0.00	
Standard tank access riser			\$418.00	ea	\$418.00	\$0.00	
Plastic tank (above-ground)			\$0.29	L	\$0.00	\$6,670.00	
Concrete tank delivery			\$233.00	tank	\$233.00	\$0.00	
Bedding	20 mm clear (material)		\$37.00	m ³	\$106.19	\$0.00	Concrete tanks usually have bedding of 6" of 20-25 mm clean stone. Vol = 6" (152 mm) x 3.2 m x 5.9 m = 2.87 m ³ .
	1.5 m ³ bucket excavator + 1 labourer (labour)	\$2.11		m ³	\$6.06	\$0.00	Assumed cost is similar to cost of excavation but for gravel. Used RSMeans cost for excavation but deducted 15% as suggested by RSMeans for soft soil or sand. Assumed similar for gravel.
	1.5 m ³ bucket excavator + 1 labourer (equipment)	\$3.21		m ³	\$9.21	\$0.00	
Installation/craning	For precast concrete tanks > 20,000 L		\$155.00	hr	\$620.00	\$0.00	
Attach connections (conveyance pipe, service pipe, overflow pipe, fill pipe, wiring)			\$100.00	ea	\$500.00	\$500.00	
Backfill	80 HP dozer + 0.5 labourer (labour)	\$1.02		Lm ³	\$114.65	\$0.00	Vol of backfill = vol of excav (111.94 Bm ³) - vol of tank (28.175 m ³) - vol of bedding (2.87 Cm ³) = 80.895 Cm ³ . Assuming swell factor of 25% and compaction factor of 0.9 (US Army 2000*), 80.895 Cm ³ would equal 112.4 Lm ³ (80.895 Cm ³ * 1.25 / 0.9)
	80 HP dozer (equipment)	\$0.66		Lm ³	\$74.18	\$0.00	
Compact backfill	Walk-behind vibrating plate (labour)	\$3.07		Cm ³	\$248.35	\$0.00	
	Walk-behind vibrating plate (equipment)	\$0.28		Cm ³	\$22.65	\$0.00	
Plumbing Accessories							
Submersible pump	81.2 lpm fountain pump with controls (material)	\$2,485.00		ea	\$2,485.00	\$2,485.00	3/4 hp costs \$2234.05 (material); thus \$30.60 (material)/lpm. Thus for 81.2 lpm --> \$2485 (material).
	81.2 lpm fountain pump with controls (labour)	\$245.00		ea	\$245.00	\$245.00	3/4 hp costs \$220.33 (labour); thus \$3.02 (labour)/lpm. Thus for 81.2 lpm --> \$245 (labour).
Pressure tank	439 L (116 gallons) potable water tank (material)	\$3,362.49		ea	\$3,362.49	\$3,362.49	Steel water tanks can be used as pressure tanks

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost (CND) - other source	Units	Concrete Tank Outdoor	Plastic Tank Indoor	Assumptions/Notes
	439 L (116 gallons) potable water tank (labour)	\$68.62		ea	\$68.62	\$68.62	
Pump float switch	Approx. 1hp		97.38	ea	\$97.38	\$97.38	
Pump float electrical wiring	Approx. 14 gauge	\$1.76		m	\$26.40	\$8.80	
Service pipe: Polyethylene (PE) C901	40 mm diameter (material)	\$5.15		m	\$87.55	\$25.75	PE C901 usually comes in coils or 20' lengths, no couplings required
	40 mm diameter (labour)	\$5.58		m	\$94.86	\$27.90	
Service pipe: Polyethylene (PE) C901 fittings	40 mm diameter elbow (material)	\$7.24		ea	\$28.96	\$14.48	
	40 mm diameter elbow (labour)	\$14.94		ea	\$59.76	\$29.88	
Service pipe through wall	Pipe sleeve with link seal for 1-1/2" diameter pipe (material)	\$64.09		ea	\$64.09	\$0.00	
	Pipe sleeve with link seal for 1-1/2" diameter pipe (labour)	\$93.80		ea	\$93.80	\$0.00	
Service pipe: hangers every meter indoors	Hanger consisting of clamp, clevis & rod (material)	\$11.81		ea	\$59.05	\$35.43	
	Hanger consisting of clamp, clevis & rod (labour)	\$15.69		ea	\$78.45	\$47.07	
Supply pipe: Copper Class K	40 mm diameter, includes couplings & hangers (material)	\$46.95		m	\$4,436.78	\$4,436.78	
	40 mm diameter, includes couplings & hangers (labour)	\$37.39		m	\$3,533.36	\$3,533.36	
Supply pipe: Copper fittings	40 mm diameter 90 degree elbows (material)	\$20.98		ea	\$629.40	\$629.40	Assume cost of fittings on average is the cost of an elbow
	40 mm diameter 90 degree elbows (labour)	\$43.78		ea	\$1,313.40	\$1,313.40	
Top-up float switch	Approx. 1/2 hp		\$54.16	ea	\$54.16	\$54.16	
Top-up float electrical wiring	Approx. 14 gauge	\$1.76		m	\$26.40	\$8.80	
Solenoid valve	Domestic/commercial, bronze, compound, flanged, 20 mm		\$309.90	ea	\$309.90	\$309.90	
Water hammer arrestor	20 mm (material)	\$22.48		ea	\$22.48	\$22.48	
	20 mm (labour)	\$47.42		ea	\$47.42	\$47.42	
Water meter	40 mm (material)	\$305.90		ea	\$305.90	\$305.90	
	40 mm (labour)	\$75.88		ea	\$75.88	\$75.88	
Air gap (tundish)	3P Tundish by 3P Technik (material)		\$75.00	ea	\$75.00	\$75.00	
	Tundish (labour)		\$50.00	ea	\$50.00	\$50.00	
Top-up pipe: ABS (int. installation)	50 mm diameter, including couplings and hangers (material)	\$8.49		m	\$42.45	\$42.45	
	50 mm diameter, including couplings and hangers (labour)	\$54.26		m	\$271.30	\$271.30	
Top-up pipe: ABS elbow	50 mm diameter (material)	\$2.53		ea	\$2.53	\$2.53	Assumed need atleast 1 elbow
	50 mm diameter (labour)	\$31.01		ea	\$31.01	\$31.01	
Top-up pipe through wall	Pipe sleeve with link seal for 2" diameter pipe (material)	\$75.05		ea	\$75.05		Method used to bring pipes through walls in commercial applications
	Pipe sleeve with link seal for 2" diameter pipe (labour)	\$106.68		ea	\$106.68	\$0.00	
Top-up pipe: ABS (ext.installation)	50 mm diameter (does not include coupling or hangers) (material)	\$4.84		m	\$48.40	\$0.00	
	50 mm diameter (does not include coupling or hangers) (labour)	\$28.22		m	\$282.20	\$0.00	
Top-up pipe: ABS couplings	50 mm diameter (material)	\$1.06		ea	\$1.06	\$0.00	Assume 1 coupling required
	50 mm diameter (labour)	\$31.01		ea	\$31.01	\$0.00	
Reduced pressure backflow preventer	50 mm (material)	\$909.09		ea	\$909.09	\$909.09	
	50 mm (labour)	\$81.17		ea	\$81.17	\$81.17	
Overflow							

Item	Item detail	RSMeans Unit Cost (2010\$CND)	Unit cost (CND) - other source	Units	Concrete Tank Outdoor	Plastic Tank Indoor	Assumptions/Notes
PVC SDR 35	300 mm diameter	\$93.13		m	\$0.00	\$0.00	Do not cost as would be needed even without rainwater harvesting
Pipe bedding	0.6 m wide	\$13.18		m	\$0.00	\$0.00	
1 bend	PVC SDR 35, 300 mm diameter elbow (material)	\$279.73		ea	\$0.00	\$0.00	
	PVC SDR 35, 300 mm diameter elbow (labour)	\$101.85		ea	\$0.00	\$0.00	
SUBTOTAL					\$42,943.11	\$36,942.82	
Fees							
Project Overhead		10.00%		% of sub total	\$4,294.31	\$3,694.28	
TOTAL					\$47,237	\$40,637	

Table A7: Extensive greenroof

Item	Item detail	RSMMeans Unit Cost (2010 or 2011\$CND)	Unit cost (\$CND) - other source	Units	Cheap	Expensive	Assumptions/Notes
Site Preparation							
Pre-construction meeting	Part of construction mgmt fee				\$0.00	\$0.00	
Mobilization/demobilization	Crane, 55 ton	\$158.00		2-way	\$316.00	\$0.00	Assumed for 2 mobilizations/demobilization's because crane is initially needed to lift membrane, then is not needed until later for the rest of the materials.
Mobilization/demobilization	Crane, 100 ton	\$453.00		way	\$0.00	\$1,812.00	Assumed for 2 mobilizations/demobilization's because crane is initially needed to lift membrane, then is not needed until later for the rest of the materials.
Crane							
55T crane to lift membrane, drainage layer, stone, edging, cuttings to under 5 storeys	Equipment & labour	\$4,632.38		day	\$13,897.14	\$0.00	55T crane does 28 picks per day, will need it for 2.6 days - round up to 3 days.
100T crane to lift membrane, root barrier, drainage layer, stone, edging, sedum mats to 6-10 storeys	Equipment & labour	\$5,152.38		day	\$0.00	\$56,676.18	100T crane does 21 picks per day will need it fo 11.2 days - round down to 11 days.
Materials and Installation							
Waterproof membrane: TPO	Material & delivery - TPO, 60mils thick, fully adhered	\$12.48		m ²	\$24,960.00	\$0.00	
	Lift onto roof - equipment & labour	See above -Crane					
	Labour	\$10.14		m ²	\$20,280.00	\$0.00	
	Equipment	\$0.83		m ²	\$1,660.00	\$0.00	
	Extra labour for flashing around parapets & roof penetrations - assume labour cost similar to PVC sheet flashing	\$17.22		m ²	\$1,248.45	\$0.00	Assume 330 mm of flashing around parapets, mechanical units and drains.
Waterproof membrane: EPDM	Material & delivery - EPDM, 60 mils thick, fully adhered	\$18.21		m ²	\$0.00	\$36,420.00	
	Lift onto roof - equipment & labour	See above -Crane					
	Labour	\$9.74		m ²	\$0.00	\$19,480.00	
	Equipment	\$0.79		m ²	\$0.00	\$1,580.00	
	Extra labour for flashing around parapets & roof penetrations - assume labour cost similar to PVC sheet flashing	\$17.22		m ²	\$0.00	\$1,248.45	Assume 330 mm of flashing around parapets, mechanical units and drains.
Water leakage test: EFVM	EFVM by International Leak Detection - cost to install grid & conduct initial test		\$13.99	m ²	\$0.00	\$27,980.00	Tested in one visit
Water leakage test: other option more cheaper than EFVM	Applied potential electrical method or water lance method by I-CORP International - cost to do initial test		\$3,000.00	lump sum	\$3,000.00	\$0.00	Conductive material is required below the waterproof membrane, assumed a concrete structure. These methods cannot be used for black EPDM, which has too much carbon content.
Root barrier (not needed for TPO, possibly needed for EPDM)	Material		\$4.30	m ²	\$0.00	\$8,600.00	Average of quotes
	Lift onto roof - equipment & labour	See above -Crane					
	Labour - assume similar to laying drainage mat.	\$4.16		m ²	\$0.00	\$8,320.00	
R20 insulation	Material - Dow Roofmate		\$23.68	m ²	\$47,360.00	\$47,360.00	
	Lift onto roof - equipment & labour	See above -Crane					
	Labour	\$4.95		m ²	\$9,900.00	\$9,900.00	
Drainage layer + filter cloth (combined):	Material + delivery: average cost of 3 different drainage layers (3RFoam, dimple board, and another dimple board).		\$11.09	m ²	\$22,180.00	\$22,180.00	Average cost for drainage layer
	Onto roof - equipment & labour	See above -Crane					

Item	Item detail	RSMeans Unit Cost (2010 or 2011\$CND)	Unit cost (\$CND) - other source	Units	Cheap	Expensive	Assumptions/Notes
	Labour - similar to laying drainage mat	\$4.16		m ²	\$8,320.00	\$8,320.00	
Irrigation system	Total installed cost		\$10.76	m ²	\$0.00	\$21,520.00	Assumed irrigation cost costs approx. \$1/sf.
Edging: aluminium	Permaloc's GeoEdge 4-1/2" aluminum edging		\$30.70	m	\$7,521.50	\$0.00	
	Permaloc's GeoEdge 4-1/2" aluminum corners		\$43.06	corner	\$1,033.44	\$0.00	No delivery cost included, assumed not significant. Assume pre-made corners for the 4 perimeter corners and the corners around the 5 mechanical units (4 corners each), but not for the drains.
	Permaloc's GeoEdge 6-1/2" aluminum edging		\$46.05	m	\$0.00	\$11,282.25	
	Permaloc's GeoEdge 6-1/2" aluminum corners		\$57.46	corner	\$0.00	\$1,379.04	No delivery cost included, assumed not significant. Assume pre-made corners for the 4 perimeter corners and the corners around the 5 mechanical units (4 corners each), but not for the drains.
	Onto roof - equipment & labour	See above -Crane					
	Labour - assume similar to installing lumber edging	\$8.89			m	\$2,311.40	\$2,311.40
Vegetation-free zone - 3" of 1-1/2" washed round stone	Material + delivery: average cost of 3 different suppliers		\$7.93	m ²	\$958.34	\$958.34	
	Onto roof - equipment & labour	See above -Crane					
	Labour - assume similar to spreading same volume of pea gravel	\$62.13			m ³	\$1,568.38	\$1,568.38
4" growing medium - in bulk, not in sacks	Material - average of 3 suppliers		\$13.00	m ²	\$24,206.00	\$0.00	
	Delivery in bulk		\$0.85	m ²	\$1,582.70	\$0.00	
	Blowing onto roof with blower truck		\$7.10	m ²	\$13,220.20	\$0.00	
6" growing medium - in sacks	Material		\$23.40	m ²	\$0.00	\$43,570.80	
	Delivery of sacks		\$1.99	m ²	\$0.00	\$3,705.38	
	Lifting onto roof with crane 6-10 stories & spreading - equipment & labour	\$20.77			m ²	\$0.00	\$38,673.74
Plants; sedum cuttings	Material		\$2.12	m ²	\$3,947.44	\$0.00	
	Delivery		\$0.11	m ²	\$204.82	\$0.00	
	Onto roof - equipment & labour	See above -Crane					
	Labour - assume similar to applying 2 bushels/1000 sf of sprigs	\$0.31			m ²	\$577.22	\$0.00
Plants; sedum plugs	Material		\$4.30	m ²	\$0.00	\$0.00	Toronto's Green Roof Bylaw says min. 1 plug/sf.
	Delivery		\$0.61	m ²	\$0.00	\$0.00	Assume truck cost of \$500 plus \$160 per rack. 4 racks would be needed. Therefore, total delivery charge is \$1140, or \$0.61/m ² .
	Onto roof - equipment & labour	See above -Crane					
	Labour - assume similar to planting 2-1/4" potted plants at 1/sf.	\$5.49			m ²	\$0.00	\$0.00
Plants; sedum mats	Material		\$31.22	m ²	\$0.00	\$58,131.64	This material includes 1" of growing medium, so we can minus 1" of growing medium.
	Delivery		\$1.42	m ²	\$0.00	\$2,644.04	3 trucks needed at a cost of \$750 each. Pallets are required for \$390. Therefore total cost is \$2640, or \$1.42/m ² .
	Onto roof - equipment & labour	See above -Crane					
	Labour	\$4.63			m ²	\$0.00	\$8,621.06
	SAVINGS because can reduce growing medium by 1"			-\$14,325.00	lump sum	\$0.00	-\$14,325.00
SUBTOTAL with membranes					\$210,253.03	\$429,917.70	
Fees							

Item	Item detail	RSMeans Unit Cost (2010 or 2011\$CND)	Unit cost (\$CND) - other source	Units	Cheap	Expensive	Assumptions/Notes
Project Overhead (10%)		10.00%		% of sub total	\$21,025.30	\$42,991.77	
TOTAL					\$231,278	\$472,909	
SUBTOTAL without membranes ¹							
Fees					\$100,054	\$307,871	
Project Overhead (10%)		10.00%			\$10,005	\$30,787	
TOTAL					\$110,060	\$338,658	

Notes: ¹Subtotal without membranes excluded costs for membranes and R20 insulation, as well craning was reduced to 1 day for cheap, and 2 days for expensive.

Table A8: Asphalt (used for comparative analysis)

Item	Item detail	RSMMeans Unit Cost (2010\$CND)	Unit cost (\$CND) - other source	Units	Total (\$CND)	Assumptions/Notes
Site Investigation						
Soil strength testing						Not costed; assumed geotech tests done previously
Soil quality testing						Not costed; assumed soil dumped elsewhere on site
Site Preparation						
Pre-construction meeting	Part of overhead					
Stakeout of utilities	Assume no interfering utilities found as a result		\$500.00	lump sum	\$500.00	
Mobilization/demobilization						Active construction site, so all equipment on site
Excavation						
Vegetation removal	Clearing, grubbing, haul away material				\$0.00	Active construction site, assume already done
Topsoil salvage, haul to stockpile	6" removed, 60 m travel to stockpile, 200 HP dozer + 0.5 labourer (labour)	\$1.15		\$/m ³	\$175.26	
	6" removed, 60 m travel to stockpile, 200 HP dozer (equipment)	\$2.06		\$/m ³	\$313.94	
Excavate	1.5 m ³ bucket excavator + 1 labourer, productivity 100 Bm ³ /hr (labour)	\$1.24		\$/Bm ³	\$248.50	Assumed a productivity of 100 Bm ³ /hr. Assumed common earth. 6" of topsoil has already been removed, so do not need to excavate full depth, plus catchbasins and pipe 60 m x 16.7 m x 0.2 m = 200.4 m ³ for parking lot
	1.5 m ³ bucket excavator, productivity 100 Bm ³ /hr (equipment)	\$1.89		\$/Bm ³	\$378.76	
	Loading	15%		% of excavation cost	\$94.09	
	Hauling in a 13.76 m ³ truck (including truck & driver)	\$172.92		\$/hr	\$1,038.84	Assumed swell factor of 25% (*US Army 2000), cycle time of 20 min.
Compaction of native soil	30,000 lb grader + 25T vibratory roller + 1 labourer (labour)	\$1.16		\$/m ²	\$1,160.00	
	30,000 lb grader + 25T vibratory roller (equipment)	\$1.14		\$/m ²	\$1,140.00	
	Proctor test	\$149.45		\$/test	\$149.45	1 test required
	Nuclear density test	\$42.81		\$/test	\$171.24	Avg. 4 tests required - test is done to check compaction.
Hydrodynamic Separator						
Hydrodynamic Separator	Downstream Defender - 4' wide (mat + delivery)		\$12,000.00	lump sum	\$0.00	
	Assume installation cost is roughly similar to that of 4' dia., 10' deep precast manhole in RSMMeans that includes 47.9 Bm ³ excavation (labour & equip)	\$2,883.00		lump sum	\$0.00	
	Loading excavated soil	15%		% of excavation cost	\$0.00	
	Hauling in a 13.76 m ³ truck (includes driver)	\$172.92		\$/hr	\$0.00	
Materials and Installation						
Catchbasins	Frame and cover		\$500.00	each	\$0.00	
	Catchbasin		\$367.00	m	\$0.00	Minimum size is 0.762, two catchbasins = 1.524 m
	Installation		\$500.00	each	\$0.00	
Conveyance pipes from catchbasins to HDS	Armtec Boss 2000 solid pipe, 300 mm dia. (material)		\$22.81	m	\$0.00	Catchbasins on either end of parking lot at halfway point, drain to HDS at halfway along other end, so 2 x 30 m = 60 m, plus 2 x 8.335 m = 16.67 m, TOTAL 66.67 m
	(labour & equip)	\$5.10		m	\$0.00	
Base, 300 mm deep	20 mm crusher run (material)		\$43.00	\$/Cm ³	\$12,900.00	
	30,000 lb grader, 300 HP dozer, 25 T vibratory roller, truck tractor & water tank trailer + 1 labour foreman (labour)	\$0.75		\$/m ²	\$750.00	

Item	Item detail	RSMMeans Unit Cost (2010\$CND)	Unit cost (\$CND) - other source	Units	Total (\$CND)	Assumptions/Notes
	30,000 lb grader, 300 HP dozer, 25 T vibratory roller, truck tractor & water tank trailer (equipment)	\$1.20		\$/m ²	\$1,200.00	
Compaction test	Proctor test				\$0.00	Assume supplier provides curve, so not required.
	Nuclear density test	\$42.81		\$/test	\$214.05	Assume 5 tests
Asphalt	Plant mix asphalt, wearing course, 50 mm thick (material)	\$18.37		\$/m ²	\$18,370.00	60 m x 33.34 m = 2000 m ²
	1 foreman, 7 labourers, 4 equipment operators (labour)	\$1.02		\$/m ²	\$1,020.00	
	130 HP asphalt paver, 2 10T tandem rollers, 1 12T pneumatic whl roller (equipment)	\$0.61		\$/m ²	\$610.00	
	Hauling in a 13.76 m ³ truck (including truck & driver)	\$172.92		\$/hr	\$864.60	Assume cycle time of 1h, assumed a 18 cy (13.76 m ³) / 25T ton truck. The vol of asphalt required is 50 mm x 1000 m ² = 50 m ³ . If the compacted density of asphalt is 145 lb/cu ft (2322 kg/m ³), then we need 50 m ³ x 2322 kg/m ³ = 116.1 T. If each truck load takes 25 T, we need 4.6 (i.e. 5) truck loads. Therefore 5 truck hours.
	Asphalt lab test		\$200.00	\$/test	\$200.00	For the 1000 m ² parking lot, we need 116.1 T. Thus, assume just 1 test.
	Asphalt nuclear density tests		\$60.00	\$/hr	\$180.00	For this 1000 m ² parking lot, the asphalt paving productivity is 5305m ² per day according to RSMMeans which is lower than other sources. Assume 3 hours.
Striping		\$0.46		\$/m ²	\$460.00	
SUBTOTAL					\$42,138.72	
Fees						
Project overhead		10.00%		% of sub total	\$4,213.87	
TOTAL					\$46,353	

APPENDIX B:

Maintenance Costs

Table B1: Bioretention maintenance yearly costs

Maintenance Task	Frequency	Full Infiltration	Partial or No Infiltration
Watering	Year 1: Weekly first 2 months, bi-weekly May to August	\$302	\$302
	Year 2: 10% of plants that are new, weekly for first 2 months, biweekly May-August	\$212	\$212
	Year 3: Biweekly May-August	\$20	\$20
Inspection	Years 1 & 2: 4.5 times per year	\$212	\$212
	2.5 times per year in subsequent years	\$118	\$118
Remove litter and debris	6 times per year	\$120	\$120
Remove Sediment	Every 2 years, or as needed	\$912	\$912
	After year 2	\$362	\$362
Prune	Annually or as needed	\$58	\$58
Weed	6 times per year	\$120	\$120
Add mulch to maintain 75 mm	Replace every 3 years	\$980	\$980
Restore lost vegetation	10% in year 2	\$437	\$437
Unclog underdrain	Every 10 years	\$0	\$77
Average per year		\$945	\$952

Table B2: Bioretention rehabilitation

Item	Full, Partial or No Infiltration
Remove all plants	\$137
Install new plants	\$4,367
Install new filter media	\$1,738
Till	\$103
TOTAL	\$6,345

Table B3: Permeable pavement maintenance yearly costs

Maintenance Task	Frequency	Full Infiltration	Partial or No Infiltration
Surface sweeping with vacuum	Every 2 years	\$582	\$582
Restriping	Every 3 years	\$460	\$460
Pave replacement (10 pavers)	Every 8 years	\$57	\$57
Clean out pipes	Every 10 years	\$0	\$38
Average per year		\$433	\$436

Table B4: Permeable pavement rehabilitation

Item	Full, Partial or No Infiltration
Removal of Pavers and Stone	
Remove pavers	\$1,625
Remove No. 8, and top 2" of No.57 stone	\$2,057
Cost of removal at year 30	\$3,682
Installation of New Pavers and Stone	
Erosion and sediment control	\$600
Mobilization & demobilization	\$1,516
Base, 50 mm deep	\$3,440
Compaction test	\$86
Plastic edge restraints	\$709
Bedding & pavers	\$56,860
Striping	\$460
Cost of installation at year 30	\$63,670
SUBTOTAL	\$67,35
Clean up	\$6,735
TOTAL	\$74,088

Table B5: Infiltration trenches yearly costs

Maintenance Task	Frequency	Roof Only	Road & Roof
Catchbasin cleanout	Once a year for roof runoff only design	\$75	\$0
Vacuum sediment & oil from hydrodynamic separator	Annually for parking lot runoff design	\$0	\$1,200
Soil Test	At 8 years for parking lot runoff design	\$0	\$550
Remove & replace filter cloth inner lining from perforated pipe. Test & dispose of sediment.	Once every 8 years for parking lot runoff design	\$0	\$750
Average per year		\$74	\$1,277

Table B6: Infiltration chambers yearly costs

Maintenance task	Frequency	Roof Only	Road & Roof
Catchbasin cleanout	Once a year for Roof Runoff only design	\$75	\$0
Vacuum sediment & oil from hydrodynamic separator	Annually	\$0	\$1,200
Jet vac & vacuum sediment from isolator row of infiltration chambers	Once every 8 years	\$0	\$300
Average per year		\$74	\$1,212

Table B7: Enhanced grass swales yearly costs

Maintenance task	Frequency	Curb/Filter sock/Rock check dam
Watering	Year 1: Weekly first 2 months, bi-weekly May to August	\$767
	Year 2: 10% of plants that are new, weekly for first 2 months, biweekly May-August	\$534
	Year 3: Biweekly May-August	\$51
Inspection	Years 1 & 2: 4.5 times per year	\$212
	2.5 times per year in subsequent years	\$118
Remove litter and debris	Years 1 & 2: 4.5 times per year	\$90
	2.5 times per year in subsequent years	\$50
Remove sediment	Every 2 years, or as needed	\$912
	After year 2	\$362
Restore lost vegetation	In Year 2	\$66
Mowing	Once a month, as as needed May to September	\$106
Average per year		\$500

Table B8: Rainwater harvesting yearly costs

Maintenance Task	Frequency	Concrete Tank Outdoor	Plastic Tank Indoor
Cleaning in-line filter	Annually	\$75	\$75
Inspection	Annually	\$100	\$100
Cleaning out tank	Every 10 years	\$1,200	\$1,200
Replacing pump	Every 10 years	\$2,485	\$2,485
Replacing pressure tank	Every 10 years	\$3,431	\$3,431
Average per year		\$744	\$863
Rehabilitation (replace plastic tank)	Every 40 years	n/a	\$7,170

Table B9: Extensive greenroof maintenance yearly costs

Maintenance Task	Frequency	Cheap	Expensive
Watering	Year 1: Cheap case – once or twice a day until establishment (14 weeks), then once a week for 2.5 months	\$15,800	\$0
	Year 2: Cheap case – once every 2-3 weeks for 4 months	\$700	\$0
	Year 3: Cheap case – once every 2-3 weeks for 4 months	\$700	\$0
Weeding	Year 1: Cheap case – every other week for 2 months, then once a month for 4 months Expensive case - Once	\$8,640	\$1,080
	Year 2: Cheap case – once a month for 6 months Expensive case – ½ of area once	\$6,480	\$540
	Year 3: Cheap case – three times Expensive case – ½ of area once	\$3,240	\$540
	Subsequent years: Both case – ½ of area once	\$540	\$540
Plant replacement (10%)	Every 40 years	\$2,080	\$2,080
Check drains, flashing, membrane	Twice a year	\$100	\$100
Test Membrane	Every 5 years	\$3,000	\$5,000
Membrane repair of small leak	Every 5 years after 10 years	\$762	\$762
Average per year		\$9495	\$13.985

Table B10: Extensive greenroof rehabilitation

Item	Cheap	Expensive
Remove sedum, growing medium & stone ¹	\$45,470	\$68,205
Remove drainage layer ¹	\$10,505	\$10,505
Remove insulation ¹	\$27,034	\$27,034
Remove TPO/EPDM ¹	\$13,036	\$13,036
Chute	\$4,731	\$12,617
Cost of demolition	\$100,776	\$131,397
Cost of new greenroof	\$210,253	\$429,918
Subtotal	\$311,029	\$561,315
Project overhead	\$31,103	\$56,131
TOTAL	\$342,132	\$617,446

Notes: ¹Includes carrying across roof and disposal

Table B11: Asphalt yearly costs (used for comparative analysis)

Maintenance Task	Frequency	Yearly cost
Sealcoat	Every 3 years	\$2,900
Cleaning surface prior to sealcoating	Every 3 years	\$220
Restriping (after sealcoat)	Every 3 years	\$460
Crack filling, pothole filling, patches	Ongoing as needed	\$1,000
Average per year		\$2,146

Table B12: Asphalt rehabilitation (used for comparative analysis)

Item	Total cost
Remove asphalt	\$6,470
Regrading, compacting as necessary	\$490
New asphalt	\$21,245
Striping	\$460
Cost of rehabilitation at 25 years	\$28,665
Project overhead	\$2,867
Overall cost of rehabilitation at 25 years	\$31,532

APPENDIX C:

Life Cycle Maintenance Costs

Table C1: Bioretention

Maintenance Task	Full Infiltration	Partial or No Infiltration
Water	\$534	\$534
Inspection	\$6,088	\$6,088
Litter	\$6,000	\$6,000
Sediment	\$9,600	\$9,600
Prune	\$2,900	\$2,900
Weed	\$6,000	\$6,000
Mulch	\$15,680	\$15,680
Vegetation	\$437	\$437
Underdrain	\$0	\$385
Rehab	\$7504	\$7504
TOTAL	\$54,743	\$55,128

Table C2: Permeable pavement

Maintenance Task	Full Infiltration	Partial or No Infiltration
Vacuum sweep	\$13,968	\$13,968
Replace pavers	\$339	\$339
Clean out pipes	\$0	\$154
Restriping	\$7,360	\$7,360
Rehab	\$72,990	\$72,990
TOTAL	\$94,657	\$94,811

Table C3: Infiltration trenches

Maintenance Task	Roof Only	Road & Roof
Cleanout catchbasin	\$3,675	\$0
Clean-out hydrodynamic separator	\$0	\$58,800
Replace filter cloth & dispose sediment	\$0	\$4,500
Test sediment	\$0	\$550
TOTAL	\$3,675	\$63,850

Table C4: Infiltration chambers

Maintenance Task	Roof Only	Road & Roof
Cleanout catchbasin	\$3,675	\$0
Clean-out separator	\$0	\$58,800
Clean-out infiltration chamber	\$0	\$1,800
TOTAL	\$3,675	\$60,600

Table C5: Enhanced grass swales

Maintenance Task	Curb/Filter sock/Rock check dam
Water	\$1,351
Inspection	\$6,088
Litter	\$2,580
Remove sediment	\$9,600
Restore vegetation	\$66
Mowing	\$5,300
TOTAL	\$24,985

Table C6: Rainwater harvesting

Maintenance Task	Concrete Tank	Plastic Tank
Cleaning in-line filter	\$3,750	\$3,750
Inspection	\$5,000	\$5,000
Cleaning out tank	\$4,800	\$4,800
Replacing pump & pressure tank	\$23,664	\$23,664
Replacement	n/a	\$5,970
TOTAL	\$37,214	\$43,184

Table C7: Extensive greenroof

Maintenance Task	Cheap	Expensive
Water	\$17,200	\$0
Weeding	\$43,740	\$27,540
Plant replacement	\$2,080	\$2,080
Check drains, flashing, membrane	\$5,000	\$5,000
Test membrane	\$27,000	\$45,000
Repair membrane, small leak	\$6,096	\$6,096
Replacement	\$373,628	\$613,542
TOTAL	\$474,744	\$699,258

Table C8: Asphalt (used for comparative analysis)

Maintenance Task	Asphalt
Clean, sealcoat and restriping	\$57,280
Crack filling, pothole filling and patching	\$50,000
Rehabilitation	\$26,951
TOTAL	\$134,231

APPENDIX

B

Stormwater Management Calculations



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 1

Subject **Rainfall Data**

2. Rainfall data

Stormwater management facilities should be designed based on the IDF tables developed by Environment Canada for Barrie WPCC based on rain gauge data for the period 1979 - 2003 including a 15% increase in rainfall intensity data to account for climate change. The adjusted Chicago distribution parameters for different return periods are provided below

Rainfall parameters

Return Period (Year)	2	5	10	25	50	100
A	678.085	853.608	975.865	1146.275	1236.152	1426.41
B	4.699	4.699	4.699	4.922	4.699	5.273
C	0.781	0.766	0.76	0.757	0.751	0.759

City of Barrie 4-hr Chicago Storm Hyetographs (Adjusted to Account for Climate Change)

Time (min)	Intensity (mm/hr)					
	2-YR	5-YR	10-YR	25-YR	50-YR	100-YR
0	0.00	0.00	0.00	0.00	0.00	0.00
10	2.47	3.57	4.32	5.22	5.93	6.41
20	2.82	4.07	4.91	5.94	6.74	7.29
30	3.31	4.76	5.73	6.93	7.85	8.52
40	4.05	5.79	6.96	8.42	9.50	10.36
50	5.30	7.53	9.03	10.91	12.27	13.45
60	7.98	11.20	13.36	16.13	18.04	19.96
70	18.78	25.64	30.27	36.37	40.22	45.22
80	83.11	108.92	126.55	148.15	164.22	180.15
90	24.57	33.31	39.22	47.06	51.92	58.54
100	13.01	17.99	21.35	25.72	28.58	31.96
110	9.01	12.60	15.01	18.11	20.23	22.45
120	6.97	9.82	11.74	14.17	15.88	17.52
130	5.73	8.12	9.72	11.74	13.20	14.50
140	4.89	6.96	8.35	10.09	11.37	12.44
150	4.28	6.12	7.35	8.89	10.03	10.94
160	3.82	5.48	6.59	7.96	9.00	9.80
170	3.46	4.97	5.99	7.24	8.19	8.90
180	3.17	4.56	5.50	6.65	7.53	8.16
190	2.93	4.22	5.09	6.15	6.98	7.56
200	2.72	3.93	4.74	5.74	6.51	7.04
210	2.55	3.68	4.45	5.38	6.11	6.60
220	2.39	3.47	4.19	5.08	5.77	6.22
230	2.26	3.28	3.97	4.80	5.46	5.89
240	2.15	3.12	3.77	4.57	5.19	5.59
Depth (mm)	36.96	50.52	59.69	71.24	79.45	87.58

A	678.085	853.608	975.865	1146.275	1236.152	1426.408
B	4.699	4.699	4.699	4.922	4.699	5.273
C	0.781	0.766	0.760	0.757	0.751	0.759

Storm Event	Depth (mm)	
	12-hr SCS (no climate change)	12-hr SCS (climate change)
2-year	40.8	46.8
5-year	56.4	64.8
10-year	66.0	75.6
25-year	79.2	91.2
50-year	88.8	102.0
100-year	98.4	112.8
Regional (Hazel)	212	



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			2

Subject **Pre-Development Conditions Time to Peak**

Airport Equation To be used if "C" value is less than or equal to 0.4

$$t_c = 3.26 * (1.1 - C) * L^{0.5} * S_w^{-0.33}$$

Where: t_c = time of concentration, minutes
 C = runoff coefficient
 L = watershed length, m
 S_w = watershed slope, %

Catchment Name	Max Ele.	Minimum Ele.	L	Slope %	C	Tc	TP
Catchment 101	275	226	2335	2.10	0.2	1.85	1.23
Catchment 102	237	224.7	980.5	1.25	0.2	1.42	0.95
Catchment 103	272	222.8	4350	1.13	0.2	3.10	2.06
Catchment 104	270.5	223.99	3271	1.42	0.20	2.49	1.66
Catchment 105	236.5	222.82	303	4.51	0.20	0.52	0.35
Catchment 106-1	253.5	226.15	787	3.48	0.20	0.91	0.61
Catchment 107	255.5	229.30	1292	2.03	0.20	1.39	0.93
Catchment 108	275	235.71	4287.00	0.92	0.20	3.30	2.20
Catchment 109	248	241.00	296.00	2.36	0.20	0.63	0.42
Catchment 110	250	227.30	1103.00	2.06	0.20	1.28	0.85
Catchment 111	236.5	230.90	1576.00	0.36	0.20	2.73	1.82
Catchment 112	255	245.56	3146.00	0.30	0.20	4.08	2.72



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 3

Subject **Pre-development Conditions Road Drainage Areas**

Pre-development Area Breakdown

Catchment Name	Area (ha)	Road Area (ha)	TIMP (%)	XIMP (%)	Road Imp (%)	Outlet name
Catchment 101	182	N.A	23	23	N.A	(1800 mm and 900mm diameter Existing Culverts)
Catchment 101-1	42	N.A	35	35	N.A	
Catchment 1001L (R)	N.A	0.2100	N.A	N.A	79	
Catchment 1001R (R)	N.A	0.2300	N.A	N.A	78	
Catchment 102	45	N.A	0	0	N.A	(500 mm diameter Existing Culvert)
Catchment 1002L (R)	N.A	0.7300	N.A	N.A	71	
Catchment 1002R (R)	N.A	0.8300	N.A	N.A	75	
Catchment 103	384	N.A	0	0	N.A	(1100mm x 1500 mm diameter Existing Culvert)
Catchment 1003L (R)	N.A	0.3800	N.A	N.A	66	
Catchment 1003R (R)	N.A	0.3900	N.A	N.A	79	
Catchment 104	242	N.A	0	0	N.A	(800mm diameter Existing Culvert)
Catchment 1004L (R)	N.A	0.3500	N.A	N.A	69	
Catchment 1004R (R)	N.A	0.3800	N.A	N.A	74	
Catchment 105	135	N.A	0	0	N.A	(1200mm and 800 mm diameters Existing Culverts)
Catchment 1005L (R)	N.A	0.3700	N.A	N.A	66	
Catchment 1005R (R)	N.A	0.3600	N.A	N.A	62	
Catchment 106	29	N.A	0	0	N.A	(500mm x 700 mm diameter Existing Culverts)
Catchment 1006L (R)	N.A	0.6500	N.A	N.A	65	
Catchment 1006R (R)	N.A	0.6600	N.A	N.A	62	
Catchment 107	59.6	N.A	0	0	N.A	(800mm diameter Existing Culvert)
Catchment 1007L (R)	N.A	0.7200	N.A	N.A	60	
Catchment 1007R (R)	N.A	0.7400	N.A	N.A	71	
Catchment 108-1	207	N.A	0	0	N.A	(700mm x 1000 mm diameter Existing Culverts)
Catchment 1008-L	N.A	0.21	N.A	N.A	52	
Catchment 1008-R	N.A	0.16	N.A	N.A	87	
Catchment 108-2	207	N.A	0	0	N.A	(900mm diameter Existing Culvert)
Catchment 1008-L	N.A	0.21	N.A	N.A	52	
Catchment 1008-R	N.A	0.21	N.A	N.A	87	
Catchment 109	6	N.A	0	0	N.A	(450mm diameter Existing Culvert)
Catchment 1009L	N.A	0.50	N.A	N.A	74	
Catchment 1009R	N.A	0.63	N.A	N.A	81	
Catchment 110	178	N.A	38	38	N.A	(Existing Bridge 6 m x 3.2 m)
Catchment 1100 L	N.A	0.91	N.A	N.A	55	
Catchment 1100R	N.A	0.88	N.A	N.A	72	
Catchment 111	109	N.A	0	0	N.A	Existing Culvert
Catchment 1110-L	N.A	1.06	N.A	N.A	59	Existing Culvert
Catchment 1110-R	N.A	0.97	N.A	N.A	78	Existing Culvert
Catchment 112	192	N.A	0	0	N.A	Existing Culvert
Catchment 1120-L	N.A	1.21	N.A	N.A	68	Existing Culvert
Catchment 1120-R	N.A	1.22	N.A	N.A	69	Existing Culvert
Total		15.29	68.1			



Subject Post-Development Drainage Areas and LID Units Footprint Areas

Post-Development Area Breakdown

Catchment Name	Road Catchment Area (ha)	TIMP (%)	XIMP (%)	LID Footprint Area (ha)	Total Pervious Area (ha)	Outlet name
Catchment 2001.1(L)- Context 3	0.100	90	90	0.006	0.01	Existing storm pipe
Catchment 2001.1 R- Context 3	0.099	79	79	0.011	0.02	
Catchment Name	Road Catchment Area (ha)	TIMP (%)	XIMP (%)	LID Footprint Area (ha)	Total Pervious Area (ha)	Outlet name
Catchment 2001.2(L)- Context 1	0.11	95	95	0.01	0.01	(1800 mm and 900mm diameter Existing Culverts)
Catchment 2001.2(R)- Context 1	0.13	96	96	0.01	0.01	
Catchment 2002L (R) -Context 1	0.70	94	94	0.044	0.08	(500 mm diameter Existing Culvert)
Catchment 2002R (R) (Context 1)	0.80	95	95	0.04	0.12	
Catchment 2003L (R) (Context 1)	0.43	90	90	0.0167	0.04	(1100mm x 1500 mm diameter Existing Culvert)
Catchment 2003R (R) (Context 1)	0.47	87	87	0.04	0.06	
Catchment 2004L (R) (Context 1)	0.35	89	89	0.015	0.04	(800mm diameter Existing Culvert)
Catchment 2004R (R) (Context 1)	0.36	85	85	0.05	0.05	
Catchment 2005L (R) (Context 1)	0.28	90	90	0.01	0.03	(1200mm and 800 mm diameters Existing Culverts)
Catchment 2005R (R) (Context 1)	0.29	78	78	0.03	0.06	
Catchment 2006L (R) (Context 1)	0.65	88	88	0.05	0.08	(500mm x 700 mm diameter Existing Culverts)
Catchment 2006R (R) (Context 1)	0.73	80	80	0.03	0.15	
Catchment 2007L (R) (Context 1)	0.58	85	85	0.06	0.09	(800mm diameter Existing Culvert)
Catchment 2007R (R) (Context 1)	0.64	84	84	0.07	0.11	
Catchment 20081-L (Context 1)	0.07	84	84	0.007	0.011	700mm x 1000 mm diameter Existing Culverts and 900mm diameter Existing Culvert
Catchment 20081-R (Context 1)	0.07	83	83	0.006	0.012	
Catchment Name	Road Catchment Area (ha)	TIMP (%)	XIMP (%)	LID Footprint Area (ha)	Total Pervious Area (ha)	Outlet name
Catchment 20082-L (Context 3)	0.38	70	70	0.0434	0.117	(450mm diameter Existing Culvert)
Catchment 20082-R (Context 3)	0.39	59	59	0.043	0.160	
Catchment 2009L (Context 3)	0.40	77	77	0.032	0.093	(450mm diameter Existing Culvert)
Catchment 2009R (Context 3)	0.39	77	77	0.037	0.091	
Catchment 2010 L (Context 3)	0.25	82	82	0.02	0.04	(Existing Bridge 6 m x 3.2 m)
Catchment 2010R (Context 3)	0.25	82	82	0.01	0.04	
Catchment Name	Road Catchment Area (ha)	TIMP (%)	XIMP (%)	LID Footprint Area (ha)	Total Pervious Area (ha)	Outlet name
Catchment 2010 L (Context 1)	0.47	74	74	0.033	0.12	(Existing Bridge 6 m x 3.2 m)
Catchment 2010R (Context 1)	0.47	86	86	0.02	0.07	
Catchment 2010 L Total (Context 1, 3)	0.72	77				(Existing Bridge 6 m x 3.2 m)
Catchment 2010 R Total (Context 1, 3)	0.72	85				
Catchment 2011 L (Context 3,2)	0.87	86	86	0.05	0.12	(450mm diameter Existing Culvert)
Catchment 2011 R (Context 3,2)	0.90	85	85	0.06	0.14	
Catchment 2012-L (Context 2)	1.11	56	56	0.07	0.49	Existing 400 mm Culvert
Catchment 2012-R (Context 2)	1.11	93	93	0.08	0.08	
Total	15.289	82				

Ratios of impervious drainage areas to bioswales **14.4**
Ratios of impervious drainage areas to Chambers **10.7**



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			5

Subject **Required Storage Volumes within LID units to meet Stormwater Management Control**

Catchment Name	Infiltration Gallery Footprint Area (ha)	LID Required Storage Volume for Quality Control (m ³)	Infiltration Chamber Required Storage Volume to retain 12.5 mm of rainfall depth (m ³)	Infiltration Gallery Provided Volume (m)	Unit Depth (m)
Catchment 2001.1(L)- Context 3	0.006	5	11.2	27.7	1.2
Catchment 2001.1 R- Context 3	0.011	4	9.8	31.0	0.7
Catchment Name	LID Footprint Area (ha)	LID Required Storage Volume for Quality Control (m ³)	LID Required Storage Volume to retain 12.5 mm of rainfall depth (m ³)	LID Provided Volume (m)	Unit Depth (m)
Catchment 2001.2(L)- Context 1	0.01	8	13.1	39	1.9
Catchment 2001.2(R)- Context 1	0.01	1.25	15.6	30.9	1.2
Catchment 2002 (L)- Context 1	0.04	46.39	82.0	254.6	1.2
Catchment 2002(R)- Context 1	0.04	54.83	94.5	268.9	1.2
Catchment 2003 (L)- Context 1	0.02	23.1	48.2	166	2.3
Catchment 2003(R)- Context 1	0.04	21.1	50.9	228	1.2
Catchment 2004 (L)- Context 1	0.015	18.1	38.9	111	1.6
Catchment 2004(R)- Context 1	0.05	14.9	38.4	269	1.2
Catchment 2005 (L)- Context 1	0.008	15	31.4	64	1.5
Catchment 2005(R)- Context 1	0.03	11	28.3	185	1.2
Catchment 2006 (L)- Context 1	0.05	31	71.2	291	1.2
Catchment 2006 (R)- Context 1	0.03	28	72.9	240	1.9
Catchment 2007 (L)- Context 1	0.06	23	61.7	358	1.2
Catchment 2007 (R)- Context 1	0.64	23	66.8	400	1.2
Catchment 20081 (L)- Context 1	0.01	2.54	7.1	45	1.6
Catchment 20082 (L)- Context 3	0.04	13.34	33.3	173	1.00
Catchment 2008 (L)- Context 1&3	Total	16	40.3	219	
Catchment 20081 (R)- Context 1	0.006	3	7.6	36	1.3
Catchment 20082 (R)- Context 3	0.04	12	28.3	172	1.00
Catchment 2008 (R)- Context 1&3	Total	161		208	
Catchment 2009 (L)- Context 3	0.032	15	38.4	63	0.50
Catchment 2009 (R)- Context 3	0.037	15	37.4	67	0.50
Catchment 20101 (L)- Context 3	0.022	10	25.6	43	0.50
Catchment 20102 (L)- Context 1	0.033	17	43.8	191	1.2
Catchment 2010 (L)	Total	27	69.4	235	
Catchment 20101(R)- Context 3	0.027	10	25.6	140	1.3
Catchment 20102 (R)- Context 1	0.024	19	76.1	152	1.5
Catchment 2010 (R)	Total	29		292	
Catchment 2011 (L)- Context 3,2	0.053	30	93.3	258	1.2
Catchment 2011 (R)- Context 3,2	0.056	36	95.2	268	1.2
Catchment 2012 (L)- Context 2	0.0693	34	77.5	314	1.2
Catchment 2012 (R)- Context 2	0.077	47	129.1	388	1.3
Total				5285	



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			6

Subject **Infiltration Rate Calculations**

Table C 2: Approximate relationships between hydraulic conductivity, percolation time and infiltration rate

Hydraulic Conductivity, K_p (centimetres/second)	Percolation Time, T (minutes/centimetre)	Infiltration Rate, 1/T (millimetres/hour)
0.1	2	300
0.01	4	150
0.001	8	75
0.0001	12	50
0.00001	20	30
0.000001	50	12

Source: Ontario Ministry of Municipal Affairs and Housing (OMMAH). 1997. Supplementary Guidelines to the Ontario Building Code 1997. SG-6 Percolation Time and Soil Descriptions. Toronto, Ontario.

Percolation Rate:	BH3	0.0100	cm/s	150.0
	BH12	0.001	cm/s	75.0
	BH1	0.000001	cm/s	50.0
	BH5	0.00000001	cm/s	

Table C 3: Safety correction factors for calculating design infiltration rates

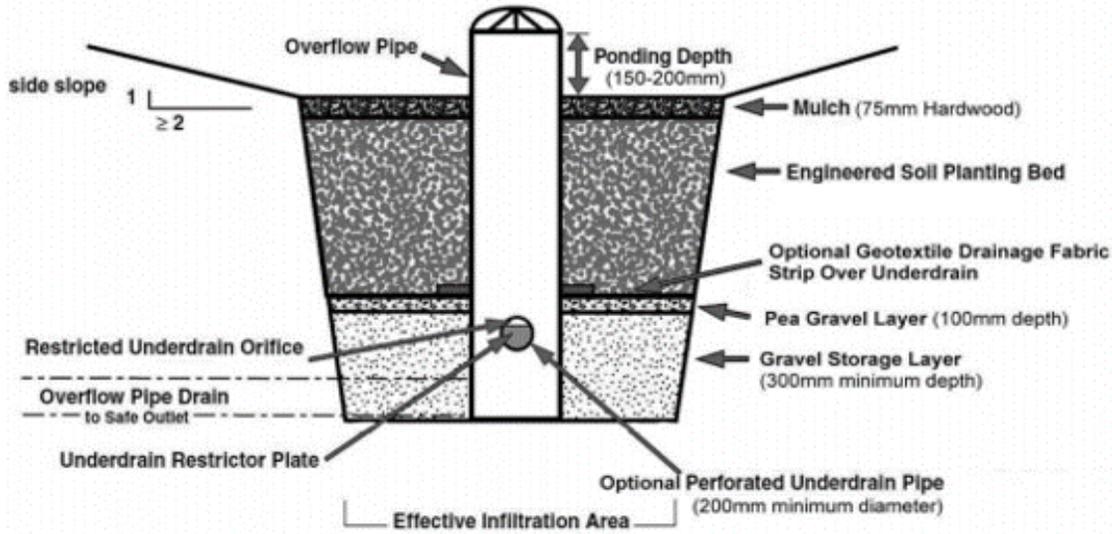
Ratio of Mean Measured Infiltration Rates ¹	Safety Correction Factor ²
≤ 1	2.5
1.1 to 4.0	3.5
4.1 to 8.0	4.5
8.1 to 16.0	6.5
16.1 or greater	8.5

Source: Wisconsin Department of Natural Resources. 2004. Conservation Practice Standards. Site Evaluation for Stormwater Infiltration (1002). Madison, WI.

	Ratio	Safety Factor	Infiltration Rate
BH3	12.50	6.50	23.08 mm/hr
BH12	6.25	4.50	33.33
BH1	1.0000	2.50	4.80
BH5	NA		



Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**



Bioswale

Bioswale Catchment 2001.2 (L):			Bioswale Catchment 2001.2 (R):		
Base Area	Catchment 2001.2 (L)	56.00 m ²	Base Area	Catchment 2001.2 (R)	55.00 m ²
mulch depth		0.075 m	mulch depth		0.075 m
Porosity of mulch		70%	Porosity of mulch		70%
Storage in mulch I		2.940 m ³	Storage in mulch I		2.888 m ³
Filter Media Depth		1.40 m	Filter Media Depth		0.65 m
Porosity of Filter Media		20%	Porosity of Filter Media		20%
Storage in voids of filter media		16 m ³	Storage in voids of filter media		7 m ³
Percolation rate within gravel layer		300.0 mm/hr	Percolation rate within gravel layer		300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)		5.0 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)		2.5 hr
Ponding Depth		0.2 m	Ponding Depth		0.2 m
Surface Storage		11.2 m ³	Surface Storage		11.0 m ³
Percolation rate within filter media layer		25.0 mm/hr	Percolation rate within filter media layer		25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)		11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)		11.0 hr
Gravel layer depth		0.30 m	Gravel layer depth		0.35 m
Porosity of Gravel layer		40%	Porosity of Gravel layer		40%
Storage in voids of gravel		6.7 m ³	Storage in voids of gravel		7.7 m ³
Percolation rate within native soil		33.3 mm/hr	Percolation rate within native soil		33.3 mm/hr
Drawdown time within native soil		9.0 hr	Drawdown time within native soil		10.5 hr
Chocker course		0.1 m	Chocker course		0.1 m
Porosity of Chocker course		40%	Porosity of Chocker course		40%
Storage in chocker course layer		2.2 m ³	Storage in chocker course layer		2.2 m ³
Total depth of system		1.9 m	Total depth of system		1.18 m
Total Storage		39 m ³	Total Storage		31 m ³
Total drawdown time		25.0 hr	Total drawdown time		24.0 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 8

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 2002 (L):		Bioswale Catchment 2002(R):	
Base Area Catchment 2002 (L)	437.00 m ²	Base Area Catchment 2002 (R)	439.00 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l:	22.943 m ³	Storage in mulch l	23.048 m ³
Filter Media Depth	0.65 m	Filter Media Depth	0.50 m
Porosity of Filter Media	20%	Porosity of Filter Media	20%
Storage in voids of filter media	57 m ³	Storage in voids of filter media	44 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	2.5 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	2.0 hr
Ponding Depth	0.2 m	Ponding Depth	0.2 m
Surface Storage	87.4 m ³	Surface Storage	87.8 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr
Gravel layer depth	0.40 m	Gravel layer depth	0.55 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	69.9 m³	Storage in voids of gravel	96.6 m³
Percolation rate within native soil	33.3 mm/hr	Percolation rate within native soil	33.3 mm/hr
Drawdown time within native soil	12.0 hr	Drawdown time within native soil	16.5 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	17.5 m ³	Storage inchocker course layer	17.6 m ³
Total depth of system	1.23 m	Total depth of system	1.23 m
Total Storage	255 m ³	Total Storage	269 m ³
Total drawdown time	25.5 hr	Total drawdown time	29.5 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 9

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 2003 (L):		Bioswale Catchment 2003 (R):	
Base Area Catchment 2003 (L)	167.00 m ²	Base Area Catchment 2003 (R)	399.00 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l:	8.768 m ³	Storage in mulch l	20.948 m ³
Filter Media Depth	1.40 m	Filter Media Depth	0.70 m
Porosity of Filter Media	30%	Porosity of Filter Media	20%
Storage in voids of filter media	70 m ³	Storage in voids of filter media	56 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	5.0 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	2.7 hr
Ponding Depth	0.2 m	Ponding Depth	0.2 m
Surface Storage	33.4 m ³	Surface Storage	79.8 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr
Gravel layer depth	0.70 m	Gravel layer depth	0.35 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	46.8 m ³	Storage in voids of gravel	55.9 m ³
Percolation rate within native soil	33.3 mm/hr	Percolation rate within native soil	33.3 mm/hr
Drawdown time within native soil	21.0 hr	Drawdown time within native soil	10.5 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	6.7 m ³	Storage inchocker course layer	16.0 m ³
Total depth of system	2.28 m	Total depth of system	1.2 m
Total Storage	166 m ³	Total Storage	228 m ³
Total drawdown time	37.0 hr	Total drawdown time	24.2 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			10

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 2004 (L):		Bioswale Catchment 2004 (R):	
Base Area Catchment 2004 (L)	150.00 m ²	Base Area Catchment 2004 (R)	454.00 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l	7.875 m ³	Storage in mulch l	23.835 m ³
Filter Media Depth	0.75 m	Filter Media Depth	0.50 m
Porosity of Filter Media	25%	Porosity of Filter Media	20%
Storage in voids of filter media	28 m ³	Storage in voids of filter media	45 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	2.8 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	2.0 hr
Ponding Depth	0.2 m	Ponding Depth	0.2 m
Surface Storage	30.0 m ³	Surface Storage	90.8 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr
Gravel layer depth	0.65 m	Gravel layer depth	0.50 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	39.0 m³	Storage in voids of gravel	90.8 m³
Percolation rate within native soil	33.3 mm/hr	Percolation rate within native soil	33.3 mm/hr
Drawdown time within native soil	19.5 hr	Drawdown time within native soil	15.0 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	6.0 m ³	Storage inchocker course layer	18.2 m ³
Total depth of system	1.58 m	Total depth of system	1.18 m
Total Storage	111 m ³	Total Storage	269 m ³
Total drawdown time	33.3 hr	Total drawdown time	28.0 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			11

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 2005 (L):		Bioswale Catchment 2005 (R):	
Base Area Catchment 2005 (L)	84.87 m ²	Base Area Catchment 2005 (R)	313.00 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l:	4.456 m ³	Storage in mulch l	16.433 m ³
Filter Media Depth	0.40 m	Filter Media Depth	0.50 m
Porosity of Filter Media	20%	Porosity of Filter Media	20%
Storage in voids of filter media	7 m ³	Storage in voids of filter media	31 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	1.7 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	2.0 hr
Ponding Depth	0.2 m	Ponding Depth	0.2 m
Surface Storage	17.0 m ³	Surface Storage	62.6 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr
Gravel layer depth	0.95 m	Gravel layer depth	0.50 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	32.3 m ³	Storage in voids of gravel	62.6 m ³
Percolation rate within native soil	33.3 mm/hr	Percolation rate within native soil	33.3 mm/hr
Drawdown time within native soil	28.5 hr	Drawdown time within native soil	15.0 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	3.4 m ³	Storage inchocker course layer	12.5 m ³
Total depth of system	1.53 m	Total depth of system	1.18 m
Total Storage	64 m ³	Total Storage	185 m ³
Total drawdown time	41.2 hr	Total drawdown time	28.0 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 12

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 2006 (L):		Bioswale Catchment 2006 (R):	
Base Area Catchment 2006 (L)	491.00 m ²	Base Area Catchment 2006 (R)	319.00 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l:	25.778 m ³	Storage in mulch l	16.748 m ³
Filter Media Depth	0.50 m	Filter Media Depth	1.10 m
Porosity of Filter Media	20%	Porosity of Filter Media	20%
Storage in voids of filter media	49 m ³	Storage in voids of filter media	70 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	2.0 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	4.0 hr
Ponding Depth	0.2 m	Ponding Depth	0.2 m
Surface Storage	98.2 m ³	Surface Storage	63.8 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr
Gravel layer depth	0.50 m	Gravel layer depth	0.60 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	98.2 m ³	Storage in voids of gravel	76.6 m ³
Percolation rate within native soil	33.3 mm/hr	Percolation rate within native soil	33.3 mm/hr
Drawdown time within native soil	15.0 hr	Drawdown time within native soil	18.0 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	19.6 m ³	Storage inchocker course layer	12.8 m ³
Total depth of system	1.18 m	Total depth of system	1.88 m
Total Storage	291 m ³	Total Storage	240 m ³
Total drawdown time	28.0 hr	Total drawdown time	33.0 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			13

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 2007 (L):		Bioswale Catchment 2007 (R):	
Base Area Catchment 2007 (L)	648.00 m ²	Base Area Catchment 2007 (R)	698.00 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l	34.020 m ³	Storage in mulch l	36.645 m ³
Filter Media Depth	0.70 m	Filter Media Depth	0.60 m
Porosity of Filter Media	20%	Porosity of Filter Media	20%
Storage in voids of filter media	91 m ³	Storage in voids of filter media	84 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	2.7 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	2.3 hr
Ponding Depth	0.2 m	Ponding Depth	0.2 m
Surface Storage	129.6 m ³	Surface Storage	139.6 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr
Gravel layer depth	0.30 m	Gravel layer depth	0.40 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	77.8 m ³	Storage in voids of gravel	111.7 m ³
Percolation rate within native soil	33.3 mm/hr	Percolation rate within native soil	33.3 mm/hr
Drawdown time within native soil	9.0 hr	Drawdown time within native soil	12.0 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	25.9 m ³	Storage inchocker course layer	27.9 m ³
Total depth of system	1.18 m	Total depth of system	1.18 m
Total Storage	358 m ³	Total Storage	400 m ³
Total drawdown time	22.7 hr	Total drawdown time	25.3 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			14

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 20081 (L):		Bioswale Catchment 20081 (R):	
Base Area Catchment 20081 (L)	69.21 m ²	Base Area Catchment 20081 (R)	61.12 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l	3.634 m ³	Storage in mulch l	3.209 m ³
Filter Media Depth	1.00 m	Filter Media Depth	0.70 m
Porosity of Filter Media	20%	Porosity of Filter Media	20%
Storage in voids of filter media	14 m ³	Storage in voids of filter media	9 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	3.7 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	2.7 hr
Ponding Depth	0.2 m	Ponding Depth	0.2 m
Surface Storage	13.8 m ³	Surface Storage	12.2 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr
Gravel layer depth	0.40 m	Gravel layer depth	0.40 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	11.1 m ³	Storage in voids of gravel	9.8 m ³
Percolation rate within native soil	23.1 mm/hr	Percolation rate within native soil	23.1 mm/hr
Drawdown time within native soil	17.3 hr	Drawdown time within native soil	17.3 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	2.8 m ³	Storage inchocker course layer	2.4 m ³
Total depth of system	1.58 m	Total depth of system	1.28 m
Total Storage	45 m ³	Total Storage	36 m ³
Total drawdown time	32.0 hr	Total drawdown time	31.0 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			15

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 2010 (L):		Bioswale Catchment 2010 (R):	
Base Area Catchment 2010 (L)	334.47 m ²	Base Area Catchment 2010 (R)	240.00 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l:	17.560 m ³	Storage in mulch l	12.600 m ³
Filter Media Depth	0.60 m	Filter Media Depth	0.90 m
Porosity of Filter Media	20%	Porosity of Filter Media	20%
Storage in voids of filter media	40 m ³	Storage in voids of filter media	43 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	2.3 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	3.3 hr
Ponding Depth	0.2 m	Ponding Depth	0.2 m
Surface Storage	66.9 m ³	Surface Storage	48.0 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	60.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	11.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	4.6 hr
Gravel layer depth	0.40 m	Gravel layer depth	0.40 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	53.5 m ³	Storage in voids of gravel	38.4 m ³
Percolation rate within native soil	8.0 mm/hr	Percolation rate within native soil	8.0 mm/hr
Drawdown time within native soil	50.0 hr	Drawdown time within native soil	50.0 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	13.4 m ³	Storage inchocker course layer	9.6 m ³
Total depth of system	1.18 m	Total depth of system	1.48 m
Total Storage	191 m ³	Total Storage	152 m ³
Total drawdown time	63 hr	Total drawdown time	58 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			16

Subject **Proposed Bioswale Systems Total Depth, Provided Storage Volumes and Draw Down Time**

Bioswale

Bioswale Catchment 2011 (L):		Bioswale Catchment 2011 (R):	
Base Area Catchment 2011 (L)	534.00 m ²	Base Area Catchment 2011 (R)	555.00 m ²
mulch depth	0.075 m	mulch depth	0.075 m
Porosity of mulch	70%	Porosity of mulch	70%
Storage in mulch l	28.035 m ³	Storage in mulch l	29.138 m ³
Filter Media Depth	0.55 m	Filter Media Depth	0.55 m
Porosity of Filter Media	20%	Porosity of Filter Media	20%
Storage in voids of filter media	59 m ³	Storage in voids of filter media	61 m ³
Percolation rate within gravel layer	300.0 mm/hr	Percolation rate within gravel layer	300.0 mm/hr
Drawdown time within gravel layer (filter media and chocker course layer were included)	2.2 hr	Drawdown time within gravel layer (filter media and chocker course layer were included)	2.2 hr
Ponding Depth	0.1 m	Ponding Depth	0.1 m
Surface Storage	53.4 m ³	Surface Storage	55.5 m ³
Percolation rate within filter media layer	25.0 mm/hr	Percolation rate within filter media layer	25.0 mm/hr
Drawdown time within filter media (ponding depth and mulch later were included)	8.0 hr	Drawdown time within filter media (ponding depth and mulch later were included)	8.0 hr
Gravel layer depth	0.45 m	Gravel layer depth	0.45 m
Porosity of Gravel layer	40%	Porosity of Gravel layer	40%
Storage in voids of gravel	96.1 m³	Storage in voids of gravel	99.9 m³
Percolation rate within native soil	8.0 mm/hr	Percolation rate within native soil	8.0 mm/hr
Drawdown time within native soil	56.3 hr	Drawdown time within native soil	56.3 hr
Chocker course	0.1 m	Chocker course	0.1 m
Porosity of Chocker course	40%	Porosity of Chocker course	40%
Storage inchocker course layer	21.4 m ³	Storage inchocker course layer	22.2 m ³
Total depth of system	1.18 m	Total depth of system	1.18 m
Total Storage	258 m³	Total Storage	268 m³
Total drawdown time	66 hr	Total drawdown time	66 hr



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-08-04
Checked	Steven Van Haren	Checked	
			Page 17

Subject Proposed Bioswale Systems Total Depth, Provided Storage Volumes, Draw Down Time & Maintenance cost

Bioswale

Bioswale Catchment 2012 (L):			Bioswale Catchment 2012 (R):		
Base Area	Catchment 2012 (L)	693.00 m ²	Base Area	Catchment 2012 (R)	773.00 m ²
	mulch depth	0.075 m		mulch depth	0.075 m
	Porosity of mulch	70%		Porosity of mulch	70%
	Storage in mulch I	36.383 m ³		Storage in mulch I	40.583 m ³
	Filter Media Depth	0.70 m		Filter Media Depth	0.65 m
	Porosity of Filter Media	20%		Porosity of Filter Media	20%
	Storage in voids of filter media	97 m ³		Storage in voids of filter media	100 m ³
	Percolation rate within gravel layer	300.0 mm/hr		Percolation rate within gravel layer	300.0 mm/hr
	Drawdown time within gravel layer (filter media and chocker course layer were included)	2.7 hr		Drawdown time within gravel layer (filter media and chocker course layer were included)	2.5 hr
	Ponding Depth	0.1 m		Ponding Depth	0.1 m
	Surface Storage	69.3 m ³		Surface Storage	77.3 m ³
	Percolation rate within filter media layer	25.0 mm/hr		Percolation rate within filter media layer	25.0 mm/hr
	Drawdown time within filter media (ponding depth and mulch later were included)	7.0 hr		Drawdown time within filter media (ponding depth and mulch later were included)	7.0 hr
	Gravel layer depth	0.30 m		Gravel layer depth	0.45 m
	Porosity of Gravel layer	40%		Porosity of Gravel layer	40%
	Storage in voids of gravel	83.2 m³		Storage in voids of gravel	139.1 m³
	Percolation rate within native soil	8.0 mm/hr		Percolation rate within native soil	8.0 mm/hr
	Drawdown time within native soil	37.5 hr		Drawdown time within native soil	56.3 hr
	Chocker course	0.1 m		Chocker course	0.1 m
	Porosity of Chocker course	40%		Porosity of Chocker course	40%
	Storage inchocker course layer	27.7 m ³		Storage inchocker course layer	30.9 m ³
	Total depth of system	1.18 m		Total depth of system	1.28 m
	Total Storage	314 m³		Total Storage	388 m³
	Total drawdown time	47 hr		Total drawdown time	66 hr

Bioswale Maintenance Cost

Total Bioswale Footprint Area (m ²)	Cost per 130 m2	Total Cost
7967.67	\$952	\$58,347.86

Infiltration Chamber Maintenance Cost

Total Bioswale Footprint Area (m ²)	Cost per 104.7 m2	Total Cost
2043.39	\$1,212	\$23,654.14



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 18

Subject **Drain Pipe Sizing Calculations**

Catchment 2001.1(L)- Context 3

$$Q = CA\sqrt{2gh}$$

12- hr SCS 4- hr Chicago Storm

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.041	m ³ /s	0.041	0.049
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	150	mm		
	A = Cross-section Area of Orifice (m ²)	0.018	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	0.425	m		
	Invert of Orifice	0.00	m		

Catchment 2001.1 R- Context 3

$$Q = CA\sqrt{2gh}$$

12- hr SCS 4- hr Chicago Storm

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.036	m ³ /s	0.04	0.046
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	150	mm		
	A = Cross-section Area of Orifice (m ²)	0.018	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	0.325	m		
	Invert of Orifice			h = Water Head above Centerli	m

Catchment 2001.2 L-Context 1

$$Q = CA\sqrt{2gh}$$

12- hr SCS 4- hr Chicago Storm

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.084	m ³ /s	0.045	0.053
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	150	mm		
	A = Cross-section Area of Orifice (m ²)	0.018	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	1.800	m		
	Invert of Orifice	0.00	m		

Catchment 2001.2 R-Context 1

$$Q = CA\sqrt{2gh}$$

12- hr SCS 4- hr Chicago Storm

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.066	m ³ /s	0.053	0.063
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	150	mm		
	A = Cross-section Area of Orifice (m ²)	0.018	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	1.100	m		
	Invert of Orifice	0.00	m		



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 19

Subject **Drain Pipe Sizing Calculations**

Catchment 2002 L-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

0.285 0.339

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.399	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	375	mm
	A = Cross-section Area of Orifice (m ²)	0.110	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	1.038	m
	Invert of Orifice	0.00	m

Catchment 2002 R-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

0.321 0.381

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.399	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	375	mm
	A = Cross-section Area of Orifice (m ²)	0.110	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	1.038	m
	Invert of Orifice	0.00	m

Catchment 2003 L-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

0.175 0.209

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.255	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	250	mm
	A = Cross-section Area of Orifice (m ²)	0.049	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	2.150	m
	Invert of Orifice	#REF!	m

Catchment 2003 R-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

0.191 0.226

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.260	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	300	mm
	A = Cross-section Area of Orifice (m ²)	0.071	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	1.075	m
	Invert of Orifice	0.00	m



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 20

Subject **Drain Pipe Sizing Calculations**

Catchment 2004 L-Context 1

12- hr SCS 4- hr Chicago Storm
0.17 0.35

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.461	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	375	mm
	A = Cross-section Area of Orifice (m ²)	0.110	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	1.388	m
	Invert of Orifice	0.00	m

Catchment 2004 R-Context 1

12- hr SCS 4- hr Chicago Storm
0.146 0.172

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.200	m ³ /s
	C = Flow Coefficient for Orifice	0.63	
	d = Required Overflow Pipe Size (mm)	300	mm
	A = Cross-section Area of Orifice (m ²)	0.071	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	1.025	m
	Invert of Orifice	0.00	m

Catchment 2005 L-Context 1

12- hr SCS 4- hr Chicago Storm
0.114 0.136

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.133	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	200	mm
	A = Cross-section Area of Orifice (m ²)	0.031	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	1.425	m
	Invert of Orifice	0.00	m

Catchment 2005 R-Context 1

12- hr SCS 4- hr Chicago Storm
0.115 0.135

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.254	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	300	mm
	A = Cross-section Area of Orifice (m ²)	0.071	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	1.025	m
	Invert of Orifice	0.00	m



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 21

Subject **Drain Pipe Sizing Calculations**

Catchment 2006 L-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.389	m ³ /s	0.26	0.31
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	375	mm		
	A = Cross-section Area of Orifice (m ²)	0.110	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	0.988	m		
	Invert of Orifice	0.00	m		

Catchment 2006 R-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.508	m ³ /s	0.29	0.33
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	375	mm		
	A = Cross-section Area of Orifice (m ²)	0.110	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	1.688	m		
	Invert of Orifice	0.00	m		

Catchment 2007 L-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.389	m ³ /s	0.23	0.28
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	375	mm		
	A = Cross-section Area of Orifice (m ²)	0.110	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	0.988	m		
	Invert of Orifice	0.00	m		

Catchment 2007 R-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.389	m ³ /s	0.26	0.28
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	375	mm		
	A = Cross-section Area of Orifice (m ²)	0.110	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	0.988	m		
	Invert of Orifice	0.00	m		



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page 22

Subject **Drain Pipe Sizing Calculations**

Catchment 20081 L-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.034	m ³ /s	0.028	0.033
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	100	mm		
	A = Cross-section Area of Orifice (m ²)	0.008	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	1.525	m		
	Invert of Orifice	0.00	m		

Catchment 20082 L-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.353	m ³ /s	0.147	0.163
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	375	mm		
	A = Cross-section Area of Orifice (m ²)	0.110	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	0.813	m		
	Invert of Orifice	0.00	m		

Catchment 20081 R -Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.069	m ³ /s	0.033	0.07
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	150	mm		
	A = Cross-section Area of Orifice (m ²)	0.018	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	1.200	m		
	Invert of Orifice	0.00	m		

Catchment 20082 R-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.353	m ³ /s	0.151	0.39
	C = Flow Coefficient for Orifice	0.80			
	d = Required Overflow Pipe Size (mm)	375	mm		
	A = Cross-section Area of Orifice (m ²)	0.110	m ²		
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
	h = Water Head above Centerline of Orifice (m)	0.813	m		
	Invert of Orifice	0.00	m		



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			23

Subject **Drain Pipe Sizing Calculations**

Catchment 2009 L-Context 1 12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where, Q_{100} =Flow Rate (m ³ /s)	0.219	m ³ /s	0.157	0.179
C = Flow Coefficient for Orifice	0.80			
d = Required Overflow Pipe Size (mm)	375	mm		
A = Cross-section Area of Orifice (m ²)	0.110	m ²		
g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
h = Water Head above Centerline of Orifice (m)	0.313	m		
Invert of Orifice	0.00	m		

Catchment 2009 R-Context 1 12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where, Q_{100} =Flow Rate (m ³ /s)	0.219	m ³ /s	0.25	0.296
C = Flow Coefficient for Orifice	0.80			
d = Required Overflow Pipe Size (mm)	375	mm		
A = Cross-section Area of Orifice (m ²)	0.110	m ²		
g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
h = Water Head above Centerline of Orifice (m)	0.313	m		
Invert of Orifice	0.00	m		

Catchment 2010 L -Context 1 12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where, Q_{100} =Flow Rate (m ³ /s)	0.389	m ³ /s	0.284	0.325
C = Flow Coefficient for Orifice	0.80			
d = Required Overflow Pipe Size (mm)	375	mm		
A = Cross-section Area of Orifice (m ²)	0.110	m ²		
g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
h = Water Head above Centerline of Orifice (m)	0.988	m		
Invert of Orifice	0.00	m		

Catchment 2010 R-Context 1 12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

Where, Q_{100} =Flow Rate (m ³ /s)	0.444	m ³ /s	0.38	0.449
C = Flow Coefficient for Orifice	0.80			
d = Required Overflow Pipe Size (mm)	375	mm		
A = Cross-section Area of Orifice (m ²)	0.110	m ²		
g = Gravity Acceleration (m/s ²)	9.81	m/s ²		
h = Water Head above Centerline of Orifice (m)	1.288	m		
Invert of Orifice	0.00	m		



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	
			Page
			24

Subject **Drain Pipe Sizing Calculations**

Catchment 2011 L-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

0.331 0.364

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.389	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	375	mm
	A = Cross-section Area of Orifice (m ²)	0.110	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	0.988	m
	Invert of Orifice	0.00	m

Catchment 2011 R-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

0.356 0.409

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.389	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	375	mm
	A = Cross-section Area of Orifice (m ²)	0.110	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	0.988	m
	Invert of Orifice	0.00	m

Catchment 2012 L -Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

0.398 0.417

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.389	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	375	mm
	A = Cross-section Area of Orifice (m ²)	0.110	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	0.988	m
	Invert of Orifice	0.00	m

Catchment 2012 R-Context 1

12- hr SCS 4- hr Chicago Storm

$$Q = CA\sqrt{2gh}$$

0.44 0.506

Where,	Q ₁₀₀ =Flow Rate (m ³ /s)	0.408	m ³ /s
	C = Flow Coefficient for Orifice	0.80	
	d = Required Overflow Pipe Size (mm)	375	mm
	A = Cross-section Area of Orifice (m ²)	0.110	m ²
	g = Gravity Acceleration (m/s ²)	9.81	m/s ²
	h = Water Head above Centerline of Orifice (m)	1.088	m
	Invert of Orifice	0.00	m



Subject Water Quality Control

Require long-term average removal of 80% TSS on an annual loading basis from all runoff leaving the site.

7.1.1 Infiltration

The TSS removal efficiency of the Infiltration System shall be evaluated per Table 3.2 of 2003 MOECP SWMPDM.

Protection Level	SWMP Type	Storage Volume (m ³ /ha) for Impervious Level			
		35%	55%	70%	85%
Enhanced 80% long-term S.S. removal	Infiltration	25	30	35	40
	Wetlands	80	105	120	140
	Hybrid Wet Pond/Wetland	110	150	175	195
	Wet Pond	140	190	225	250

Quality Control Required Storage Volumes

Catchment	Area (ha)	IMP Imp (%)	Required Infiltration Volume		TSS Removal (%)
			(m ³)	(m ³ /ha)	
Catchment 2001.1(L)- Context 3	0.10	90	5.37	53.67	80.00
Catchment 2001.1 R- Context 3	0.10	79	3.78	38.15	80.00
Catchment 2001.2(L)- Context 1	0.11	95	7.67	69.73	80.00
Catchment 2001.2(R)- Context 1	0.13	96	1.25	72.31	80.00
Catchment 2002L (R) -Context 1	0.70	94	46.39	66.27	80.00
Catchment 2002R (R) (Context 1)	0.80	95	54.83	68.54	80.00
Catchment 2003L (R) (Context 1)	0.43	90	23.08	53.67	80.00
Catchment 2003R (R) (Context 1)	0.47	87	21.07	44.84	80.00
Catchment 2004L (R) (Context 1)	0.35	89	18.06	51.60	80.00
Catchment 2004R (R) (Context 1)	0.36	85	14.85	41.26	80.00
Catchment 2005L (R) (Context 1)	0.28	90	15.03	53.67	80.00
Catchment 2005R (R) (Context 1)	0.29	78	10.93	37.69	80.00
Catchment 2006L (R) (Context 1)	0.65	88	31.11	47.86	80.00
Catchment 2006R (R) (Context 1)	0.73	80	27.95	38.28	80.00
Catchment 2007L (R) (Context 1)	0.58	85	23.40	40.35	80.00
Catchment 2007R (R) (Context 1)	0.64	84	22.81	35.64	80.00
Catchment 20081-L (Context 1)	0.07	84	2.54	37.87	80.00
Catchment 20082-L (Context 3)	0.38	70	13.34	34.84	80.00
Catchment 20081-R (Context 1)	0.07	83	2.55	34.86	80.00
Catchment 20082-R (Context 3)	0.39	59	12.13	31.33	80.00
Catchment 2009-L (Context 3)	0.40	77	14.92	37.29	80.00
Catchment 2009-R (Context 3)	0.39	77	14.53	37.26	80.00
Catchment 20101-L (Context 3)	0.25	82	9.75	39.00	80.00
Catchment 20102-L (Context 1)	0.47	74	17.15	36.49	80.00
Catchment 20101-R (Context 3)	0.25	82	9.75	39.00	80.00
Catchment 20102-R (Context 1)	0.47	86	18.93	40.28	80.00
Catchment 2011-L (Context 3,2)	0.87	86	29.58	34.00	80.00
Catchment 2011-R (Context 3,2)	0.90	85	35.89	39.88	80.00
Catchment 2012-L (Context 2)	1.11	56	33.67	30.33	80.00
Catchment 2012-R (Context 2)	1.11	93	47.37	42.68	80.00
Total			589.68		



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	

Subject **Post to Pre Control (4 -hr Chicago Storm)**

Catchment Name	Pre-development Flow 4-hr Chicago Storm (m3/s)	Post-development Flow 4-hr Chicago Storm (m3/s)	Required Storage Volume (m3)	Available Storage Volume for Quantity Control (m3)
Catchment 2001 (L)- Contexts 1&3	0.098	0.102	25	29.2
Catchment 2001 R- Contexts 1& 3	0.107	0.109	24	31.5
Catchment 2002 (L)- Context 1	0.314	0.339	83	126.1
Catchment 2002(R)- Context 1	0.366	0.381	84	119.5
Catchment 2003 (L)- Context 1	0.159	0.209	80	94.5
Catchment 2003(R)- Context 1	0.182	0.226	79	156.5
Catchment 2004 (L)- Context 1	0.149	0.17	51	54.1
Catchment 2004(R)- Context 1	0.167	0.172	38	215.7
Catchment 2005 (L)- Context 1	0.155	0.136	N.A	17.5
Catchment 2005(R)- Context 1	0.142	0.135	91	146.2
Catchment 2006 (L)- Context 1	0.269	0.31	91	188.6
Catchment 2006 (R)- Context 1	0.26	0.332	114	139.2
Catchment 2007 (L)- Context 1	0.28	0.277	50	272.9
Catchment 2007 (R)- Context 1	0.319	0.304	N.A	310.0
Catchment 2008 (L)- Context 1&3	0.078	0.197	122	162.3
Catchment 2008 (R)- Context 1&3	0.101	0.184	90	157.6
Catchment 2009 (L)- Context 3	0.22	0.179	N.A	9.6
Catchment 2009 (R)- Context 3	0.296	0.296	N.A	15.4
Catchment 2010 (L)- Contexts 1&3	0.34	0.325	N.A	138.6
Catchment 2010 (R)- Contexts 1&3	0.38	0.449	140	161.8
Catchment 2011 (L)- Context 3,2	0.407	0.364	N.A	134.8
Catchment 2011 (R)- Context 3,2	0.435	0.109	N.A	136.7
Catchment 2012 (L)- Context 2	0.508	0.417	N.A	202.4
Catchment 2012 (R)- Context 2	0.516	0.506	N.A	212.0
Total				



Project	25th Side Road	No.	211-06027-00
By	Ellie Fazeli	Date	2022-05-18
Checked	Steven Van Haren	Checked	

Subject **Post to Pre Control (12-hr SCS II)**

Catchment Name	Pre-development Flow 12-hr SCS II (m3/s)	Post-development Flow 12- hr SCS II (m3/s)	Required Storage Volume (m3)	Available Storage Volume for Quantity Control (m3)
Catchment 2001 (L)- Contexts 1&3	0.083	0.085	25	29.2
Catchment 2001 R- Contexts 1& 3	0.091	0.092	27	31.5
Catchment 2002 (L)- Context 1	0.282	0.285	40	126.1
Catchment 2002(R)- Context 1	0.324	0.321	N.A	119.5
Catchment 2003 (L)- Context 1	0.142	0.175	94	94.5
Catchment 2003(R)- Context 1	0.154	0.191	102	156.5
Catchment 2004 (L)- Context 1	0.135	0.142	51	54.1
Catchment 2004(R)- Context 1	0.148	0.146	N.A	215.7
Catchment 2005 (L)- Context 1	0.138	0.114	N.A	17.5
Catchment 2005(R)- Context 1	0.133	0.115	N.A	146.2
Catchment 2006 (L)- Context 1	0.241	0.263	99	188.6
Catchment 2006 (R)- Context 1	0.243	0.289	135	139.2
Catchment 2007 (L)- Context 1	0.263	0.234	N.A	272.9
Catchment 2007 (R)- Context 1	0.285	0.256	N.A	310.0
Catchment 2008 (L)- Context 1&3	0.075	0.175	162	162.3
Catchment 2008 (R)- Context 1&3	0.085	0.171	141	157.6
Catchment 2009 (L)- Context 3	0.195	0.157	N.A	9.6
Catchment 2009 (R)- Context 3	0.25	0.25	N.A	15.4
Catchment 2010 (L)- Contexts 1&3	0.326	0.284	N.A	138.6
Catchment 2010 (R)- Contexts 1&3	0.34	0.38	159	161.8
Catchment 2011 (L)- Context 3,2	0.385	0.331	N.A	134.8
Catchment 2011 (R)- Context 3,2	0.382	0.356	N.A	136.7
Catchment 2012 (L)- Context 2	0.461	0.398	N.A	202.4
Catchment 2012 (R)- Context 2	0.466	0.44	N.A	212.0



Project	25th Side Road	No.	211-06027-00	
By	E.F	Date	2022-05-16	Page
Checked	S.V.H	Checked		27

Subject Water Balnce Calculations Pre-Development Conditions

Percipitation 932.9 mm According to Comprehensive Stormwater Managemnt Master Plan

Average Annual Precipitation 932.9 mm, or 0 m³
Annual Evapotranspiration 598.2 mm, or 0 m³

Available Water Surplus (or excess of precipitation over evapotranspiration)
 334.7 mm, or 0 m³

Then, infiltration factors are used to determine the fraction of water surplus that infiltrates into the ground and the fraction that runs off the site. Infiltration factor is determined by summing a factor for topography, soil, and cover.

Table 3.1: Hydrologic Cycle Component Values

**This is the total infiltration of which some discharges back to the stream as base flow. The infiltration factor is determined by summing a factor for topography, soils and cover.*

<u>Topography</u>	Flat Land, average slope < 0.6 m/km	0.3
	Rolling Land, average slope 2.8 m to 3.8 m/km	0.2
	Hilly Land, average slope 28 m to 47 m/km	0.1
<u>Soils</u>	Tight impervious clay	0.1
	Medium combinations of clay and loam	0.2
	Open Sandy loam	0.4
<u>Cover</u>	Cultivated Land	0.1
	Woodland	0.2

Pervious Area Pre-Development Conditions 4.87 ha
Impervious Area Pre-Development Conditions 10.42 ha

Pervious Area Pre-Development Conditions 4.87

Topography Factor 0.20
 Soil Factor 0.30 silt
 Cover 0.15

Therefore, the total infiltration factor is 0.65

Therefore, the annual infiltration amount is 388.8 mm, or 18,936
 and the annual runoff amount is 209.4 mm, or 10,196

	(%)	(mm)	(m ³)	Comments/Assumptions:
Infiltration	41.7%	388.8	18,936	...
Evapotranspiration	35.9%	334.7	16,300	...
Runoff	22.4%	209.4	10,196	...
Precipitation	100.0%	932.9	45,432	



Project	25 th Side Road	No.	211-06027-00	
By	E.F	Date	2022-05-16	Page
Checked	S.V.H	Checked		28

Subject Water Balnce Calculations Pre-Development Conditions

Impervious Area Pre-Development Conditions		10.42		
	(%)	(mm)	(m ³)	Comments/Assumptions:
Infiltration	0.0	0.0	0	...
Evapotranspiration	12.5	116.6	12,151	1 mm depression.
Runoff	87.5	816.3	85,057	...
Precipitation	100.0	932.9	97,208	

Pre-Development Conditions

Hydrologic Cycle Components	Pre-development Conditions (m ³)
Infiltration	18,936
Evapotranspiration	28,451
Runoff	95,253
Precipitation	142,640



Project	25th Side Road	No.	211-06027-00	
By	Ellie Fazeli	Date	2022-05-16	Page
Checked	A.M.B	Checked		30

Subject **Water Balance with Mitigation Measures**

Ratios of impervious drainage areas to bioswales: 14.5

Ratios of impervious drainage areas to underground chambers. 10.7

Land Use	Area (ha)	Cover %
Drainage Areas to Bioswales	9.944	65.0
Drainage Areas to the Underground Chamber	3.160	20.7
Remaining Pervious Area of the Road	2.184	14.3
Total Study Area	15.29	100.0

Infiltration Deficit	7,621	m ³ /year		
Average Annual Precipitation	932.9	mm, or	142,640	m ³
Annual Evapotranspiration	598.2	mm, or	91,465	m ³
Available Water Surplus (or excess of precipitation over evapotranspiration)				
	334.7	mm, or	51,176	m ³
Bioswale Units			70.0	% runoff reduction

	(%)	(mm)	m ³
Infiltration	35.0	326.5	32470
Evapotranspiration	35.0	326.5	32470
Runoff	30.0	279.9	27831
Precipitation	100.0	932.9	92771

Table 4.5.2 Volumetric runoff reduction¹ achieved by bioretention

LID Practice	Location	% Runoff Reduction ¹	Reference
Bioretention without underdrain	Connecticut	99%	Dietz and Clausen (2005)
	Pennsylvania	80%	Ermilio (2005)
	Pennsylvania	70%	Emerson and Traver (2004)
Bioretention with underdrain	North Carolina	40 to 60%	Smith and Hunt (2007)
	North Carolina	33 to 50%	Hunt and Lord (2006)
	Maryland and North Carolina	20 to 50%	Li <i>et al.</i> (2009)
Runoff Reduction Estimate²		85% without underdrain 45% with underdrain	

Infiltration Chamber 60.0 % runoff reduction

	(%)	(mm)	m ³
Infiltration	30.0	279.9	8845
Evapotranspiration	30.0	279.9	8845
Runoff	40.0	373.2	11794
Precipitation	100.0	932.9	29484

Table 4.4.2 Volumetric runoff reduction¹ achieved by infiltration trenches and perforated pipe systems

LID Practice	Location	Runoff Reduction ¹	Reference
Infiltration trench with underdrain	Virginia	60%	Schueler (1983)
Grass swale/ Perforated pipe system	Ontario	73%	J.F. Sabourin and Associates (2008a)
Grass swale/ Perforated pipe system	Ontario	86%	J.F. Sabourin and Associates (2008a)
Perforated pipe system	Ontario	95%	SWAMP (2005)
Perforated pipe system	Ontario	89%	SWAMP (2005)
Runoff Reduction Estimate²		85%	

Notes:



Project	25th Side Road	No.	211-06027-00	
By	Ellie Fazeli	Date	2022-05-16	Page
Checked	A.M.B	Checked		31

Subject Water Balance with Mitigation Measures

Pervious Area

	(%)	(mm)	m ³
Infiltration	41.7	388.8	8492
Evapotranspiration	35.9	334.7	7310
Runoff	22.4	209.4	4573
Precipitation	100.0	932.9	20376

Water Balance Components with Mitigation Measures

Hydrologic Cycle Components	Area #1 Drainage Areas to Bioswales	Area#2 Drainage Areas to Chamber Systems	Area#3 Remaining Pervious Area of the Road
% Land-Use Coverage	65.0%	20.7%	14.3%
Infiltration	326.5	279.9	388.8
Evapotranspiration	326.5	279.9	334.7
Runoff	279.9	373.2	209.4
Precipitation	932.9	932.9	932.9

Post-Development Condition Water Balance Relationship with Mitigation Measures

Hydrologic Cycle Components	Post-Development Condition Water Balance Relationship with Mitigation Measures		
	mm	%	m ³
% Land-Use Coverage		100.0%	
Infiltration	325.8	34.9%	49,808
Evapotranspiration	318.0	34.1%	48,625
Runoff	289.1	31.0%	44,198
Precipitation	932.9	100.0%	142,640

Pre-development Infiltration (Hydrogeology Report):	1375	m ³
Post-development Infiltration without Mitigation Measures (Hydrogeology Report):	919	m ³
Post-development Infiltration with Mitigation Measures:	49808	m³



Project	25th Side Road	No.	211-06027-00
By	E.F	Date	18-May-22
Checked	S.V.H	Page	26

Subject Phosphorus Loading Study - Calculation Summary Sheet

Catchment Description	Land Cover Classification	Proposed Treatment Method(s)	Area	'P' Export Coefficient	SWM Treatment Reduction	Annual 'P' Loading
			(ha)	(kg/ha/yr)	(%)	(kg/yr)
PRE-DEVELOPMENT CONDITIONS:						
Road Impervious Areas	High Residential	--	10.42	1.32	--	13.75
Road Pervious Areas	Hay-pasture		4.87	0.07		0.34
Totals	--	--	15.29	--	--	14.10
POST-DEVELOPMENT CONDITIONS (without water quality controls):						
Road Impervious Areas	High Residential	---	12.38	1.32	--	16.34
Road Pervious Areas	Hay-pasture		2.91	0.07		0.20
Totals	--	--	15.29	--	--	16.55
Target Removal Efficiency:					14.8	
POST-DEVELOPMENT CONDITIONS (with mitigation measures):						
Road Areas to Infiltration Chamber	High Residential	Infiltration Trench	3.16	1.32	100.0	0.00
Remaining Road Pervious Areas	Hay-pasture		2.19	0.07		0.15
Road Impervious to Bioswales	High Residential		9.94	1.32	80.0	2.62
Totals	--	--	15.29	--		2.78
Target Removal Efficiency:					83.2	

* From LSCRA Phosphorous Offsetting Policy (2017):

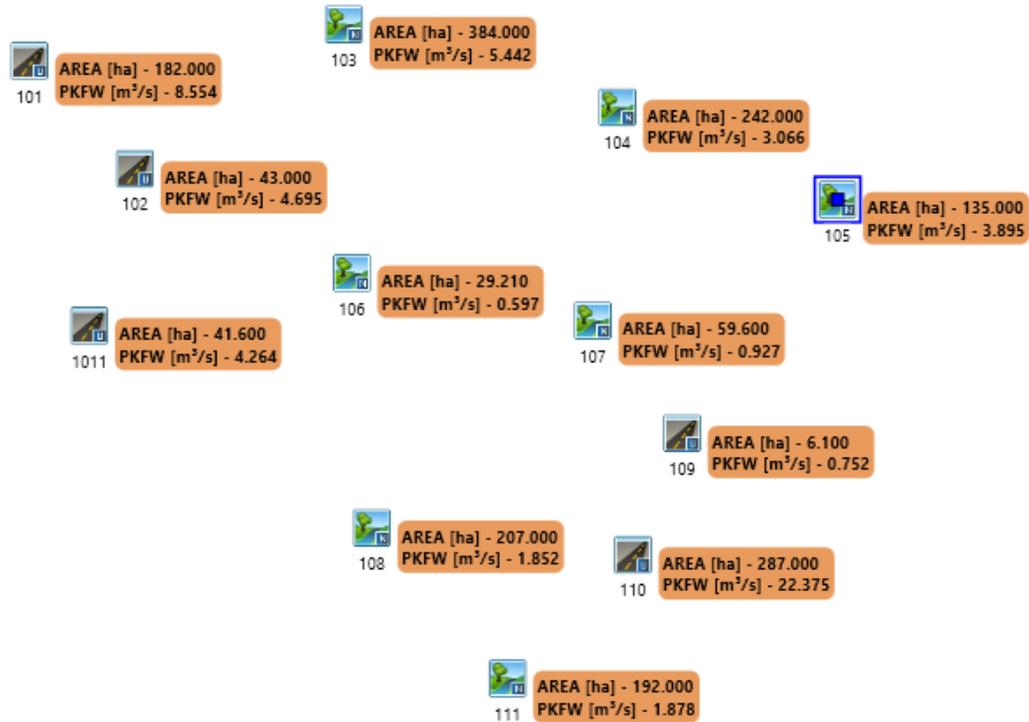
Net Effect on 'P' Load (kg/year)	2.78
* Offset Ratio (-)	2.5
* Offset Cost (/kg/year)	\$35,000
15% LSRCA Administration Fee	\$5,250
Total	\$248,256

APPENDIX

C

Visual OTTHYMO Results

25- year 4 hour Chicago Storm existing conditions



```

V V I SSSSS U U A L (v 6.1.2002)
V V I SS U A A L
V V I SS U AAAA L
V V I SS U A A L
W V I SSSSS UUUU A A LLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO
Developed and Distributed by Smart City Water Inc
copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

```

**** DETAILED OUTPUT ****

Input filename: C:\Program Files (x86)\Visual OTTHMO 6.1\VO2\voindat
Output filename: C:\Users\caef070146\AppData\Local\Civica\WH5\30564c49-8c07-4086-8157-8729fb3caba3\F0359d21
Summary filename: C:\Users\caef070146\AppData\Local\Civica\WH5\30564c49-8c07-4086-8157-8729fb3caba3\F0359d21

DATE: 09-14-2021 TIME: 08:38:27

USER:

COMMENTS:

```

***** SIMULATION : Run 04 *****
*****
READ STORM File: C:\Users\caef070146\AppData\Local\Temp\71794fb4-24ae-4a1d-b11d-298611ff8a03\53217781
Ptotal= 71.24 mm Comments: 25-year, 4 hr-Chicago, City of Barrie

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	5.22	1.17	36.37	2.17	11.74	3.17	6.15
0.33	5.94	1.33	148.15	2.33	10.09	3.33	5.74
0.50	6.93	1.50	47.06	2.50	8.89	3.50	5.38
0.67	8.42	1.67	25.72	2.67	7.96	3.67	5.08
0.83	10.91	1.83	18.11	2.83	7.24	3.83	4.80
1.00	16.13	2.00	14.17	3.00	6.65	4.00	4.57

```

CALIB NASHYD ( 0105) Area (ha)= 135.00 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.35

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	5.22	1.17	36.37	2.17	11.74	3.17	6.15
0.33	5.94	1.33	148.15	2.33	10.09	3.33	5.74
0.50	6.93	1.50	47.06	2.50	8.89	3.50	5.38
0.67	8.42	1.67	25.72	2.67	7.96	3.67	5.08
0.83	10.91	1.83	18.11	2.83	7.24	3.83	4.80
1.00	16.13	2.00	14.17	3.00	6.65	4.00	4.57

Unit Hyd Qpeak (cms)= 14.732

```

ID= 1 DT= 5.0 min Ia (mm)= 7.05 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 1.86

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	5.22	1.17	36.37	2.17	11.74	3.17	6.15
0.33	5.94	1.33	148.15	2.33	10.09	3.33	5.74
0.50	6.93	1.50	47.06	2.50	8.89	3.50	5.38
0.67	8.42	1.67	25.72	2.67	7.96	3.67	5.08
0.83	10.91	1.83	18.11	2.83	7.24	3.83	4.80
1.00	16.13	2.00	14.17	3.00	6.65	4.00	4.57

Unit Hyd Qpeak (cms)= 4.251

PEAK FLOW (cms)= 1.852 (i)
TIME TO PEAK (hrs)= 3.917
RUNOFF VOLUME (mm)= 13.490
TOTAL RAINFALL (mm)= 71.237
RUNOFF COEFFICIENT = 0.189

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB NASHYD ( 0111) Area (ha)= 192.00 Curve Number (CN)= 63.3
ID= 1 DT= 5.0 min Ia (mm)= 7.15 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 2.72

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	5.22	1.17	36.37	2.17	11.74	3.17	6.15
0.33	5.94	1.33	148.15	2.33	10.09	3.33	5.74
0.50	6.93	1.50	47.06	2.50	8.89	3.50	5.38
0.67	8.42	1.67	25.72	2.67	7.96	3.67	5.08
0.83	10.91	1.83	18.11	2.83	7.24	3.83	4.80
1.00	16.13	2.00	14.17	3.00	6.65	4.00	4.57

Unit Hyd Qpeak (cms)= 2.696

PEAK FLOW (cms)= 1.878 (i)
TIME TO PEAK (hrs)= 4.833
RUNOFF VOLUME (mm)= 19.421
TOTAL RAINFALL (mm)= 71.237
RUNOFF COEFFICIENT = 0.273

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB STANHYD ( 1011) Area (ha)= 41.60 Dir. Conn. (%) = 30.00
ID= 1 DT= 5.0 min Total Imp (%) = 35.00

```

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 14.56	27.04
Dep. Storage (mm)= 6.27	3.67
Average Slope (%)= 0.70	2.00
Length (m)= 526.62	1082.00
Mannings n = 0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

PEAK FLOW (cms)= 3.895 (i)
TIME TO PEAK (hrs)= 1.750
RUNOFF VOLUME (mm)= 14.818
TOTAL RAINFALL (mm)= 71.237
RUNOFF COEFFICIENT = 0.208
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

CALIB NASHYD ( 0106) Area (ha)= 29.21 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.61

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

----- TRANSFORMED HYETOGRAPH -----

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.083	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.167	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.250	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.333	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.417	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.500	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.583	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.667	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.750	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.833	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.917	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.000	4.57

Unit Hyd Qpeak (cms)= 1.829

```

PEAK FLOW (cms)= 0.597 (i)
TIME TO PEAK (hrs)= 2.083
RUNOFF VOLUME (mm)= 14.821
TOTAL RAINFALL (mm)= 71.237
RUNOFF COEFFICIENT = 0.208
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

CALIB NASHYD ( 0107) Area (ha)= 59.60 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.93

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

----- TRANSFORMED HYETOGRAPH -----

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.083	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.167	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.250	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.333	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.417	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.500	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.583	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.667	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.750	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.833	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.917	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.000	4.57

Unit Hyd Qpeak (cms)= 2.448

```

PEAK FLOW (cms)= 0.927 (i)
TIME TO PEAK (hrs)= 2.083
RUNOFF VOLUME (mm)= 14.821
TOTAL RAINFALL (mm)= 71.237
RUNOFF COEFFICIENT = 0.208
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

CALIB NASHYD ( 0108) Area (ha)= 207.00 Curve Number (CN)= 51.3

```

```

----- TRANSFORMED HYETOGRAPH -----

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.083	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.167	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.250	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.333	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.417	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.500	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.583	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.667	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.750	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.833	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.917	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.000	4.57

Max. Eff. Inten. (mm/hr)= 148.15
over (min)= 5.00
Storage Coeff. (min)= 6.58 (i) 111.43 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.18

TOTALS
PEAK FLOW (cms)= 4.25 0.46 4.264 (iii)
TIME TO PEAK (hrs)= 1.33 3.42 1.33
RUNOFF VOLUME (mm)= 64.97 22.66 35.35
TOTAL RAINFALL (mm)= 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.91 0.32 0.50

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 66.2 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB STANHYD ( 0102) Area (ha)= 43.00
ID= 1 DT= 5.0 min Total Imp (%) = 39.00 Dir. Conn. (%) = 31.00

```

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 16.77	26.23
Dep. Storage (mm)= 6.27	3.49
Average Slope (%)= 1.00	0.87
Length (m)= 535.41	1000.00
Mannings n = 0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

----- TRANSFORMED HYETOGRAPH -----

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.083	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.167	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.250	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.333	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.417	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.500	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.583	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.667	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.750	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.833	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.917	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.000	4.57

Max. Eff. Inten. (mm/hr)= 148.15 19.55
over (min)= 5.00 130.00
Storage Coeff. (min)= 5.97 (i) 126.05 (ii)
Unit Hyd. Tpeak (min)= 5.00 130.00
Unit Hyd. peak (cms)= 0.19 0.01

TOTALS
PEAK FLOW (cms)= 4.68 0.49 4.695 (iii)
TIME TO PEAK (hrs)= 1.33 3.67 1.33
RUNOFF VOLUME (mm)= 64.97 25.39 37.66
TOTAL RAINFALL (mm)= 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.91 0.36 0.53

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 66.2 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, NASHYD (0103), Area (ha), Curve Number (CN), ID=1 DT=5.0 min, Ia (mm), # of Linear Res. (N), U.H. Tp (hrs).

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

Unit Hyd. Peak (cms) = 7.843
PEAK FLOW (cms) = 5.442 (i)
TIME TO PEAK (hrs) = 3.833
RUNOFF VOLUME (mm) = 21.339
TOTAL RAINFALL (mm) = 71.237
RUNOFF COEFFICIENT = 0.300

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, NASHYD (0104), Area (ha), Curve Number (CN), ID=1 DT=5.0 min, Ia (mm), # of Linear Res. (N), U.H. Tp (hrs).

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

Unit Hyd. Peak (cms) = 5.568
PEAK FLOW (cms) = 3.066 (i)
TIME TO PEAK (hrs) = 3.583
RUNOFF VOLUME (mm) = 17.651
TOTAL RAINFALL (mm) = 71.237
RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, STANDHYD (0101), Area (ha), Total Imp (%), ID=1 DT=5.0 min, Dir. Conn. (%).

Surface Area (ha) = 38.22
Dep. Storage (mm) = 6.27
Average Slope (%) = 0.70
IMPERVIOUS (%) = 38.22
PERVIOUS (i) (%) = 143.78

TOTAL RAINFALL (mm) = 71.24
RUNOFF COEFFICIENT = 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, STANDHYD (0110), Area (ha), Total Imp (%), ID=1 DT=5.0 min, Dir. Conn. (%).

Surface Area (ha) = 106.19
Dep. Storage (mm) = 6.31
Average Slope (%) = 1.00
Mannings n = 0.013
IMPERVIOUS (%) = 106.19
PERVIOUS (i) (%) = 180.81

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

Max. Eff. Inten. (mm/hr) = 148.15
over (min) = 10.00
Storage Coeff. (min) = 10.56 (ii)
Unit Hyd. Tpeak (min) = 10.00
Unit Hyd. peak (cms) = 0.11
PEAK FLOW (cms) = 22.06
TIME TO PEAK (hrs) = 1.42
RUNOFF VOLUME (mm) = 64.93
TOTAL RAINFALL (mm) = 71.24
RUNOFF COEFFICIENT = 0.91

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

FINISH

Length (m) = 1101.51
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

Max. Eff. Inten. (mm/hr) = 148.15
over (min) = 10.00
Storage Coeff. (min) = 10.25 (ii)
Unit Hyd. Tpeak (min) = 10.00
Unit Hyd. peak (cms) = 0.11

PEAK FLOW (cms) = 8.49
TIME TO PEAK (hrs) = 1.42
RUNOFF VOLUME (mm) = 64.97
TOTAL RAINFALL (mm) = 71.24
RUNOFF COEFFICIENT = 0.91

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, STANDHYD (0109), Area (ha), Total Imp (%), ID=1 DT=5.0 min, Dir. Conn. (%).

Surface Area (ha) = 2.26
Dep. Storage (mm) = 6.05
Average Slope (%) = 1.00
Length (m) = 201.66
Mannings n = 0.013
IMPERVIOUS (%) = 2.26
PERVIOUS (i) (%) = 3.84

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

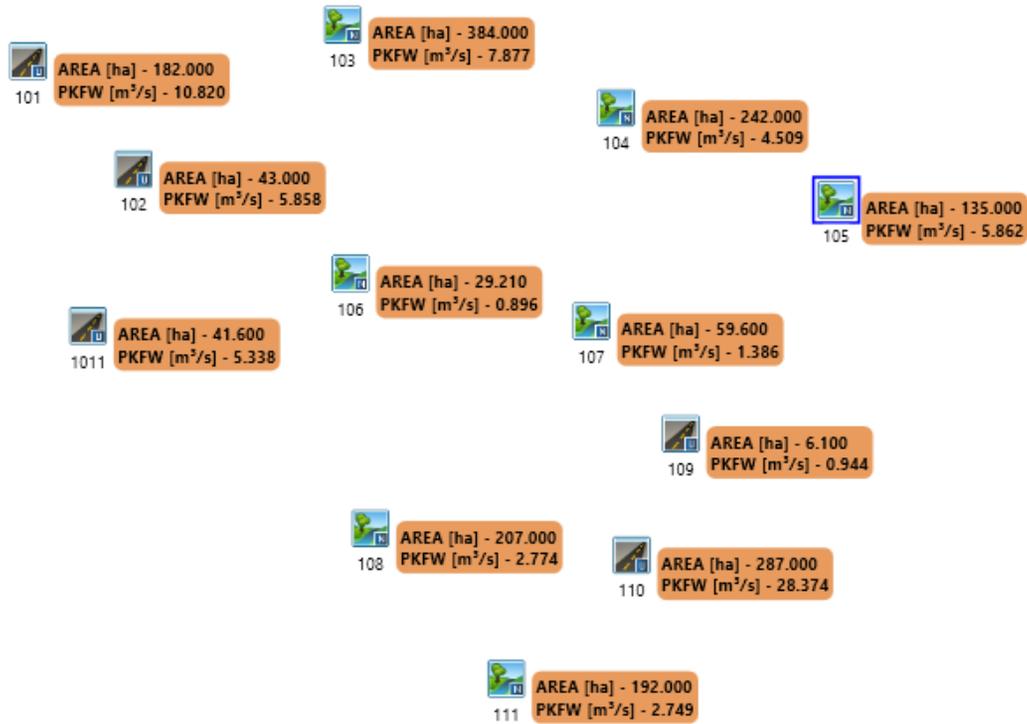
TRANSFORMED HYETOGRAPH table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

Max. Eff. Inten. (mm/hr) = 148.15
over (min) = 5.00
Storage Coeff. (min) = 3.33 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.26

PEAK FLOW (cms) = 0.72
TIME TO PEAK (hrs) = 1.33
RUNOFF VOLUME (mm) = 65.19

TOTALS
PEAK FLOW (cms) = 0.752 (iii)
TIME TO PEAK (hrs) = 2.00
RUNOFF VOLUME (mm) = 40.04

100-year 4 hour Chicago storm existing conditions



V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U AAAA L
V V I SS U U A A L
W I SSSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M 000
O O T T H H Y Y M M 000
000 T T H H Y Y M M 000

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHMO 6.1\VO2\voindat
Output filename: C:\Users\caef070146\AppData\Local\civica\vh5\50564c49-8c07-4086-8157-8729fb3caba3\c1f105e
Summary filename: C:\Users\caef070146\AppData\Local\civica\vh5\50564c49-8c07-4086-8157-8729fb3caba3\c1f105e

DATE: 09-14-2021 TIME: 08:43:00
USER:

COMMENTS:

***** SIMULATION : Run 06 *****

READ STORM File: C:\Users\caef070146\AppData\Local\Temp\ba1c97d1-3fce-403b-892b-c79a1f826375\4c83eb39
Ptotal= 87.58 mm Comments: 100-year, 4 hr-chicago, City of Barrie

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	6.41	1.17	45.22	2.17	14.50	3.17	7.56
0.33	7.29	1.33	180.15	2.33	12.44	3.33	7.04
0.50	8.52	1.50	58.54	2.50	10.94	3.50	6.60
0.67	10.36	1.67	31.96	2.67	9.80	3.67	6.22
0.83	13.45	1.83	22.45	2.83	8.90	3.83	5.89
1.00	19.96	2.00	17.52	3.00	8.16	4.00	5.59

CALIB NASHYD (0105) Area (ha)= 135.00 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.35

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Unit Hyd Qpeak (cms)= 14.732

ID= 1 DT= 5.0 min | Ia (mm)= 7.05 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 1.86

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Unit Hyd Qpeak (cms)= 4.251

PEAK FLOW (cms)= 2.774 (i)
TIME TO PEAK (hrs)= 3.917
RUNOFF VOLUME (mm)= 20.155
TOTAL RAINFALL (mm)= 87.578
RUNOFF COEFFICIENT = 0.230

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0111) Area (ha)= 192.00 Curve Number (CN)= 63.3
ID= 1 DT= 5.0 min Ia (mm)= 7.15 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 2.72

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Unit Hyd Qpeak (cms)= 2.696

PEAK FLOW (cms)= 2.749 (i)
TIME TO PEAK (hrs)= 4.750
RUNOFF VOLUME (mm)= 28.394
TOTAL RAINFALL (mm)= 87.578
RUNOFF COEFFICIENT = 0.324

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (1011) Area (ha)= 41.60 Dir. Conn. (%) = 30.00
ID= 1 DT= 5.0 min Total Imp (%) = 35.00

Surface Area (ha)= 14.56 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 6.27 27.04 3.67
Average Slope (%)= 0.70 2.00
Length (m)= 526.52 1082.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

PEAK FLOW (cms)= 5.862 (i)
TIME TO PEAK (hrs)= 1.750
RUNOFF VOLUME (mm)= 21.925
TOTAL RAINFALL (mm)= 87.578
RUNOFF COEFFICIENT = 0.250

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0106) Area (ha)= 29.21 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.61

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Unit Hyd Qpeak (cms)= 1.829

PEAK FLOW (cms)= 0.896 (i)
TIME TO PEAK (hrs)= 2.083
RUNOFF VOLUME (mm)= 21.929
TOTAL RAINFALL (mm)= 87.578
RUNOFF COEFFICIENT = 0.250

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0107) Area (ha)= 59.60 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.93

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Unit Hyd Qpeak (cms)= 2.448

PEAK FLOW (cms)= 1.386 (i)
TIME TO PEAK (hrs)= 2.083
RUNOFF VOLUME (mm)= 21.929
TOTAL RAINFALL (mm)= 87.578
RUNOFF COEFFICIENT = 0.250

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0108) Area (ha)= 207.00 Curve Number (CN)= 51.3

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 23.66
over (min)= 5.00 100.00
Storage Coeff. (min)= 6.09 (ii) 96.94 (iii)
Unit Hyd. Tpeak (min)= 5.00 100.00
Unit Hyd. peak (cms)= 0.19 0.01

PEAK FLOW (cms)= 5.30 0.73 5.338 (iii)
TIME TO PEAK (hrs)= 3.33 3.08 1.33
RUNOFF VOLUME (mm)= 81.31 32.30 47.00
TOTAL RAINFALL (mm)= 87.58 87.58 87.58
RUNOFF COEFFICIENT = 0.93 0.37 0.54

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN= 66.2 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (0102) Area (ha)= 43.00
ID= 1 DT= 5.0 min Total Imp (%) = 39.00 Dir. Conn. (%) = 31.00

Surface Area (ha)= 16.77 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 6.27 3.49
Average Slope (%)= 1.00 0.87
Length (m)= 535.41 1000.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 27.69
over (min)= 5.00 110.00
Storage Coeff. (min)= 5.52 (ii) 109.98 (iii)
Unit Hyd. Tpeak (min)= 5.00 110.00
Unit Hyd. peak (cms)= 0.20 0.01

PEAK FLOW (cms)= 5.83 0.76 5.858 (iii)
TIME TO PEAK (hrs)= 3.33 3.25 1.33
RUNOFF VOLUME (mm)= 81.31 35.83 49.93
TOTAL RAINFALL (mm)= 87.58 87.58 87.58
RUNOFF COEFFICIENT = 0.93 0.41 0.57

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0103) ID= 1 DT= 5.0 min	Area (ha)= 384.00 Ia (mm)= 5.50 U.H. Tp(hrs)= 1.87	Curve Number (CN)= 65.0 # of Linear Res. (N)= 3.00
---	--	---

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Unit Hyd Qpeak (cms)= 7.843

PEAK FLOW (cms)= 7.877 (i)
 TIME TO PEAK (hrs)= 3.833
 RUNOFF VOLUME (mm)= 30.783
 TOTAL RAINFALL (mm)= 87.578
 RUNOFF COEFFICIENT = 0.351

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0104) ID= 1 DT= 5.0 min	Area (ha)= 242.00 Ia (mm)= 5.90 U.H. Tp(hrs)= 1.66	Curve Number (CN)= 59.0 # of Linear Res. (N)= 3.00
---	--	---

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Unit Hyd Qpeak (cms)= 5.568

PEAK FLOW (cms)= 4.509 (i)
 TIME TO PEAK (hrs)= 3.583
 RUNOFF VOLUME (mm)= 25.839
 TOTAL RAINFALL (mm)= 87.578
 RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0101) ID= 1 DT= 5.0 min	Area (ha)= 182.00 Total Imp(%)= 21.00	Dir. Conn.(%)= 18.00
Surface Area (ha)= 38.22	IMPERVIOUS (ha)= 143.78	PERVIOUS (i) (ha)= 180.81
Dep. Storage (mm)= 6.27	(mm)= 6.50	
Average Slope (%)= 0.70		

TOTAL RAINFALL (mm)= 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.93 0.47 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0110) ID= 1 DT= 5.0 min	Area (ha)= 287.00 Total Imp(%)= 37.00	Dir. Conn.(%)= 30.00
Surface Area (ha)= 106.19	IMPERVIOUS (ha)= 180.81	PERVIOUS (i) (ha)= 180.81
Dep. Storage (mm)= 6.31	(mm)= 3.90	
Average Slope (%)= 1.00	(%)= 2.00	
Length (m)= 1383.23	(m)= 1103.00	
Mannings n = 0.013	= 0.250	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 31.34
 over (min)= 10.00 95.00
 Storage Coeff. (min)= 9.76 (ii) 91.90 (ii)
 Unit Hyd. Tpeak (min)= 10.00 95.00
 Unit Hyd. peak (cms)= 0.11 0.01

PEAK FLOW (cms)= 27.80 6.70 *TOTALS*
 TIME TO PEAK (hrs)= 1.42 3.00 28.374 (iii)
 RUNOFF VOLUME (mm)= 81.27 40.86 52.98
 TOTAL RAINFALL (mm)= 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.93 0.47 0.60

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

FINISH

Length (m)= 1101.51 1082.00
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 16.46
 over (min)= 10.00 115.00
 Storage Coeff. (min)= 9.48 (ii) 114.54 (ii)
 Unit Hyd. Tpeak (min)= 10.00 115.00
 Unit Hyd. peak (cms)= 0.12 0.01

PEAK FLOW (cms)= 10.69 2.39 *TOTALS*
 TIME TO PEAK (hrs)= 1.42 3.50 10.820 (iii)
 RUNOFF VOLUME (mm)= 81.31 23.49 33.89
 TOTAL RAINFALL (mm)= 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.93 0.27 0.39

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0109) ID= 1 DT= 5.0 min	Area (ha)= 6.10 Total Imp(%)= 37.00	Dir. Conn.(%)= 30.00
Surface Area (ha)= 2.26	IMPERVIOUS (ha)= 3.84	PERVIOUS (i) (ha)= 3.84
Dep. Storage (mm)= 6.05	(mm)= 3.92	
Average Slope (%)= 1.00	(%)= 2.40	
Length (m)= 201.66	(m)= 296.00	
Mannings n = 0.013	= 0.250	

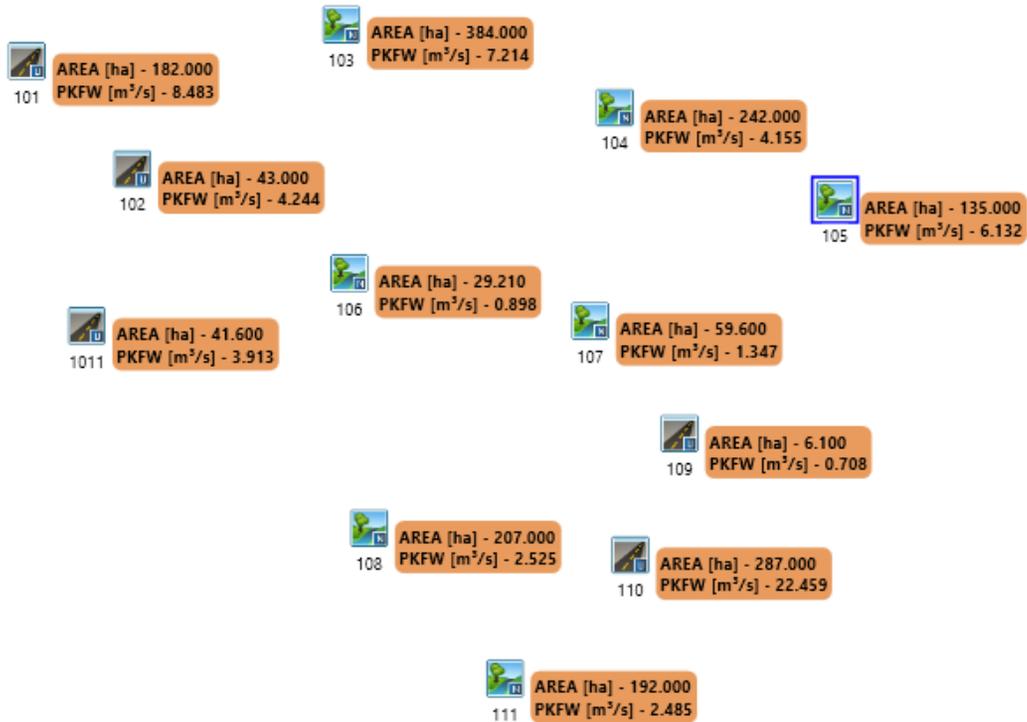
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 49.08
 over (min)= 5.00 35.00
 Storage Coeff. (min)= 3.08 (ii) 32.59 (ii)
 Unit Hyd. Tpeak (min)= 5.00 35.00
 Unit Hyd. peak (cms)= 0.27 0.03

PEAK FLOW (cms)= 0.89 0.29 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.83 0.944 (iii)
 RUNOFF VOLUME (mm)= 81.53 40.85 53.05

25 hour 12 hour SCS Type II Existing Conditions



V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\VisualOTHYMO 6.1\VO2\vojn.dat
Output filename: C:\Users\caef070146\AppData\Local\Civica\H5\30564c49-8c07-4086-8157-8729fb3caba3\804f9f4d
Summary filename: C:\Users\caef070146\AppData\Local\Civica\H5\30564c49-8c07-4086-8157-8729fb3caba3\804f9f4d

DATE: 09-14-2021 TIME: 08:44:16

USER:

COMMENTS:

***** SIMULATION Run 16 *****

READ STORM Filename: C:\Users\caef070146\AppData\Local\Temp\74df3ab6-df94-43e0-ab28-f5e8d61d5ee\2d33b8b1
Comments: 25-Vr,12-hr SCS, City of Barrie

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.27	3.25	3.63	6.25	16.33	9.25	3.17
0.50	2.27	3.50	3.63	6.50	16.33	9.50	3.17
0.75	2.27	3.75	3.63	6.75	7.26	9.75	3.17
1.00	2.27	4.00	3.63	7.00	7.26	10.00	3.17
1.25	2.27	4.25	5.44	7.25	5.44	10.25	1.81
1.50	2.27	4.50	5.44	7.50	5.44	10.50	1.81
1.75	2.27	4.75	7.26	7.75	5.44	10.75	1.81
2.00	2.27	5.00	7.26	8.00	5.44	11.00	1.81
2.25	2.27	5.25	10.88	8.25	3.17	11.25	1.81
2.50	2.27	5.50	10.88	8.50	3.17	11.50	1.81
2.75	2.27	5.75	43.54	8.75	3.17	11.75	1.81
3.00	2.27	6.00	119.72	9.00	3.17	12.00	1.81

CALIB NASHYD (0105) Area (ha)= 135.00 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.35

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17

2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Unit Hyd Qpeak (cms) = 1.829

PEAK FLOW (cms) = 0.898 (i)
TIME TO PEAK (hrs) = 6.583
RUNOFF VOLUME (mm) = 23.399
TOTAL RAINFALL (mm) = 90.700
RUNOFF COEFFICIENT = 0.258

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0107) Area (ha)= 59.60 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.93

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	5.44	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Unit Hyd Qpeak (cms) = 2.448

PEAK FLOW (cms) = 1.347 (i)
TIME TO PEAK (hrs) = 6.917
RUNOFF VOLUME (mm) = 23.399
TOTAL RAINFALL (mm) = 90.700
RUNOFF COEFFICIENT = 0.258

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0108) Area (ha)= 207.00 Curve Number (CN)= 51.3
ID= 1 DT= 5.0 min Ia (mm)= 7.05 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 1.86

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Unit Hyd Qpeak (cms) = 14.732

PEAK FLOW (cms) = 6.132 (i)
TIME TO PEAK (hrs) = 6.250
RUNOFF VOLUME (mm) = 23.394
TOTAL RAINFALL (mm) = 90.700
RUNOFF COEFFICIENT = 0.258

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0106) Area (ha)= 29.21 Curve Number (CN)= 53.4
ID= 1 DT= 5.0 min Ia (mm)= 6.01 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.61

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	5.44	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81

1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Unit Hyd Qpeak (cms) = 2.696

PEAK FLOW (cms) = 2.485 (i)
 TIME TO PEAK (hrs) = 9.167
 RUNOFF VOLUME (mm) = 30.227
 TOTAL RAINFALL (mm) = 90.700
 RUNOFF COEFFICIENT = 0.333

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha) = 41.60
STANDHYD (0101)	Total Imp (%) = 35.00 Dir. Conn. (%) = 30.00
ID= 1 DT= 5.0 min	

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	= 14.56	27.04	
Dep. Storage (mm)	= 6.27	3.67	
Average Slope (%)	= 0.70	2.00	
Length (m)	= 526.62	1082.00	
Mannings n	= 0.013	0.250	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr) = 119.72 over (min) = 5.00

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.2 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha) = 384.00	Curve Number (CN) = 65.0
NASHYD (0103)	Ia (mm) = 5.30	# of LInear Res. (N) = 3.00
ID= 1 DT= 5.0 min	U. H. Tp (hrs) = 1.87	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Unit Hyd Qpeak (cms) = 7.843

PEAK FLOW (cms) = 7.214 (i)
 TIME TO PEAK (hrs) = 8.083
 RUNOFF VOLUME (mm) = 32.703
 TOTAL RAINFALL (mm) = 90.700
 RUNOFF COEFFICIENT = 0.361

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha) = 242.00	Curve Number (CN) = 59.0
NASHYD (0104)	Ia (mm) = 5.90	# of LInear Res. (N) = 3.00
ID= 1 DT= 5.0 min	U. H. Tp (hrs) = 1.66	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17

Storage Coeff. (min) = 7.17 (ii) 100.59 (ii)
 Unit Hyd. Tpeak (min) = 5.00 105.00
 Unit Hyd. peak (cms) = 0.17 0.01

PEAK FLOW (cms) = 3.81 0.68
 TIME TO PEAK (hrs) = 6.00 7.67
 RUNOFF VOLUME (mm) = 84.43 34.25 49.31
 TOTAL RAINFALL (mm) = 90.70 90.70 90.70
 RUNOFF COEFFICIENT = 0.93 0.38 0.54

TOTALS
 3.913 (iii)
 6.00

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 63.6 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha) = 43.00
STANDHYD (0102)	Total Imp (%) = 39.00 Dir. Conn. (%) = 31.00
ID= 1 DT= 5.0 min	

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	= 16.77	26.23	
Dep. Storage (mm)	= 6.27	3.49	
Average Slope (%)	= 1.00	0.87	
Length (m)	= 535.41	1000.00	
Mannings n	= 0.013	0.250	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44		

1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72
over (min) = 10.00
Storage Coeff. (min)= 11.16 (ii)
Unit Hyd. Tpeak (min)= 10.00
Unit Hyd. peak (cms)= 0.10

PEAK FLOW (cms)= 8.23
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 84.43
TOTAL RAINFALL (mm)= 90.70
RUNOFF COEFFICIENT = 0.93

TOTALS
8.483 (iii)
6.00
35.74
90.70
0.39

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 55.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (0109)	Area (ha)= 6.10	Dir. Conn.(%)= 30.00
ID= 1 DT= 5.0 min		Total Imp(%)= 37.00	

Surface Area (ha)= 1.26
Dep. Storage (mm)= 6.05
Average Slope (%)= 1.00
Length (m)= 201.66
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81

Max. Eff. Inten. (mm/hr)= 119.72
over (min) = 10.00
Storage Coeff. (min)= 11.50 (ii)
Unit Hyd. Tpeak (min)= 10.00
Unit Hyd. peak (cms)= 0.10

PEAK FLOW (cms)= 21.38
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 84.39
TOTAL RAINFALL (mm)= 90.70
RUNOFF COEFFICIENT = 0.93

TOTALS
6.21
7.58
55.54
90.70
0.61

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

FINISH

2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72
over (min) = 5.00
Storage Coeff. (min)= 8.62 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.25

PEAK FLOW (cms)= 0.60
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 84.65
TOTAL RAINFALL (mm)= 90.70
RUNOFF COEFFICIENT = 0.93

TOTALS
0.27
6.50
43.16
90.70
0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

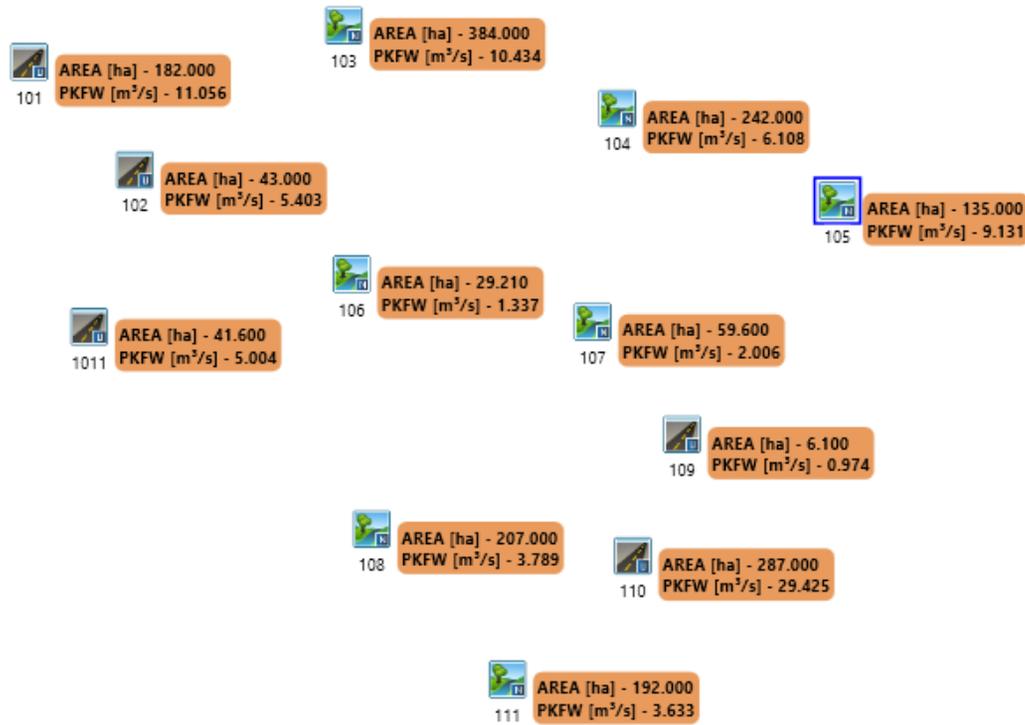
CALIB	STANDHYD (0110)	Area (ha)= 287.00	Dir. Conn.(%)= 30.00
ID= 1 DT= 5.0 min		Total Imp(%)= 37.00	

Surface Area (ha)= 106.19
Dep. Storage (mm)= 3.31
Average Slope (%)= 1.00
Length (m)= 1383.23
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

100-year 12 hour SCS Type II Existing Conditions



1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	13.50	8.583	3.94	11.58	2.25
2.667	3.37	5.667	13.50	8.667	3.94	11.67	2.25
2.750	3.37	5.750	13.50	8.750	3.94	11.75	2.25
2.833	3.37	5.833	13.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	13.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	13.50	9.000	3.94	12.00	2.25

Unit Hyd Qpeak (cms) = 2.696

PEAK FLOW (cms) = 3.633 (i)
 TIME TO PEAK (hrs) = 9.083
 RUNOFF VOLUME (mm) = 43.913
 TOTAL RAINFALL (mm) = 112.500
 RUNOFF COEFFICIENT = 0.390

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (0101)
 ID= 1 DT= 5.0 min

Area (ha) = 41.60
 Total Imp (%) = 35.00 Dir. Conn. (%) = 30.00

Surface Area (ha)	(ha) = 14.56	IMPERVIOUS (%)	(%) = 27.00	PERVIOUS (i)	(i) = 0.083
Dep. Storage (mm)	(mm) = 6.27				
Average Slope (%)	(%) = 0.70				
Length (m)	(m) = 525.62				
Mannings n	= 0.013				

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	20.25	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	4.50	7.083	6.75	10.08	2.25
1.167	2.81	4.167	4.50	7.167	6.75	10.17	2.25
1.250	2.81	4.250	4.50	7.250	6.75	10.25	2.25
1.333	2.81	4.333	4.50	7.333	6.75	10.33	2.25
1.417	2.81	4.417	4.50	7.417	6.75	10.42	2.25
1.500	2.81	4.500	4.50	7.500	6.75	10.50	2.25
1.583	2.81	4.583	4.50	7.583	6.75	10.58	2.25
1.667	2.81	4.667	4.50	7.667	6.75	10.67	2.25
1.750	2.81	4.750	4.50	7.750	6.75	10.75	2.25
1.833	2.81	4.833	4.50	7.833	6.75	10.83	2.25
1.917	2.81	4.917	4.50	7.917	6.75	10.92	2.25
2.000	2.81	5.000	4.50	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	13.50	8.583	3.94	11.58	2.25
2.667	3.37	5.667	13.50	8.667	3.94	11.67	2.25
2.750	3.37	5.750	13.50	8.750	3.94	11.75	2.25
2.833	3.37	5.833	13.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	13.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	13.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00

31.54 90.00

Storage Coeff. (min) = 6.58 (ii) 87.57 (iii)
 Unit Hyd. Tpeak (min) = 5.00 90.00
 Unit Hyd. peak (cms) = 0.18 0.01

PEAK FLOW (cms) = 4.80 1.08 *TOTALS* 5.004 (iii)
 TIME TO PEAK (hrs) = 6.00 7.42 6.00
 RUNOFF VOLUME (mm) = 106.23 48.72 65.97
 TOTAL RAINFALL (mm) = 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.94 0.43 0.59

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 63.6 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (0102)
 ID= 1 DT= 5.0 min

Area (ha) = 43.00
 Total Imp (%) = 39.00 Dir. Conn. (%) = 31.00

Surface Area (ha)	(ha) = 16.77	IMPERVIOUS (%)	(%) = 26.23	PERVIOUS (i)	(i) = 0.083
Dep. Storage (mm)	(mm) = 6.27				
Average Slope (%)	(%) = 1.00				
Length (m)	(m) = 535.41				
Mannings n	= 0.013				

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	4.50	7.083	6.75	10.08	2.25
1.167	2.81	4.167	4.50	7.167	6.75	10.17	2.25
1.250	2.81	4.250	4.50	7.250	6.75	10.25	2.25
1.333	2.81	4.333	4.50	7.333	6.75	10.33	2.25
1.417	2.81	4.417	4.50	7.417	6.75	10.42	2.25
1.500	2.81	4.500	4.50	7.500	6.75	10.50	2.25
1.583	2.81	4.583	4.50	7.583	6.75	10.58	2.25
1.667	2.81	4.667	4.50	7.667	6.75	10.67	2.25
1.750	2.81	4.750	4.50	7.750	6.75	10.75	2.25
1.833	2.81	4.833	4.50	7.833	6.75	10.83	2.25
1.917	2.81	4.917	4.50	7.917	6.75	10.92	2.25
2.000	2.81	5.000	4.50	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	13.50	8.583	3.94	11.58	2.25
2.667	3.37	5.667	13.50	8.667	3.94	11.67	2.25
2.750	3.37	5.750	13.50	8.750	3.94	11.75	2.25
2.833	3.37	5.833	13.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	13.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	13.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
 Storage Coeff. (min) = 5.97 (ii) 99.56 (iii)
 Unit Hyd. Tpeak (min) = 5.00 100.00
 Unit Hyd. peak (cms) = 0.19 0.01

PEAK FLOW (cms) = 5.21 1.11 *TOTALS* 5.403 (iii)
 TIME TO PEAK (hrs) = 6.00 7.58 6.00
 RUNOFF VOLUME (mm) = 106.23 53.40 69.78
 TOTAL RAINFALL (mm) = 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.94 0.47 0.62

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 66.2 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0103)
 ID= 1 DT= 5.0 min

Area (ha) = 384.00 Curve Number (CN) = 65.0
 Ia (mm) = 5.90 # of Llinear Res. (N) = 3.00
 U.H. Tp (hrs) = 1.87

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	4.50	7.083	6.75	10.08	2.25
1.167	2.81	4.167	4.50	7.167	6.75	10.17	2.25
1.250	2.81	4.250	4.50	7.250	6.75	10.25	2.25
1.333	2.81	4.333	4.50	7.333	6.75	10.33	2.25
1.417	2.81	4.417	4.50	7.417	6.75	10.42	2.25
1.500	2.81	4.500	4.50	7.500	6.75	10.50	2.25
1.583	2.81	4.583	4.50	7.583	6.75	10.58	2.25
1.667	2.81	4.667	4.50	7.667	6.75	10.67	2.25
1.750	2.81	4.750	4.50	7.750	6.75	10.75	2.25
1.833	2.81	4.833	4.50	7.833	6.75	10.83	2.25
1.917	2.81	4.917	4.50	7.917	6.75	10.92	2.25
2.000	2.81	5.000	4.50	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50				

1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50
over (min) = 10.00
Storage Coeff. (min)= 10.24 (ii)
Unit Hyd. Tpeak (min)= 10.00
Unit Hyd. peak (cms)= 0.11

PEAK FLOW (cms)= 10.53
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 106.23
TOTAL RAINFALL (mm)= 112.50
RUNOFF COEFFICIENT = 0.94

TOTALS
11.056 (iii)
6.00
49.32
112.50
0.44

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 55.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (0109)	Area (ha)= 6.10	Dir. Conn.(%)= 30.00
ID= 1 DT= 5.0 min		Total Imp(%)= 37.00	

Surface Area (ha)=	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)=	2.26	3.84
Average Slope (%)=	6.05	3.92
Length (m)=	1.00	2.40
Mannings n =	201.66	296.00
	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25

Max. Eff. Inten. (mm/hr)= 148.50
over (min) = 10.00
Storage Coeff. (min)= 10.55 (ii)
Unit Hyd. Tpeak (min)= 10.00
Unit Hyd. peak (cms)= 0.11

PEAK FLOW (cms)= 27.38
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 106.19
TOTAL RAINFALL (mm)= 112.50
RUNOFF COEFFICIENT = 0.94

TOTALS
29.425 (iii)
6.00
73.88
112.50
0.66

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

FINISH

2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50
over (min) = 5.00
Storage Coeff. (min)= 3.32 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.26

PEAK FLOW (cms)= 0.75
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 106.45
TOTAL RAINFALL (mm)= 112.50
RUNOFF COEFFICIENT = 0.95

TOTALS
0.974 (iii)
6.00
73.94
112.50
0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (0110)	Area (ha)= 287.00	Dir. Conn.(%)= 30.00
ID= 1 DT= 5.0 min		Total Imp(%)= 37.00	

Surface Area (ha)=	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)=	106.19	180.81
Average Slope (%)=	3.31	3.90
Length (m)=	1.00	2.00
Mannings n =	1383.23	1103.00
	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

100-Yr Pre-Developmet Roadway Drainage Area Flows (12-hr SCSII)

10011 1001L AREA [ha] - 0.210 PKFW [m ³ /s] - 0.083	10021 1002L AREA [ha] - 0.730 PKFW [m ³ /s] - 0.282	10031 1003L AREA [ha] - 0.380 PKFW [m ³ /s] - 0.142	10041 1004L AREA [ha] - 0.350 PKFW [m ³ /s] - 0.135
10012 1001R AREA [ha] - 0.230 PKFW [m ³ /s] - 0.091	10022 1002R AREA [ha] - 0.830 PKFW [m ³ /s] - 0.324	10032 1003R AREA [ha] - 0.390 PKFW [m ³ /s] - 0.154	10042 1004R AREA [ha] - 0.380 PKFW [m ³ /s] - 0.148
10051 1005L AREA [ha] - 0.370 PKFW [m ³ /s] - 0.138	10061 1006L AREA [ha] - 0.650 PKFW [m ³ /s] - 0.241	10071 1007L AREA [ha] - 0.720 PKFW [m ³ /s] - 0.263	10081 1008L AREA [ha] - 0.210 PKFW [m ³ /s] - 0.075
10052 1005R AREA [ha] - 0.360 PKFW [m ³ /s] - 0.133	10062 1006R AREA [ha] - 0.660 PKFW [m ³ /s] - 0.243	10072 1007R AREA [ha] - 0.740 PKFW [m ³ /s] - 0.285	10082 1008R AREA [ha] - 0.210 PKFW [m ³ /s] - 0.085
10091 1009L AREA [ha] - 0.500 PKFW [m ³ /s] - 0.195	1101 1100L AREA [ha] - 0.910 PKFW [m ³ /s] - 0.326	1111 1110L AREA [ha] - 1.060 PKFW [m ³ /s] - 0.385	1121 1120L AREA [ha] - 1.210 PKFW [m ³ /s] - 0.461
10092 1009R AREA [ha] - 0.630 PKFW [m ³ /s] - 0.250	1102 1100R AREA [ha] - 0.880 PKFW [m ³ /s] - 0.340	1112 1110R AREA [ha] - 0.970 PKFW [m ³ /s] - 0.382	1122 1120R AREA [ha] - 1.220 PKFW [m ³ /s] - 0.466

V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U A A L
W I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M O O
Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\visual OTTHMO 6.1\VO2\voindat
Output filename: C:\Users\caef070146\AppData\Local\Civica\WHS\50564c49-8c07-4086-8157-8729fb3caba3\0f390bea
Summary filename: C:\Users\caef070146\AppData\Local\Civica\WHS\50564c49-8c07-4086-8157-8729fb3caba3\0f390bea

DATE: 05-18-2022 TIME: 12:41:42

USER:

COMMENTS:

** SIMULATION Run 13 *****

READ STORM Filename: C:\Users\caef070146\AppData\Local\Temp\
b414f864-f56d-471c-a2fc-7a8c6fa5585\39b42806
Ptotal= 46.70 mm
Comments: 2-vr, 12-hr SCS, City of Barrie

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows rainfall intensity and cumulative totals over time.

CALIB STANDBYD (10011) Area (ha)= 0.21
Total Imp(%)= 79.00 Dir. Conn.(%)= 79.00

Surface Area (ha)= 0.17 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 37.42 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall data.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows detailed rainfall data for the first event.

Max. Eff. Inten. (mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.60 (ii) 13.57 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.08

PEAK FLOW (cms)= 0.11 0.02
TIME TO PEAK (hrs)= 6.00 6.08
RUNOFF VOLUME (mm)= 41.04 46.70
TOTAL RAINFALL (mm)= 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10021) Area (ha)= 0.73
Total Imp(%)= 71.00 Dir. Conn.(%)= 71.00

Surface Area (ha)= 0.52 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 60.76 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall data for the second event.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows detailed rainfall data for the first event.

Max. Eff. Inten. (mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff. (min)= 1.72 (ii) 15.00 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.08

PEAK FLOW (cms)= 0.03 0.00
TIME TO PEAK (hrs)= 6.00 6.08
RUNOFF VOLUME (mm)= 45.70 27.09
TOTAL RAINFALL (mm)= 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10022) Area (ha)= 0.83
Total Imp(%)= 75.00 Dir. Conn.(%)= 75.00

Surface Area (ha)= 0.62 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 74.39 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall data.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows detailed rainfall data for the first event.

Max. Eff. Inten. (mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.50 (ii) 15.00 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.08

PEAK FLOW (cms)= 0.09 0.02
TIME TO PEAK (hrs)= 6.00 6.08
RUNOFF VOLUME (mm)= 45.70 27.09
TOTAL RAINFALL (mm)= 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10031) Area (ha)= 0.38
Total Imp(%)= 66.00 Dir. Conn.(%)= 66.00

Surface Area (ha)= 0.25 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall data.

2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max. Eff. Inten. (mm/hr)=	61.64	41.44	
over (min)	5.00	15.00	
Storage Coeff. (min)=	2.05 (ii)	13.03 (ii)	
Unit Hyd. Tpeak (min)=	5.00	15.00	
Unit Hyd. peak (cms)=	0.31	0.08	
TOTALS			
PEAK FLOW (cms)=	0.04	0.01	0.052 (iii)
TIME TO PEAK (hrs)=	6.00	6.08	6.00
RUNOFF VOLUME (mm)=	45.70	27.09	39.91
TOTAL RAINFALL (mm)=	46.70	46.70	46.70
RUNOFF COEFFICIENT =	0.98	0.58	0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN^o = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (10032)	Area (ha)= 0.39	Total Imp (%)= 79.00	Dir. Conn. (%)= 79.00
ID= 1	DT= 5.0 min			

Surface Area (ha)=	IMPERVIOUS 0.31	PERVIOUS (i) 0.08
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	50.99	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.17	5.083	4.00	8.083	1.63	11.08	0.93
2.167	1.17	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.17	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.17	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.17	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.17	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.17	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.17	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.17	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.17	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.17	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.17	6.000	61.64	9.000	1.63	12.00	0.93

Max. Eff. Inten. (mm/hr)=	61.64	41.44	
over (min)	5.00	15.00	
Storage Coeff. (min)=	2.05 (ii)	13.03 (ii)	
Unit Hyd. Tpeak (min)=	5.00	15.00	
Unit Hyd. peak (cms)=	0.31	0.08	
TOTALS			
PEAK FLOW (cms)=	0.05	0.01	0.049 (iii)
TIME TO PEAK (hrs)=	6.00	6.08	6.00
RUNOFF VOLUME (mm)=	45.70	27.09	39.91
TOTAL RAINFALL (mm)=	46.70	46.70	46.70
RUNOFF COEFFICIENT =	0.98	0.58	0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN^o = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (10042)	Area (ha)= 0.38	Total Imp (%)= 74.00	Dir. Conn. (%)= 74.00
ID= 1	DT= 5.0 min			

Surface Area (ha)=	IMPERVIOUS 0.28	PERVIOUS (i) 0.10
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	50.33	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.17	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.17	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.17	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.17	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.17	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.17	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.17	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.17	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.17	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.17	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.17	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.17	6.000	61.64	9.000	1.63	12.00	0.93

Max. Eff. Inten. (mm/hr)=	61.64	41.44	
over (min)	5.00	15.00	
Storage Coeff. (min)=	2.05 (ii)	13.03 (ii)	
Unit Hyd. Tpeak (min)=	5.00	15.00	
Unit Hyd. peak (cms)=	0.31	0.08	
TOTALS			
PEAK FLOW (cms)=	0.05	0.01	0.055 (iii)
TIME TO PEAK (hrs)=	6.00	6.08	6.00
RUNOFF VOLUME (mm)=	45.70	27.09	40.84
TOTAL RAINFALL (mm)=	46.70	46.70	46.70
RUNOFF COEFFICIENT =	0.98	0.58	0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93
-------	------	-------	-------	-------	------	-------	------

Max. Eff. Inten. (mm/hr)=	61.64	41.44	
over (min)	5.00	15.00	
Storage Coeff. (min)=	2.07 (ii)	13.04 (ii)	
Unit Hyd. Tpeak (min)=	5.00	15.00	
Unit Hyd. peak (cms)=	0.31	0.08	
TOTALS			
PEAK FLOW (cms)=	0.05	0.01	0.058 (iii)
TIME TO PEAK (hrs)=	6.00	6.08	6.00
RUNOFF VOLUME (mm)=	45.70	27.09	41.77
TOTAL RAINFALL (mm)=	46.70	46.70	46.70
RUNOFF COEFFICIENT =	0.98	0.58	0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN^o = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (10041)	Area (ha)= 0.35	Total Imp (%)= 69.00	Dir. Conn. (%)= 69.00
ID= 1	DT= 5.0 min			

Surface Area (ha)=	IMPERVIOUS 0.24	PERVIOUS (i) 0.11
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	48.30	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.8				

CALIB
STANDHYD (10051)
ID= 1 DT= 5.0 min

Area (ha)= 0.37
Total Imp (%) = 66.00 Dir. Conn.(%) = 66.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.24 0.13
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 2.00 2.00
Length (m) = 49.67 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	1.87	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	6.164	8.583	1.63	11.58	0.93
2.667	1.40	5.667	6.164	8.667	1.63	11.67	0.93
2.750	1.40	5.750	6.164	8.750	1.63	11.75	0.93
2.833	1.40	5.833	6.164	8.833	1.63	11.83	0.93
2.917	1.40	5.917	6.164	8.917	1.63	11.92	0.93
3.000	1.40	6.000	6.164	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff.(min)= 2.04 (ii) 13.01 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

PEAK FLOW (cms)= 0.04 0.01 0.050 (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 38.61
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10052)
ID= 1 DT= 5.0 min

Area (ha)= 0.36
Total Imp (%) = 62.00 Dir. Conn.(%) = 62.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.22 0.14
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 2.00 2.00

Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	6.164	8.583	1.63	11.58	0.93
2.667	1.40	5.667	6.164	8.667	1.63	11.67	0.93
2.750	1.40	5.750	6.164	8.750	1.63	11.75	0.93
2.833	1.40	5.833	6.164	8.833	1.63	11.83	0.93
2.917	1.40	5.917	6.164	8.917	1.63	11.92	0.93
3.000	1.40	6.000	6.164	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff.(min)= 2.02 (ii) 12.99 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

PEAK FLOW (cms)= 0.04 0.01 0.047 (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 38.61
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10062)
ID= 1 DT= 5.0 min

Area (ha)= 0.66
Total Imp (%) = 62.00 Dir. Conn.(%) = 62.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.41 0.25
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 2.00 2.00
Length (m) = 66.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	6.164	8.583	1.63	11.58	0.93
2.667	1.40	5.667	6.164	8.667	1.63	11.67	0.93
2.750	1.40	5.750	6.164	8.750	1.63	11.75	0.93
2.833	1.40	5.833	6.164	8.833	1.63	11.83	0.93
2.917	1.40	5.917	6.164	8.917	1.63	11.92	0.93
3.000	1.40	6.000	6.164	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff.(min)= 2.41 (ii) 13.39 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.30 0.08

PEAK FLOW (cms)= 0.07 0.02 0.087 (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 39.17
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10072)
ID= 1 DT= 5.0 min

Area (ha)= 0.74
Total Imp (%) = 71.00 Dir. Conn.(%) = 71.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.53 0.21
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 2.00 2.00
Length (m) = 70.24 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.3							

1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min) = 5.00 15.00
Storage Coeff. (min)= 2.51 (ii) 13.48 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.08

PEAK FLOW (cms)= 0.09 0.02 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.08 0.104 (iii)
RUNOFF VOLUME (mm)= 45.70 27.09 40.29
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10071)
ID= 1 DT= 5.0 min

Area (ha)= 0.72
Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00

Surface Area (ha)	0.43	0.29
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	69.28	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93

2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min) = 5.00 15.00
Storage Coeff. (min)= 2.49 (ii) 13.46 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.08

PEAK FLOW (cms)= 0.07 0.02 *TOTALS* (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 38.25
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10081)
ID= 1 DT= 5.0 min

Area (ha)= 0.21
Total Imp(%)= 52.00 Dir. Conn.(%)= 52.00

Surface Area (ha)	0.11	0.10
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	37.42	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93

Storage Coeff. (min)= 1.72 (ii) 6.10 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.03 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 43.27
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10092)
ID= 1 DT= 5.0 min

Area (ha)= 0.63
Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

Surface Area (ha)	0.51	0.12
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	64.81	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8			

RUNOFF COEFFICIENT = 0.98 0.58 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10091) Area (ha)= 0.50 Total Imp(%)= 74.00 Dir. Conn.(%)= 74.00

Surface Area (ha)= 0.37 IMPERVIOUS 0.13
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 57.74 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for various time intervals.

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.23 (ii) 13.20 (ii)
Unit Hyd. Tpeak (min)= 5.00 0.08
Unit Hyd. peak (cms)= 0.30 0.08

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

Surface Area (ha)= 0.63 IMPERVIOUS 0.25
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 76.59 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for various time intervals.

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.64 (ii) 13.62 (ii)
Unit Hyd. Tpeak (min)= 5.00 0.08
Unit Hyd. peak (cms)= 0.29 0.08

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1111) Area (ha)= 1.06 Total Imp(%)= 59.00 Dir. Conn.(%)= 59.00

Surface Area (ha)= 0.63 IMPERVIOUS 0.43
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 84.06 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1101) Area (ha)= 0.91 Total Imp(%)= 55.00 Dir. Conn.(%)= 55.00

Surface Area (ha)= 0.50 IMPERVIOUS 0.41
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 77.89 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for various time intervals.

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.67 (ii) 13.64 (ii)
Unit Hyd. Tpeak (min)= 5.00 0.08
Unit Hyd. peak (cms)= 0.29 0.08

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1102) Area (ha)= 0.88 Total Imp(%)= 72.00 Dir. Conn.(%)= 72.00

Surface Area (ha)= 0.50 IMPERVIOUS 0.25
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 76.59 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for various time intervals.

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.79 (ii) 13.77 (ii)
Unit Hyd. Tpeak (min)= 5.00 0.08
Unit Hyd. peak (cms)= 0.28 0.08

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1112) Area (ha)= 0.97 Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

Surface Area (ha)= 0.76 IMPERVIOUS 0.21
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 80.42 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for various time intervals.

0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.166	1.17	4.166	2.80	7.166	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
 over (min)= 5.00 15.00
 Storage Coeff.(min)= 2.72 (ii) 13.69 (iii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.29 0.08

PEAK FLOW (cms)= 0.13 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.08 0.165 (iii)
 RUNOFF VOLUME (mm)= 45.70 27.09 41.60
 TOTAL RAINFALL (mm)= 46.70 46.70 46.70
 RUNOFF COEFFICIENT = 0.98 0.58 0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (1122)	Area (ha)= 1.22	Dir. Conn.(%)= 69.00
ID= 1 DT= 5.0 min	Total Imp(%)= 69.00	

Surface Area (ha)=	IMPERVIOUS 0.84	PERVIOUS (i) 0.38
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	90.18	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63

1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
 over (min)= 5.00 15.00
 Storage Coeff.(min)= 2.91 (ii) 13.89 (iii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.28 0.08

PEAK FLOW (cms)= 0.14 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.08 0.168 (iii)
 RUNOFF VOLUME (mm)= 45.70 27.09 39.93
 TOTAL RAINFALL (mm)= 46.70 46.70 46.70
 RUNOFF COEFFICIENT = 0.98 0.58 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (1121)	Area (ha)= 1.21	Dir. Conn.(%)= 68.00
ID= 1 DT= 5.0 min	Total Imp(%)= 68.00	

Surface Area (ha)=	IMPERVIOUS 0.82	PERVIOUS (i) 0.39
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	89.81	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93

1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
 over (min)= 5.00 15.00
 Storage Coeff.(min)= 2.91 (ii) 13.89 (iii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.28 0.08

PEAK FLOW (cms)= 0.14 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.08 0.165 (iii)
 RUNOFF VOLUME (mm)= 45.70 27.09 39.74
 TOTAL RAINFALL (mm)= 46.70 46.70 46.70
 RUNOFF COEFFICIENT = 0.98 0.58 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10011)	Area (ha)= 0.21	Dir. Conn.(%)= 79.00
ID= 1 DT= 5.0 min	Total Imp(%)= 79.00	

Surface Area (ha)=	IMPERVIOUS 0.17	PERVIOUS (i) 0.04
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	37.42	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14		

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, STANDHYD (10022), ID= 1 DT= 5.0 min, Area (ha)= 0.83, Total Imp(%)= 75.00, Dir. Conn.(%)= 75.00

Table with columns: Surface Area (ha)= 0.62, Dep. Storage (mm)= 1.00, Average Slope (%)= 7.00, Length (m)= 74.39, Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

Summary statistics table: Max.Eff.Inten.(mm/hr)= 84.88, Storage Coeff.(min)= 2.28, Unit Hyd. Tpeak (min)= 5.00, PEAK FLOW (cms)= 0.15, TIME TO PEAK (hrs)= 6.00, RUNOFF VOLUME (mm)= 63.30, TOTAL RAINFALL (mm)= 64.30, RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: Surface Area (ha)= 0.25, Dep. Storage (mm)= 1.00, Average Slope (%)= 2.00, Length (m)= 50.33, Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

Summary statistics table: Max.Eff.Inten.(mm/hr)= 84.88, Storage Coeff.(min)= 1.81, Unit Hyd. Tpeak (min)= 5.00, PEAK FLOW (cms)= 0.06, TIME TO PEAK (hrs)= 6.00, RUNOFF VOLUME (mm)= 63.30, TOTAL RAINFALL (mm)= 64.30, RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, STANDHYD (10032), ID= 1 DT= 5.0 min, Area (ha)= 0.39, Total Imp(%)= 79.00, Dir. Conn.(%)= 79.00

Table with columns: Surface Area (ha)= 0.31, Dep. Storage (mm)= 1.00, Average Slope (%)= 1.00, Length (m)= 50.99, Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: CALIB, STANDHYD (10021), ID= 1 DT= 5.0 min, Area (ha)= 0.73, Total Imp(%)= 71.00, Dir. Conn.(%)= 71.00

Table with columns: Surface Area (ha)= 0.52, Dep. Storage (mm)= 1.00, Average Slope (%)= 2.00, Length (m)= 69.76, Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

Summary statistics table: Max.Eff.Inten.(mm/hr)= 84.88, Storage Coeff.(min)= 2.20, Unit Hyd. Tpeak (min)= 5.00, PEAK FLOW (cms)= 0.12, TIME TO PEAK (hrs)= 6.00, RUNOFF VOLUME (mm)= 64.30, TOTAL RAINFALL (mm)= 64.30, RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, STANDHYD (10031), ID= 1 DT= 5.0 min, Area (ha)= 0.38, Total Imp(%)= 66.00, Dir. Conn.(%)= 66.00

IMPERVIOUS PERVIOUS (i)

TRANSFORMED HYETOGRAPH table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

Summary statistics table: Max.Eff.Inten.(mm/hr)= 84.88, Storage Coeff.(min)= 1.82, Unit Hyd. Tpeak (min)= 5.00, PEAK FLOW (cms)= 0.07, TIME TO PEAK (hrs)= 6.00, RUNOFF VOLUME (mm)= 64.30, TOTAL RAINFALL (mm)= 64.30, RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, STANDHYD (10041), ID= 1 DT= 5.0 min, Area (ha)= 0.35, Total Imp(%)= 69.00, Dir. Conn.(%)= 69.00

Table with columns: Surface Area (ha)= 0.24, Dep. Storage (mm)= 1.00, Average Slope (%)= 2.00, Length (m)= 48.30, Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.666	1.61	3.666	2.57	6.666	5.14	9.67	2.25
0.750	1.61	3.750	2.57	7.500	5.14	9.75	2.25
0.833	1.61	3.833	2.57	8.333	5.14	9.83	2.25
0.917	1.61	3.917	2.57	9.17	5.14	9.92	2.25
1.000	1.61	4.000	2.57	10.00	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.666	1.61	4.666	5.14	7.666	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min) = 5.00 15.00
Storage Coeff. (min)= 1.76 (ii) 10.83 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.09

PEAK FLOW (cms)= 0.06 0.01 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.08 *0.069 (iii)
RUNOFF VOLUME (mm)= 63.30 42.31 56.78
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10042)
ID= 1 DT= 5.0 min

Area (ha)= 0.38
Total Imp(%)= 74.00 Dir. Conn.(%)= 74.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.28 0.10
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25

1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min) = 5.00 15.00
Storage Coeff. (min)= 1.81 (ii) 10.87 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.09

PEAK FLOW (cms)= 0.07 0.01 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.08 *0.078 (iii)
RUNOFF VOLUME (mm)= 63.30 42.31 57.82
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10012)
ID= 1 DT= 5.0 min

Area (ha)= 0.23
Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.18 0.05
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 39.16 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
1.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
1.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
1.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
1.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
1.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
1.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
1.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
1.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
1.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
1.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
2.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min) = 5.00 15.00
Storage Coeff. (min)= 1.79 (ii) 10.86 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.09

PEAK FLOW (cms)= 0.06 0.02 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 *0.072 (iii)
RUNOFF VOLUME (mm)= 63.30 42.31 56.15
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10052)
ID= 1 DT= 5.0 min

Area (ha)= 0.36
Total Imp(%)= 62.00 Dir. Conn.(%)= 62.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.22 0.14
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
1.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
1.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
1.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
1.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
1.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
1.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
1.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
1.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
1.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
1.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
2.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

CALIB
STANDHYD (10051)
ID= 1 DT= 5.0 min

Area (ha)= 0.37
Total Imp(%)= 66.00 Dir. Conn.(%)= 66.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.24 0.1

Max. Eff. Inten. (mm/hr) = 84.88 66.77
 over (min) = 5.00 15.00
 Storage Coeff. (min) = 1.78 (ii) 10.85 (iii)
 Unit Hyd. Tpeak (min) = 5.00 15.00
 Unit Hyd. peak (cms) = 0.32 0.09

TOTALS
 CN² = 89.0 Ia = Dep. Storage (Above)
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10062)
 ID= 1 DT= 5.0 min Area (ha) = 0.66 Total Imp (%) = 62.00 Dir. Conn. (%) = 62.00

Surface Area (ha) = 0.41 IMPERVIOUS 0.25 PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 66.33 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max. Eff. Inten. (mm/hr) = 84.88 66.77
 over (min) = 5.00 15.00
 Storage Coeff. (min) = 2.13 (ii) 11.20 (iii)
 Unit Hyd. Tpeak (min) = 5.00 15.00
 Unit Hyd. peak (cms) = 0.31 0.09

TOTALS
 CN² = 89.0 Ia = Dep. Storage (Above)
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10072)
 ID= 1 DT= 5.0 min Area (ha) = 0.74 Total Imp (%) = 71.00 Dir. Conn. (%) = 71.00

Surface Area (ha) = 0.53 IMPERVIOUS 0.21 PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 70.24 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max. Eff. Inten. (mm/hr) = 84.88 66.77
 over (min) = 5.00 15.00
 Storage Coeff. (min) = 2.21 (ii) 11.28 (iii)
 Unit Hyd. Tpeak (min) = 5.00 15.00
 Unit Hyd. peak (cms) = 0.30 0.09

TOTALS
 CN² = 89.0 Ia = Dep. Storage (Above)
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10081)
 ID= 1 DT= 5.0 min Area (ha) = 0.21 Total Imp (%) = 52.00 Dir. Conn. (%) = 52.00

Surface Area (ha) = 0.11 IMPERVIOUS 0.10 PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 37.42 40.00

TIME TO PEAK (hrs) = 6.00 6.08 6.00
 RUNOFF VOLUME (mm) = 63.30 42.31 55.31
 TOTAL RAINFALL (mm) = 64.30 64.30 64.30
 RUNOFF COEFFICIENT = 0.98 0.66 0.86

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10061)
 ID= 1 DT= 5.0 min Area (ha) = 0.65 Total Imp (%) = 65.00 Dir. Conn. (%) = 65.00

Surface Area (ha) = 0.42 IMPERVIOUS 0.23 PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 65.83 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max. Eff. Inten. (mm/hr) = 84.88 66.77
 over (min) = 5.00 15.00
 Storage Coeff. (min) = 2.12 (ii) 11.19 (iii)
 Unit Hyd. Tpeak (min) = 5.00 15.00
 Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.10 0.03 *TOTALS* 0.125 (iii)
 TIME TO PEAK (hrs) = 6.00 6.08 6.00
 RUNOFF VOLUME (mm) = 63.30 42.31 55.94
 TOTAL RAINFALL (mm) = 64.30 64.30 64.30
 RUNOFF COEFFICIENT = 0.98 0.66 0.87

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CALIB STANDHYD (10071)
 ID= 1 DT= 5.0 min Area (ha) = 0.72 Total Imp (%) = 60.00 Dir. Conn. (%) = 60.00

Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. It shows a transformed hyetograph with time steps of 5.0 minutes.

Max. Eff. Inten. (mm/hr) = 84.88 66.77
Storage Coeff. (min) = 5.00 15.00
Unit Hyd. Tpeak (min) = 1.51 (ii) 10.58 (iii)
Unit Hyd. peak (cms) = 0.33 0.09

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

CALIB STANDHYD (10082) Area (ha) = 0.21 Total Imp (%) = 87.00 Dir. Conn. (%) = 87.00

Surface Area (ha) = 0.18 IMPERVIOUS 0.03 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 37.42 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. It shows a transformed hyetograph with time steps of 5.0 minutes.

Max. Eff. Inten. (mm/hr) = 84.88 66.77
Storage Coeff. (min) = 2.10 (ii) 6.72 (iii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.31 0.14

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

CALIB STANDHYD (10091) Area (ha) = 0.50 Total Imp (%) = 74.00 Dir. Conn. (%) = 74.00

Surface Area (ha) = 0.37 IMPERVIOUS 0.13 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 57.74 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. It shows a transformed hyetograph with time steps of 5.0 minutes.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. It shows a transformed hyetograph with time steps of 5.0 minutes.

Max. Eff. Inten. (mm/hr) = 84.88 66.77
Storage Coeff. (min) = 1.51 (ii) 5.36 (iii)
Unit Hyd. Tpeak (min) = 5.00 15.00
Unit Hyd. peak (cms) = 0.33 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

CALIB STANDHYD (10092) Area (ha) = 0.63 Total Imp (%) = 81.00 Dir. Conn. (%) = 81.00

Surface Area (ha) = 0.51 IMPERVIOUS 0.12 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 64.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. It shows a transformed hyetograph with time steps of 5.0 minutes.

Max. Eff. Inten. (mm/hr) = 84.88 66.77
Storage Coeff. (min) = 5.00 15.00
Unit Hyd. Tpeak (min) = 5.00 15.00
Unit Hyd. peak (cms) = 0.31 0.09

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

CALIB STANDHYD (1101) Area (ha) = 0.91 Total Imp (%) = 55.00 Dir. Conn. (%) = 55.00

Surface Area (ha) = 0.50 IMPERVIOUS 0.41 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 77.89 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. It shows a transformed hyetograph with time steps of 5.0 minutes.

2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max. Eff. Inten. (mm/hr)= 84.88
 over (min)= 5.00
 Storage Coeff. (min)= 2.39 (ii)
 Unit Hyd. Tpeak (min)= 5.00
 Unit Hyd. peak (cms)= 0.30

PEAK FLOW (cms)= 0.12
 TIME TO PEAK (hrs)= 6.00
 RUNOFF VOLUME (mm)= 63.30
 TOTAL RAINFALL (mm)= 64.30
 RUNOFF COEFFICIENT = 0.98

TOTALS
 0.163 (iii)
 6.00
 53.85
 64.30
 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1102)
 ID= 1 DT= 5.0 min

Area (ha)= 0.88
 Total Imp (%) = 72.00 Dir. Conn. (%) = 72.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.63 0.25
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 76.59 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	11.57	9.58	2.25
0.667	1.61	3.667	2.57	6.667	11.57	9.67	2.25
0.750	1.61	3.750	2.57	6.750	11.57	9.75	2.25
0.833	1.61	3.833	2.57	6.833	11.57	9.83	2.25
0.917	1.61	3.917	2.57	6.917	11.57	9.92	2.25
1.000	1.61	4.000	2.57	7.000	11.57	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	3.86	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29

2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max. Eff. Inten. (mm/hr)= 84.88
 over (min)= 5.00
 Storage Coeff. (min)= 2.32 (ii)
 Unit Hyd. Tpeak (min)= 5.00
 Unit Hyd. peak (cms)= 0.30

PEAK FLOW (cms)= 0.15
 TIME TO PEAK (hrs)= 6.00
 RUNOFF VOLUME (mm)= 63.30
 TOTAL RAINFALL (mm)= 64.30
 RUNOFF COEFFICIENT = 0.98

TOTALS
 0.177 (iii)
 6.00
 57.41
 64.30
 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1111)
 ID= 1 DT= 5.0 min

Area (ha)= 1.06
 Total Imp (%) = 59.00 Dir. Conn. (%) = 59.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.63 0.43
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 84.06 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	11.57	9.58	2.25
0.667	1.61	3.667	2.57	6.667	11.57	9.67	2.25
0.750	1.61	3.750	2.57	6.750	11.57	9.75	2.25
0.833	1.61	3.833	2.57	6.833	11.57	9.83	2.25
0.917	1.61	3.917	2.57	6.917	11.57	9.92	2.25
1.000	1.61	4.000	2.57	7.000	11.57	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	3.86	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max. Eff. Inten. (mm/hr)= 84.88
 over (min)= 5.00
 Storage Coeff. (min)= 2.46 (ii)
 Unit Hyd. Tpeak (min)= 5.00
 Unit Hyd. peak (cms)= 0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1122)
 ID= 1 DT= 5.0 min

Area (ha)= 1.22
 Total Imp (%) = 69.00 Dir. Conn. (%) = 69.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.84 0.38
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 90.18 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	11.57	9.58	2.25
0.667	1.61	3.667	2.57	6.667	11.57	9.67	2.25
0.750	1.61	3.750	2.57	6.750	11.57	9.75	2.25
0.833	1.61	3.833	2.57	6.833	11.57	9.83	2.25
0.917	1.61	3.917	2.57	6.917	11.57	9.92	2.25
1.000	1.61	4.000	2.57	7.000	11.57	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	3.86	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29

CALIB STANDBYD (1121) Area (ha)= 1.21 Total Imp (%) = 68.00 Dir. Conn. (%) = 68.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.82 (mm)= 1.00 Dep. Storage (mm)= 1.00 Average Slope (%) = 1.00 Length (m) = 89.81 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 12 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals from 0.083 to 3.000 hours with corresponding rainfall and transformed values.

Max. Eff. Inten. (mm/hr)= 84.88 over (min)= 5.00 Storage Coeff. (min)= 2.56 (ii) Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.29

PEAK FLOW (cms)= 0.19 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 64.30 TOTAL RAINFALL (mm)= 64.30 RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

V V I SSSSS U U A A L (v 6.1.2002) V V I SS U U A A A L V V I SS U U A A A L

Table with 12 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals from 0.833 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 100.32 over (min)= 5.00 Storage Coeff. (min)= 1.41 (ii) Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.33

PEAK FLOW (cms)= 0.05 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 75.00 TOTAL RAINFALL (mm)= 76.00 RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10022) Area (ha)= 0.83 Total Imp (%) = 75.00 Dir. Conn. (%) = 75.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.62 (mm)= 1.00 Dep. Storage (mm)= 1.00 Average Slope (%) = 1.00 Length (m) = 74.39 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 12 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals from 0.083 to 1.417 hours.

W I SSSSS UUUUU A A LLLLL OOO TTTT TTTT H H Y Y M M OOO TM O O T T T H H Y Y M M O O O O T T H H Y Y M M O O O O T T H H Y Y M M O O O

Developed and Distributed by Smart City Water Inc Copyright 2007 - 2020 Smart City Water Inc All rights reserved. ***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.1\VO2\vojn.dat Output filename: C:\Users\caef07146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\c5677e56 Summary filename: C:\Users\caef07146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\c5677e56

DATE: 05-18-2022 TIME: 12:41:44 USER: COMMENTS:

***** SIMULATION : Run 15 ***** READ STORM File name: C:\Users\caef07146\AppData\Local\Temp\aca14864-P56d-471c-a2fc-7a8c6cfa585\ba07a6c Ptotal = 76.00 mm Comments: 10-Yr, 12-hr SCS, City of Barrie

Table with 12 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals from 0.25 to 3.000 hours.

CALIB STANDBYD (10011) Area (ha)= 0.21 Total Imp (%) = 79.00 Dir. Conn. (%) = 79.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.17 (mm)= 1.00 Dep. Storage (mm)= 1.00 Average Slope (%) = 37.42 Length (m) = 37.42 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 12 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals from 0.083 to 0.750 hours.

Table with 12 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals from 1.500 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 100.32 over (min)= 5.00 Storage Coeff. (min)= 2.14 (ii) Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.31

PEAK FLOW (cms)= 0.17 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 75.00 TOTAL RAINFALL (mm)= 76.00 RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10021) Area (ha)= 0.73 Total Imp (%) = 71.00 Dir. Conn. (%) = 71.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.52 (mm)= 1.00 Dep. Storage (mm)= 1.00 Average Slope (%) = 69.76 Length (m) = 69.76 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 12 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals from 0.083 to 2.083 hours.

2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) = 5.00 15.00
Storage Coeff.(min) = 2.06 (ii) 10.38 (ii)
Unit Hyd. Tpeak (min) = 5.00 15.00
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.14 0.03 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.08 0.175 (iii)
RUNOFF VOLUME (mm) = 75.00 52.87 68.57
TOTAL RAINFALL (mm) = 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^o = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10031)
ID= 1 DT= 5.0 min | Area (ha)= 0.38
| Total Imp(%)= 66.00 Dir. Conn.(%)= 66.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	0.25	0.13	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	50.33	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52

Unit Hyd. peak (cms) = 0.32 0.15

PEAK FLOW (cms) = 0.09 0.02 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.00 0.102 (iii)
RUNOFF VOLUME (mm) = 75.00 52.87 70.35
TOTAL RAINFALL (mm) = 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^o = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10041)
ID= 1 DT= 5.0 min | Area (ha)= 0.35
| Total Imp(%)= 69.00 Dir. Conn.(%)= 69.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	0.11	0.10	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	48.30	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) = 5.00 10.00
Storage Coeff.(min) = 1.65 (ii) 9.98 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.11

PEAK FLOW (cms) = 0.07 0.02 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.00 0.086 (iii)
RUNOFF VOLUME (mm) = 75.00 52.87 68.12
TOTAL RAINFALL (mm) = 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.90

2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) = 5.00 15.00
Storage Coeff.(min) = 1.69 (ii) 10.02 (ii)
Unit Hyd. Tpeak (min) = 5.00 15.00
Unit Hyd. peak (cms) = 0.32 0.10

PEAK FLOW (cms) = 0.07 0.02 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.08 0.089 (iii)
RUNOFF VOLUME (mm) = 75.00 52.87 67.46
TOTAL RAINFALL (mm) = 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^o = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10032)
ID= 1 DT= 5.0 min | Area (ha)= 0.39
| Total Imp(%)= 79.00 Dir. Conn.(%)= 79.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	0.31	0.08	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	50.99	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2		

CALIB
STANDHYD (10012)
ID= 1 DT= 5.0 min

Area (ha)= 0.23
Total Imp(%)= 78.00

Surface Area (ha)= IMPERVIOUS 0.01 PERVIOUS (i) 0.18
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 39.16 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) 5.00 10.00
Storage Coeff (min)= 1.66 (ii) 6.11 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15
PEAK FLOW (cms)= 0.05 0.01 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.060 (iii)
RUNOFF VOLUME (mm)= 75.00 52.87 70.12
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10051)
ID= 1 DT= 5.0 min

Area (ha)= 0.37
Total Imp(%)= 66.00

Surface Area (ha)= IMPERVIOUS 0.24 PERVIOUS (i) 0.13

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) 5.00 10.00
Storage Coeff (min)= 1.66 (ii) 9.99 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.11
PEAK FLOW (cms)= 0.06 0.02 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.086 (iii)
RUNOFF VOLUME (mm)= 75.00 52.87 66.57
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10062)
ID= 1 DT= 5.0 min

Area (ha)= 0.66
Total Imp(%)= 62.00

Surface Area (ha)= IMPERVIOUS 0.41 PERVIOUS (i) 0.15
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 66.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66

Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 49.67 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) 5.00 15.00
Storage Coeff (min)= 1.68 (ii) 10.00 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.10
PEAK FLOW (cms)= 0.07 0.02 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.08 0.087 (iii)
RUNOFF VOLUME (mm)= 75.00 52.87 67.46
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (10052)
ID= 1 DT= 5.0 min

Area (ha)= 0.36
Total Imp(%)= 62.00

Surface Area (ha)= IMPERVIOUS 0.22 PERVIOUS (i) 0.14
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

Hydrograph data table with columns for Time (hrs), Rain (mm/hr), and Rainfall (mm) for various durations (1.50 to 3.00). Includes summary statistics like Max. Eff. Inten. and Storage Coeff.

Summary statistics for the first hydrograph: Max. Eff. Inten. (mm/hr) = 100.32, Storage Coeff. (min) = 1.99, etc. Includes a warning about storage coefficient and procedure notes.

Procedure notes for the first hydrograph: (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0, (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

CALIB STANDBYD (10072) Area (ha) = 0.74, Total Imp (%) = 71.00, Dir. Conn. (%) = 71.00, ID = 1 DT = 5.0 min

Surface Area (ha) = 0.33, ImperVIOUS (mm) = 1.00, PervIOUS (i) (mm) = 2.00, Average Slope (%) = 70.24, Length (m) = 0.290, Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Transformed hydrograph data table for the first hydrograph, showing Time (hrs), Rain (mm/hr), and Rainfall (mm) with a 5.0 min time step.

Hydrograph data table for the second hydrograph, similar to the first but with different rainfall intensities.

Summary statistics for the second hydrograph: Max. Eff. Inten. (mm/hr) = 100.32, Storage Coeff. (min) = 2.05, etc. Includes a warning about storage coefficient.

Procedure notes for the second hydrograph: (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0, (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

CALIB STANDBYD (10081) Area (ha) = 0.21, Total Imp (%) = 52.00, Dir. Conn. (%) = 52.00, ID = 1 DT = 5.0 min

Surface Area (ha) = 0.11, ImperVIOUS (mm) = 1.00, PervIOUS (i) (mm) = 2.00, Average Slope (%) = 37.42, Length (m) = 0.290, Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Transformed hydrograph data table for the second hydrograph, showing Time (hrs), Rain (mm/hr), and Rainfall (mm) with a 5.0 min time step.

Hydrograph data table for the third hydrograph, showing Time (hrs), Rain (mm/hr), and Rainfall (mm).

Summary statistics for the third hydrograph: Max. Eff. Inten. (mm/hr) = 100.32, Storage Coeff. (min) = 2.06, etc. Includes a warning about storage coefficient.

Warning: STORAGE COEFF. IS SMALLER THAN TIME STEP!

Procedure notes for the third hydrograph: (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0, (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

CALIB STANDBYD (10071) Area (ha) = 0.72, Total Imp (%) = 60.00, Dir. Conn. (%) = 60.00, ID = 1 DT = 5.0 min

Surface Area (ha) = 0.43, ImperVIOUS (mm) = 1.00, PervIOUS (i) (mm) = 2.00, Average Slope (%) = 69.28, Length (m) = 0.290, Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Transformed hydrograph data table for the third hydrograph, showing Time (hrs), Rain (mm/hr), and Rainfall (mm) with a 5.0 min time step.

Hydrograph data table for the fourth hydrograph, showing Time (hrs), Rain (mm/hr), and Rainfall (mm).

Summary statistics for the fourth hydrograph: Max. Eff. Inten. (mm/hr) = 100.32, Storage Coeff. (min) = 1.41, etc. Includes a warning about storage coefficient.

Procedure notes for the fourth hydrograph: (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0, (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

CALIB STANDBYD (10082) Area (ha) = 0.21, Total Imp (%) = 87.00, Dir. Conn. (%) = 87.00, ID = 1 DT = 5.0 min

Surface Area (ha) = 0.18, ImperVIOUS (mm) = 1.00, PervIOUS (i) (mm) = 2.00, Average Slope (%) = 37.42, Length (m) = 0.290, Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Transformed hydrograph data table for the fourth hydrograph, showing Time (hrs), Rain (mm/hr), and Rainfall (mm) with a 5.0 min time step.

RUNOFF VOLUME (mm)= 75.00 52.87 72.12
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (10092) Area (ha)= 0.63
 ID= 1 DT= 5.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.51	0.12
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	64.81	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66				
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66				
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66				
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66				
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66				
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66				
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66				
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66				
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66				
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66				
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66				
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66				
1.083	1.90	4.083	3.04	7.083	13.68	10.08	2.66				
1.167	1.90	4.167	3.04	7.167	13.68	10.17	2.66				
1.250	1.90	4.250	3.04	7.250	13.68	10.25	2.66				
1.333	1.90	4.333	3.04	7.333	13.68	10.33	2.66				
1.417	1.90	4.417	3.04	7.417	13.68	10.42	2.66				
1.500	1.90	4.500	3.04	7.500	13.68	10.50	2.66				
1.583	1.90	4.583	3.04	7.583	13.68	10.58	2.66				
1.667	1.90	4.667	3.04	7.667	13.68	10.67	2.66				
1.750	1.90	4.750	3.04	7.750	13.68	10.75	2.66				
1.833	1.90	4.833	3.04	7.833	13.68	10.83	2.66				
1.917	1.90	4.917	3.04	7.917	13.68	10.92	2.66				
2.000	1.90	5.000	3.04	8.000	13.68	11.00	2.66				
2.083	2.28	5.083	3.04	8.083	13.68	11.08	2.66				
2.167	2.28	5.167	3.04	8.167	13.68	11.17	2.66				
2.250	2.28	5.250	3.04	8.250	13.68	11.25	2.66				
2.333	2.28	5.333	3.04	8.333	13.68	11.33	2.66				
2.417	2.28	5.417	3.04	8.417	13.68	11.42	2.66				
2.500	2.28	5.500	3.04	8.500	13.68	11.50	2.66				
2.583	2.28	5.583	3.04	8.583	13.68	11.58	2.66				
2.667	2.28	5.667	3.04	8.667	13.68	11.67	2.66				
2.750	2.28	5.750	3.04	8.750	13.68	11.75	2.66				
2.833	2.28	5.833	3.04	8.833	13.68	11.83	2.66				
2.917	2.28	5.917	3.04	8.917	13.68	11.92	2.66				
3.000	2.28	6.000	3.04	9.000	13.68	12.00	2.66				

Max. Eff. Inten. (mm/hr)= 100.32 82.63
 over (min)= 5.00 15.00
 Storage Coeff. (min)= 1.97 (ii) 10.52 (ii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.31 0.09

PEAK FLOW (cms)= 0.14 0.06 0.199 (iii)
 TIME TO PEAK (hrs)= 6.00 6.08 6.00
 RUNOFF VOLUME (mm)= 75.00 52.87 76.00
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
 CN* = 89.0 Ia = Dep. Storage (Above)

ID= 1 DT= 5.0 min | Total Imp(%)= 55.00 Dir. Conn.(%)= 55.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.50	0.41
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	77.89	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66				
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66				
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66				
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66				
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66				
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66				
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66				
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66				
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66				
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66				
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66				
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66				
1.083	1.90	4.083	3.04	7.083	13.68	10.08	2.66				
1.167	1.90	4.167	3.04	7.167	13.68	10.17	2.66				
1.250	1.90	4.250	3.04	7.250	13.68	10.25	2.66				
1.333	1.90	4.333	3.04	7.333	13.68	10.33	2.66				
1.417	1.90	4.417	3.04	7.417	13.68	10.42	2.66				
1.500	1.90	4.500	3.04	7.500	13.68	10.50	2.66				
1.583	1.90	4.583	3.04	7.583	13.68	10.58	2.66				
1.667	1.90	4.667	3.04	7.667	13.68	10.67	2.66				
1.750	1.90	4.750	3.04	7.750	13.68	10.75	2.66				
1.833	1.90	4.833	3.04	7.833	13.68	10.83	2.66				
1.917	1.90	4.917	3.04	7.917	13.68	10.92	2.66				
2.000	1.90	5.000	3.04	8.000	13.68	11.00	2.66				
2.083	2.28	5.083	3.04	8.083	13.68	11.08	2.66				
2.167	2.28	5.167	3.04	8.167	13.68	11.17	2.66				
2.250	2.28	5.250	3.04	8.250	13.68	11.25	2.66				
2.333	2.28	5.333	3.04	8.333	13.68	11.33	2.66				
2.417	2.28	5.417	3.04	8.417	13.68	11.42	2.66				
2.500	2.28	5.500	3.04	8.500	13.68	11.50	2.66				
2.583	2.28	5.583	3.04	8.583	13.68	11.58	2.66				
2.667	2.28	5.667	3.04	8.667	13.68	11.67	2.66				
2.750	2.28	5.750	3.04	8.750	13.68	11.75	2.66				
2.833	2.28	5.833	3.04	8.833	13.68	11.83	2.66				
2.917	2.28	5.917	3.04	8.917	13.68	11.92	2.66				
3.000	2.28	6.000	3.04	9.000	13.68	12.00	2.66				

Max. Eff. Inten. (mm/hr)= 100.32 82.63
 over (min)= 5.00 15.00
 Storage Coeff. (min)= 2.20 (ii) 10.52 (ii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.30 0.09

PEAK FLOW (cms)= 0.14 0.06 0.199 (iii)
 TIME TO PEAK (hrs)= 6.00 6.08 6.00
 RUNOFF VOLUME (mm)= 75.00 52.87 76.00
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (1102) Area (ha)= 0.88
 ID= 1 DT= 5.0 min Total Imp(%)= 72.00 Dir. Conn.(%)= 72.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.63	0.25
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	76.59	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66				
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66				
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66				
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66				
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66				
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66				
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66				
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66				
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66				
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66				
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66				
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66				
1.083	1.90	4.083	3.04	7.083	13.68	10.08	2.66				
1.167	1.90	4.167	3.04	7.167	13.68	10.17	2.66				
1.250	1.90	4.250	3.04	7.250	13.68	10.25	2.66				
1.333	1.90	4.333	3.04	7.333	13.68	10.33	2.66				
1.417	1.90	4.417	3.04	7.417	13.68	10.42	2.66				
1.500	1.90	4.500	3.04	7.500	13.68	10.50	2.66				
1.583	1.90	4.583	3.04	7.583	13.68	10.58	2.66				
1.667	1.90	4.667	3.04	7.667	13.68	10.67	2.66				
1.750	1.90	4.750	3.04	7.750	13.68	10.75	2.66				
1.833	1.90	4.833	3.04	7.833	13.68	10.83	2.66				
1.917	1.90	4.									

0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	3.04	7.083	4.56	10.08	2.66
1.167	1.90	4.167	4.56	7.167	4.56	10.17	2.66
1.250	1.90	4.250	4.56	7.250	4.56	10.25	2.66
1.333	1.90	4.333	4.56	7.333	4.56	10.33	2.66
1.417	1.90	4.417	4.56	7.417	4.56	10.42	2.66
1.500	1.90	4.500	4.56	7.500	4.56	10.50	2.66
1.583	1.90	4.583	6.08	7.583	4.56	10.58	2.66
1.667	1.90	4.667	6.08	7.667	4.56	10.67	2.66
1.750	1.90	4.750	6.08	7.750	4.56	10.75	2.66
1.833	1.90	4.833	6.08	7.833	4.56	10.83	2.66
1.917	1.90	4.917	6.08	7.917	4.56	10.92	2.66
2.000	1.90	5.000	6.08	8.000	4.56	11.00	2.66
2.083	2.28	5.083	9.12	8.083	2.66	11.08	2.66
2.167	2.28	5.167	9.12	8.167	2.66	11.17	2.66
2.250	2.28	5.250	9.12	8.250	2.66	11.25	2.66
2.333	2.28	5.333	9.12	8.333	2.66	11.33	2.66
2.417	2.28	5.417	9.12	8.417	2.66	11.42	2.66
2.500	2.28	5.500	9.12	8.500	2.66	11.50	2.66
2.583	2.28	5.583	36.48	8.583	2.66	11.58	2.66
2.667	2.28	5.667	36.48	8.667	2.66	11.67	2.66
2.750	2.28	5.750	36.48	8.750	2.66	11.75	2.66
2.833	2.28	5.833	100.32	8.833	2.66	11.83	2.66
2.917	2.28	5.917	100.32	8.917	2.66	11.92	2.66
3.000	2.28	6.000	100.32	9.000	2.66	12.00	2.66

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.30 (ii) 10.63 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.30 0.09

PEAK FLOW (cms)= 0.17 0.07 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 75.00 52.87 68.14
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1112)
ID= 1 DT= 5.0 min | Area (ha)= 0.97 Dir. Conn.(%)= 78.00
Total Imp (%)= 78.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.76 0.21
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 80.42 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66

1.500	1.90	4.500	4.56	7.500	4.56	10.50	2.66
1.583	1.90	4.583	6.08	7.583	4.56	10.58	2.66
1.667	1.90	4.667	6.08	7.667	4.56	10.67	2.66
1.750	1.90	4.750	6.08	7.750	4.56	10.75	2.66
1.833	1.90	4.833	6.08	7.833	4.56	10.83	2.66
1.917	1.90	4.917	6.08	7.917	4.56	10.92	2.66
2.000	1.90	5.000	6.08	8.000	4.56	11.00	2.66
2.083	2.28	5.083	9.12	8.083	2.66	11.08	2.66
2.167	2.28	5.167	9.12	8.167	2.66	11.17	2.66
2.250	2.28	5.250	9.12	8.250	2.66	11.25	2.66
2.333	2.28	5.333	9.12	8.333	2.66	11.33	2.66
2.417	2.28	5.417	9.12	8.417	2.66	11.42	2.66
2.500	2.28	5.500	9.12	8.500	2.66	11.50	2.66
2.583	2.28	5.583	36.48	8.583	2.66	11.58	2.66
2.667	2.28	5.667	36.48	8.667	2.66	11.67	2.66
2.750	2.28	5.750	36.48	8.750	2.66	11.75	2.66
2.833	2.28	5.833	100.32	8.833	2.66	11.83	2.66
2.917	2.28	5.917	100.32	8.917	2.66	11.92	2.66
3.000	2.28	6.000	100.32	9.000	2.66	12.00	2.66

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.40 (ii) 10.72 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.30 0.09

PEAK FLOW (cms)= 0.23 0.06 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 75.00 52.87 68.14
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1121)
ID= 1 DT= 5.0 min | Area (ha)= 1.21 Dir. Conn.(%)= 68.00
Total Imp (%)= 68.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.82 0.39
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 89.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	2.66
1.167	1.90	4.167	4.56	7.167	4.56	10.17	2.66
1.250	1.90	4.250	4.56	7.250	4.56	10.25	2.66
1.333	1.90	4.333	4.56	7.333	4.56	10.33	2.66
1.417	1.90	4.417	4.56	7.417	4.56	10.42	2.66
1.500	1.90	4.500	4.56	7.500	4.56	10.50	2.66
1.583	1.90	4.583	6.08	7.583	4.56	10.58	2.66
1.667	1.90	4.667	6.08	7.667	4.56	10.67	2.66
1.750	1.90	4.750	6.08	7.750	4.56	10.75	2.66
1.833	1.90	4.833	6.08	7.833	4.56	10.83	2.66
1.917	1.90	4.917	6.08	7.917	4.56	10.92	2.66
2.000	1.90	5.000	6.08	8.000	4.56	11.00	2.66
2.083	2.28	5.083	9.12	8.083	2.66	11.08	2.66

0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	2.66
1.167	1.90	4.167	4.56	7.167	4.56	10.17	2.66
1.250	1.90	4.250	4.56	7.250	4.56	10.25	2.66
1.333	1.90	4.333	4.56	7.333	4.56	10.33	2.66
1.417	1.90	4.417	4.56	7.417	4.56	10.42	2.66
1.500	1.90	4.500	4.56	7.500	4.56	10.50	2.66
1.583	1.90	4.583	6.08	7.583	4.56	10.58	2.66
1.667	1.90	4.667	6.08	7.667	4.56	10.67	2.66
1.750	1.90	4.750	6.08	7.750	4.56	10.75	2.66
1.833	1.90	4.833	6.08	7.833	4.56	10.83	2.66
1.917	1.90	4.917	6.08	7.917	4.56	10.92	2.66
2.000	1.90	5.000	6.08	8.000	4.56	11.00	2.66
2.083	2.28	5.083	9.12	8.083	2.66	11.08	2.66
2.167	2.28	5.167	9.12	8.167	2.66	11.17	2.66
2.250	2.28	5.250	9.12	8.250	2.66	11.25	2.66
2.333	2.28	5.333	9.12	8.333	2.66	11.33	2.66
2.417	2.28	5.417	9.12	8.417	2.66	11.42	2.66
2.500	2.28	5.500	9.12	8.500	2.66	11.50	2.66
2.583	2.28	5.583	36.48	8.583	2.66	11.58	2.66
2.667	2.28	5.667	36.48	8.667	2.66	11.67	2.66
2.750	2.28	5.750	36.48	8.750	2.66	11.75	2.66
2.833	2.28	5.833	100.32	8.833	2.66	11.83	2.66
2.917	2.28	5.917	100.32	8.917	2.66	11.92	2.66
3.000	2.28	6.000	100.32	9.000	2.66	12.00	2.66

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.24 (ii) 6.88 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.30 0.14

PEAK FLOW (cms)= 0.21 0.04 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 75.00 52.87 68.14
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1122)
ID= 1 DT= 5.0 min | Area (ha)= 1.22 Dir. Conn.(%)= 69.00
Total Imp (%)= 69.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.84 0.38
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 90.18 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	

Table with 7 columns: 1.00, 2.27, 4.00, 3.63, 7.00, 7.26, 10.00, 3.17. Values decrease in increments of 0.27 until 2.50, then increase to 3.00.

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 2 columns: CALIB STANDHYD (10022) ID= 1 DT= 5.0 min. Area (ha)= 0.83, Total Imp(%)= 75.00, Dir. Conn.(%)= 75.00. Sub-table for IMPERVIOUS and PERVIOUS (i) with Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 7 columns: 0.83, 2.27, 3.083, 3.63, 6.083, 16.33, 9.08. Values increase in increments of 0.83 until 3.083, then decrease to 0.167.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Data points from 0.083 to 3.000.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Data points from 0.083 to 3.000.

Summary statistics table: Max. Eff. Inten. (mm/hr)= 119.72, Storage Coeff. (min)= 5.00, Unit Hyd. Tpeak (min)= 5.00, Unit Hyd. peak (cms)= 0.31. Includes TOTALS* and WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 7 columns: 1.00, 2.27, 4.00, 3.63, 7.00, 7.26, 10.00, 3.17. Values decrease in increments of 0.27 until 2.50, then increase to 3.00.

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 2 columns: CALIB STANDHYD (10021) ID= 1 DT= 5.0 min. Area (ha)= 0.73, Total Imp(%)= 71.00, Dir. Conn.(%)= 71.00. Sub-table for IMPERVIOUS and PERVIOUS (i) with Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 7 columns: 0.83, 2.27, 3.083, 3.63, 6.083, 16.33, 9.08. Values increase in increments of 0.83 until 3.083, then decrease to 0.167.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Data points from 0.083 to 3.000.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Data points from 0.083 to 3.000.

Summary statistics table: Max. Eff. Inten. (mm/hr)= 119.72, Storage Coeff. (min)= 5.00, Unit Hyd. Tpeak (min)= 5.00, Unit Hyd. peak (cms)= 0.33. Includes TOTALS* and WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 7 columns: 1.00, 2.27, 4.00, 3.63, 7.00, 7.26, 10.00, 3.17. Values decrease in increments of 0.27 until 2.50, then increase to 3.00.

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 2 columns: CALIB STANDHYD (10032) ID= 1 DT= 5.0 min. Area (ha)= 0.39, Total Imp(%)= 79.00, Dir. Conn.(%)= 79.00. Sub-table for IMPERVIOUS and PERVIOUS (i) with Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 7 columns: 0.83, 2.27, 3.083, 3.63, 6.083, 16.33, 9.08. Values increase in increments of 0.83 until 3.083, then decrease to 0.167.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Data points from 0.083 to 3.000.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Data points from 0.083 to 3.000.

Summary statistics table: Max. Eff. Inten. (mm/hr)= 119.72, Storage Coeff. (min)= 5.00, Unit Hyd. Tpeak (min)= 5.00, Unit Hyd. peak (cms)= 0.33. Includes TOTALS* and WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17
0.667	2.27	3.667	3.63	6.667	16.33	9.67	3.17
0.750	2.27	3.750	3.63	6.750	16.33	9.75	3.17
0.833	2.27	3.833	3.63	6.833	16.33	9.83	3.17
0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17
1.083	2.27	4.083	3.63	7.083	16.33	10.08	1.81
1.167	2.27	4.167	3.63	7.167	16.33	10.17	1.81
1.250	2.27	4.250	3.63	7.250	16.33	10.25	1.81
1.333	2.27	4.333	3.63	7.333	16.33	10.33	1.81
1.417	2.27	4.417	3.63	7.417	16.33	10.42	1.81
1.500	2.27	4.500	3.63	7.500	16.33	10.50	1.81
1.583	2.27	4.583	3.63	7.583	16.33	10.58	1.81
1.667	2.27	4.667	3.63	7.667	16.33	10.67	1.81
1.750	2.27	4.750	3.63	7.750	16.33	10.75	1.81
1.833	2.27	4.833	3.63	7.833	16.33	10.83	1.81
1.917	2.27	4.917	3.63	7.917	16.33	10.92	1.81
2.000	2.27	5.000	3.63	8.000	16.33	11.00	1.81
2.083	2.27	5.083	3.63	8.083	16.33	11.08	1.81
2.167	2.27	5.167	3.63	8.167	16.33	11.17	1.81
2.250	2.27	5.250	3.63	8.250	16.33	11.25	1.81
2.333	2.27	5.333	3.63	8.333	16.33	11.33	1.81
2.417	2.27	5.417	3.63	8.417	16.33	11.42	1.81
2.500	2.27	5.500	3.63	8.500	16.33	11.50	1.81
2.583	2.27	5.583	3.63	8.583	16.33	11.58	1.81
2.667	2.27	5.667	3.63	8.667	16.33	11.67	1.81
2.750	2.27	5.750	3.63	8.750	16.33	11.75	1.81
2.833	2.27	5.833	3.63	8.833	16.33	11.83	1.81
2.917	2.27	5.917	3.63	8.917	16.33	11.92	1.81
3.000	2.27	6.000	3.63	9.000	16.33	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72 102.72
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.59 (ii) 5.81 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.10 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.123 (iii)
 RUNOFF VOLUME (mm)= 89.70 66.45 84.81
 TOTAL RAINFALL (mm)= 90.70 90.70 90.70
 RUNOFF COEFFICIENT = 0.99 0.73 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (10041) Area (ha)= 0.35
 ID= 1 DT= 5.0 min Total Imp(%)= 69.00 Dir. Conn.(%)= 69.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.24 0.10
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 40.30 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

1.583	2.27	4.583	3.63	7.583	16.33	10.58	1.81
1.667	2.27	4.667	3.63	7.667	16.33	10.67	1.81
1.750	2.27	4.750	3.63	7.750	16.33	10.75	1.81
1.833	2.27	4.833	3.63	7.833	16.33	10.83	1.81
1.917	2.27	4.917	3.63	7.917	16.33	10.92	1.81
2.000	2.27	5.000	3.63	8.000	16.33	11.00	1.81
2.083	2.27	5.083	3.63	8.083	16.33	11.08	1.81
2.167	2.27	5.167	3.63	8.167	16.33	11.17	1.81
2.250	2.27	5.250	3.63	8.250	16.33	11.25	1.81
2.333	2.27	5.333	3.63	8.333	16.33	11.33	1.81
2.417	2.27	5.417	3.63	8.417	16.33	11.42	1.81
2.500	2.27	5.500	3.63	8.500	16.33	11.50	1.81
2.583	2.27	5.583	3.63	8.583	16.33	11.58	1.81
2.667	2.27	5.667	3.63	8.667	16.33	11.67	1.81
2.750	2.27	5.750	3.63	8.750	16.33	11.75	1.81
2.833	2.27	5.833	3.63	8.833	16.33	11.83	1.81
2.917	2.27	5.917	3.63	8.917	16.33	11.92	1.81
3.000	2.27	6.000	3.63	9.000	16.33	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72 102.72
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.57 (ii) 6.30 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.09 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.118 (iii)
 RUNOFF VOLUME (mm)= 83.70 66.45 84.81
 TOTAL RAINFALL (mm)= 90.70 90.70 90.70
 RUNOFF COEFFICIENT = 0.99 0.73 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (10012) Area (ha)= 0.23
 ID= 1 DT= 5.0 min Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.18 0.05
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 39.16 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17
0.667	2.27	3.667	3.63	6.667	16.33	9.67	3.17
0.750	2.27	3.750	3.63	6.750	16.33	9.75	3.17
0.833	2.27	3.833	3.63	6.833	16.33	9.83	3.17
0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17
1.083	2.27	4.083	3.63	7.083	16.33	10.08	1.81
1.167	2.27	4.167	3.63	7.167	16.33	10.17	1.81
1.250	2.27	4.250	3.63	7.250	16.33	10.25	1.81
1.333	2.27	4.333	3.63	7.333	16.33	10.33	1.81
1.417	2.27	4.417	3.63	7.417	16.33	10.42	1.81
1.500	2.27	4.500	3.63	7.500	16.33	10.50	1.81
1.583	2.27	4.583	3.63	7.583	16.33	10.58	1.81
1.667	2.27	4.667	3.63	7.667	16.33	10.67	1.81
1.750	2.27	4.750	3.63	7.750	16.33	10.75	1.81
1.833	2.27	4.833	3.63	7.833	16.33	10.83	1.81
1.917	2.27	4.917	3.63	7.917	16.33	10.92	1.81
2.000	2.27	5.000	3.63	8.000	16.33	11.00	1.81
2.083	2.27	5.083	3.63	8.083	16.33	11.08	1.81
2.167	2.27	5.167	3.63	8.167	16.33	11.17	1.81

0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17
1.083	2.27	4.083	3.63	7.083	16.33	10.08	1.81
1.167	2.27	4.167	3.63	7.167	16.33	10.17	1.81
1.250	2.27	4.250	3.63	7.250	16.33	10.25	1.81
1.333	2.27	4.333	3.63	7.333	16.33	10.33	1.81
1.417	2.27	4.417	3.63	7.417	16.33	10.42	1.81
1.500	2.27	4.500	3.63	7.500	16.33	10.50	1.81
1.583	2.27	4.583	3.63	7.583	16.33	10.58	1.81
1.667	2.27	4.667	3.63	7.667	16.33	10.67	1.81
1.750	2.27	4.750	3.63	7.750	16.33	10.75	1.81
1.833	2.27	4.833	3.63	7.833	16.33	10.83	1.81
1.917	2.27	4.917	3.63	7.917	16.33	10.92	1.81
2.000	2.27	5.000	3.63	8.000	16.33	11.00	1.81
2.083	2.27	5.083	3.63	8.083	16.33	11.08	1.81
2.167	2.27	5.167	3.63	8.167	16.33	11.17	1.81
2.250	2.27	5.250	3.63	8.250	16.33	11.25	1.81
2.333	2.27	5.333	3.63	8.333	16.33	11.33	1.81
2.417	2.27	5.417	3.63	8.417	16.33	11.42	1.81
2.500	2.27	5.500	3.63	8.500	16.33	11.50	1.81
2.583	2.27	5.583	3.63	8.583	16.33	11.58	1.81
2.667	2.27	5.667	3.63	8.667	16.33	11.67	1.81
2.750	2.27	5.750	3.63	8.750	16.33	11.75	1.81
2.833	2.27	5.833	3.63	8.833	16.33	11.83	1.81
2.917	2.27	5.917	3.63	8.917	16.33	11.92	1.81
3.000	2.27	6.000	3.63	9.000	16.33	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72 102.72
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.54 (ii) 9.17 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.12

PEAK FLOW (cms)= 0.08 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.104 (iii)
 RUNOFF VOLUME (mm)= 89.70 66.45 82.47
 TOTAL RAINFALL (mm)= 90.70 90.70 90.70
 RUNOFF COEFFICIENT = 0.99 0.73 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (10042) Area (ha)= 0.38
 ID= 1 DT= 5.0 min Total Imp(%)= 74.00 Dir. Conn.(%)= 74.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.28 0.10
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 50.33 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17
0.667	2.27	3.667	3.63	6.667	16.33	9.67	3.17
0.750	2.27	3.750	3.63	6.750	16.33	9.75	3.17
0.833	2.27	3.833	3.63	6.833	16.33	9.83	3.17
0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17
1.083	2.27	4.083	3.63	7.083	16.33	10.08	1.81
1.167	2.27	4.167	3.63	7.167	16.33	10.17	1.81
1.250	2.27	4.250	3.63	7.250	16.33	10.25	1.81
1.333	2.27	4.333	3.63	7.333	16.33	10.33	1.81
1.417	2.27	4.417	3.63	7.417	16.33	10.42	1.81
1.500	2.27	4.50					

2.917 2.72 | 5.917 119.72 | 8.917 3.17 | 11.92 1.81
3.000 2.72 | 6.000 119.72 | 9.000 3.17 | 12.00 1.81

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.56 (ii) 9.19 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.12
PEAK FLOW (cms)= 0.08 0.03 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.109 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 81.77
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | STANHYD (10052) | Area (ha)= 0.36
ID= 1 DT= 5.0 min | Total Imp (%) = 62.00 Dir. Conn. (%) = 62.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.22 0.14
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.55 (ii) 9.18 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.12

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | STANHYD (10061) | Area (ha)= 0.65
ID= 1 DT= 5.0 min | Total Imp (%) = 65.00 Dir. Conn. (%) = 65.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.42 0.23
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 65.83 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.85 (ii) 9.48 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.12
PEAK FLOW (cms)= 0.14 0.05 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.190 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 81.55
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

PEAK FLOW (cms)= 0.07 0.03 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.104 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 80.85
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | STANHYD (10062) | Area (ha)= 0.66
ID= 1 DT= 5.0 min | Total Imp (%) = 62.00 Dir. Conn. (%) = 62.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.41 0.25
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 66.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.86 (ii) 9.49 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.14 0.05 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.191 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 80.86
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

CALIB | STANHYD (10072) | Area (ha)= 0.74
ID= 1 DT= 5.0 min | Total Imp (%) = 71.00 Dir. Conn. (%) = 71.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.53 0.21
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 70.24 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.92 (ii) 9.56 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.12

PEAK FLOW (cms)= 0.17 0.05 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.221 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 82.95
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | STANHYD (10071) | Area (ha)= 0.72
ID= 1 DT= 5.0 min | Total Imp (%) = 60.00 Dir. Conn. (%) = 60.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.43 0.29
Dep. Storage (mm)= 1.00 1.00

Average Slope (%) = 1.00 2.00
Length (m) = 69.28 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr) = 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min) = 1.91 (ii) 9.54 (iii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.12

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10081)
Area (ha) = 0.21
Total Imp (%) = 52.00 Dir. Conn. (%) = 52.00
Surface Area (ha) = 0.11 0.10
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 4.00 2.00
Length (m) = 37.42 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr) = 119.72 102.72
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.32 (ii) 4.67 (iii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.33 0.22

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10092)
Area (ha) = 0.63
Total Imp (%) = 81.00 Dir. Conn. (%) = 81.00
Surface Area (ha) = 0.51 0.12
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 64.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 1.167 hours.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr) = 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min) = 1.32 (ii) 8.95 (iii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.33 0.12

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10082)
Area (ha) = 0.21
Total Imp (%) = 87.00 Dir. Conn. (%) = 87.00
Surface Area (ha) = 0.18 0.03
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 37.42 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr) = 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min) = 1.83 (ii) 5.85 (iii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.15

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10091)
Area (ha) = 0.50
Total Imp (%) = 74.00 Dir. Conn. (%) = 74.00
Surface Area (ha) = 0.37 0.13
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 57.74 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 1.833 hours.

1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.72	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.72	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.72	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.72	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.72	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.72	6.000	119.72	9.000	3.17	12.00	1.81

Max.Eff.Inten.(mm/hr)= 119.72 102.72
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.71 (ii) 6.43 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.14
PEAK FLOW (cms)= 0.12 0.03 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.155 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 83.65
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 0.91
STANDHYD (1101)	Total Imp(%)= 55.00
ID= 1 DT= 5.0 min	Dir. Conn.(%)= 55.00

Surface Area (ha)= 0.50	PERVIOUS (i) 0.41
Dep. Storage (mm)= 1.00	1.00
Average Slope (%)= 1.00	2.00
Length (m)= 77.89	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	3.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81

Storage Coeff. (min)= 5.00 10.00
Unit Hyd. Tpeak (min)= 2.03 (ii) 6.91 (iii)
Unit Hyd. peak (cms)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.14
PEAK FLOW (cms)= 0.21 0.06 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.270 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 83.19
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 1.06
STANDHYD (1111)	Total Imp(%)= 59.00
ID= 1 DT= 5.0 min	Dir. Conn.(%)= 59.00

Surface Area (ha)= 0.63	PERVIOUS (i) 0.43
Dep. Storage (mm)= 1.00	1.00
Average Slope (%)= 1.00	2.00
Length (m)= 84.06	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81

Max.Eff.Inten.(mm/hr)= 119.72 102.72
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.14 (ii) 9.77 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.11
PEAK FLOW (cms)= 0.21 0.09 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.302 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 80.16

2.583	2.72	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.72	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.72	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.72	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.72	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.72	6.000	119.72	9.000	3.17	12.00	1.81

Max.Eff.Inten.(mm/hr)= 119.72 102.72
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.05 (ii) 9.68 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.11
PEAK FLOW (cms)= 0.17 0.09 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.255 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 79.23
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 0.88
STANDHYD (1102)	Total Imp(%)= 72.00
ID= 1 DT= 5.0 min	Dir. Conn.(%)= 72.00

Surface Area (ha)= 0.63	PERVIOUS (i) 0.25
Dep. Storage (mm)= 1.00	1.00
Average Slope (%)= 1.00	2.00
Length (m)= 76.59	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.72	5.583	43.				

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1122)
ID= 1 DT= 5.0 min

Area (ha)= 1.22
Total Imp(%)= 69.00 Dir. Conn.(%)= 69.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.84 0.38
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 90.18 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17	9.58	3.17
0.667	2.27	3.667	3.63	6.667	16.33	9.67	3.17	9.67	3.17
0.750	2.27	3.750	3.63	6.750	16.33	9.75	3.17	9.75	3.17
0.833	2.27	3.833	3.63	6.833	16.33	9.83	3.17	9.83	3.17
0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17	10.00	3.17
1.083	2.27	4.083	3.63	7.083	16.33	10.08	1.81	10.08	1.81
1.167	2.27	4.167	3.63	7.167	16.33	10.17	1.81	10.17	1.81
1.250	2.27	4.250	3.63	7.250	16.33	10.25	1.81	10.25	1.81
1.333	2.27	4.333	3.63	7.333	16.33	10.33	1.81	10.33	1.81
1.417	2.27	4.417	3.63	7.417	16.33	10.42	1.81	10.42	1.81
1.500	2.27	4.500	3.63	7.500	16.33	10.50	1.81	10.50	1.81
1.583	2.27	4.583	3.63	7.583	16.33	10.58	1.81	10.58	1.81
1.667	2.27	4.667	3.63	7.667	16.33	10.67	1.81	10.67	1.81
1.750	2.27	4.750	3.63	7.750	16.33	10.75	1.81	10.75	1.81
1.833	2.27	4.833	3.63	7.833	16.33	10.83	1.81	10.83	1.81
1.917	2.27	4.917	3.63	7.917	16.33	10.92	1.81	10.92	1.81
2.000	2.27	5.000	3.63	8.000	16.33	11.00	1.81	11.00	1.81
2.083	2.27	5.083	3.63	8.083	16.33	11.08	1.81	11.08	1.81
2.167	2.27	5.167	3.63	8.167	16.33	11.17	1.81	11.17	1.81
2.250	2.27	5.250	3.63	8.250	16.33	11.25	1.81	11.25	1.81
2.333	2.27	5.333	3.63	8.333	16.33	11.33	1.81	11.33	1.81
2.417	2.27	5.417	3.63	8.417	16.33	11.42	1.81	11.42	1.81
2.500	2.27	5.500	3.63	8.500	16.33	11.50	1.81	11.50	1.81
2.583	2.27	5.583	3.63	8.583	16.33	11.58	1.81	11.58	1.81
2.667	2.27	5.667	3.63	8.667	16.33	11.67	1.81	11.67	1.81
2.750	2.27	5.750	3.63	8.750	16.33	11.75	1.81	11.75	1.81
2.833	2.27	5.833	3.63	8.833	16.33	11.83	1.81	11.83	1.81
2.917	2.27	5.917	3.63	8.917	16.33	11.92	1.81	11.92	1.81
3.000	2.27	6.000	3.63	9.000	16.33	12.00	1.81	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 2.23 (ii) 9.86 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.11

PEAK FLOW (cms)= 0.27 0.08
TIME TO PEAK (hrs)= 6.00 6.00
RUNOFF VOLUME (mm)= 89.70 66.45
TOTAL RAINFALL (mm)= 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN² = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.82 0.39
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 89.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17	9.58	3.17
0.667	2.27	3.667	3.63	6.667	16.33	9.67	3.17	9.67	3.17
0.750	2.27	3.750	3.63	6.750	16.33	9.75	3.17	9.75	3.17
0.833	2.27	3.833	3.63	6.833	16.33	9.83	3.17	9.83	3.17
0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17	10.00	3.17
1.083	2.27	4.083	3.63	7.083	16.33	10.08	1.81	10.08	1.81
1.167	2.27	4.167	3.63	7.167	16.33	10.17	1.81	10.17	1.81
1.250	2.27	4.250	3.63	7.250	16.33	10.25	1.81	10.25	1.81
1.333	2.27	4.333	3.63	7.333	16.33	10.33	1.81	10.33	1.81
1.417	2.27	4.417	3.63	7.417	16.33	10.42	1.81	10.42	1.81
1.500	2.27	4.500	3.63	7.500	16.33	10.50	1.81	10.50	1.81
1.583	2.27	4.583	3.63	7.583	16.33	10.58	1.81	10.58	1.81
1.667	2.27	4.667	3.63	7.667	16.33	10.67	1.81	10.67	1.81
1.750	2.27	4.750	3.63	7.750	16.33	10.75	1.81	10.75	1.81
1.833	2.27	4.833	3.63	7.833	16.33	10.83	1.81	10.83	1.81
1.917	2.27	4.917	3.63	7.917	16.33	10.92	1.81	10.92	1.81
2.000	2.27	5.000	3.63	8.000	16.33	11.00	1.81	11.00	1.81
2.083	2.27	5.083	3.63	8.083	16.33	11.08	1.81	11.08	1.81
2.167	2.27	5.167	3.63	8.167	16.33	11.17	1.81	11.17	1.81
2.250	2.27	5.250	3.63	8.250	16.33	11.25	1.81	11.25	1.81
2.333	2.27	5.333	3.63	8.333	16.33	11.33	1.81	11.33	1.81
2.417	2.27	5.417	3.63	8.417	16.33	11.42	1.81	11.42	1.81
2.500	2.27	5.500	3.63	8.500	16.33	11.50	1.81	11.50	1.81
2.583	2.27	5.583	3.63	8.583	16.33	11.58	1.81	11.58	1.81
2.667	2.27	5.667	3.63	8.667	16.33	11.67	1.81	11.67	1.81
2.750	2.27	5.750	3.63	8.750	16.33	11.75	1.81	11.75	1.81
2.833	2.27	5.833	3.63	8.833	16.33	11.83	1.81	11.83	1.81
2.917	2.27	5.917	3.63	8.917	16.33	11.92	1.81	11.92	1.81
3.000	2.27	6.000	3.63	9.000	16.33	12.00	1.81	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 2.23 (ii) 9.86 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.11

PEAK FLOW (cms)= 0.27 0.08
TIME TO PEAK (hrs)= 6.00 6.00
RUNOFF VOLUME (mm)= 89.70 66.45
TOTAL RAINFALL (mm)= 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN² = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN² = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1121)
ID= 1 DT= 5.0 min

Area (ha)= 1.21
Total Imp(%)= 68.00 Dir. Conn.(%)= 68.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.28 0.08
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 89.70 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****

Input filename: c:\Program Files (x86)\Visual OTTHYMO 6.1\VO2\vojn.dat
Output filename: c:\Users\caef070146\AppData\Local\civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\18f25aa3
Summary filename: c:\Users\caef070146\AppData\Local\civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\18f25aa3

DATE: 05-18-2022 TIME: 12:41:46

USER:

COMMENTS:

** SIMULATION : Run 17 **

READ STORM
Filename: c:\Users\caef070146\AppData\Local\Temp\1841f864-F36d-471c-a2fc-7a8c6cfa585\98941cd
Ptotal=101.70 mm
Comments: 50-Yr,12-hr SCS, City of Barrie

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.54	3.25	4.07	6.25	18.31	9.25	3.56
0.50	2.54	3.50	4.07	6.50	18.31	9.50	3.56
0.75	2.54	3.75	4.07	6.75	18.31	9.75	3.56
1.00	2.54	4.00	4.07	7.00	18.31	10.00	3.56
1.25	2.54	4.25	4.07	7.25	18.31	10.25	3.56
1.50	2.54	4.50	4.07	7.50	18.31	10.50	3.56
1.75	2.54	4.75	4.07	7.75	18.31	10.75	3.56
2.00	2.54	5.00	4.07	8.00	18.31	11.00	3.56
2.25	3.05	5.25	4.07	8.25	18.31	11.25	3.56
2.50	3.05	5.50	4.07	8.50	18.31	11.50	3.56
2.75	3.05	5.75	4.07	8.75	18.31	11.75	3.56
3.00	3.05	6.00	4.07	9.00	18.31	12.00	3.56

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	4.07	7.083	18.31	10.08	3.56
1.167	2.54	4.167	4.07	7.167	18.31	10.17	3.56
1.250	2.54	4.250	4.07	7.250	18.31	10.25	3.56
1.333	2.54	4.333	4.07	7.333	18.31	10.33	3.56
1.417	2.54	4.417	4.07	7.417	18.31	10.42	3.56
1.500	2.54	4.500	4.07	7.500	18.31	10.50	3.56
1.583	2.54	4.583	4.07	7.583	18.31	10.58	3.56
1.667	2.5						

2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max. Eff. Inten. (mm/hr)= 134.24 over (min)= 5.00
Storage Coeff. (min)= 1.90 (ii) 6.32 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.23 0.06 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 93.75
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10021) Area (ha)= 0.73 Total Imp (%) = 71.00 Dir. Conn. (%) = 71.00

Surface Area (ha)= 0.52 IMPERVIOUS 0.21 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 69.76 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07	6.083	18.31
0.167	2.54	3.167	4.07	6.167	18.31
0.250	2.54	3.250	4.07	6.250	18.31
0.333	2.54	3.333	4.07	6.333	18.31
0.417	2.54	3.417	4.07	6.417	18.31
0.500	2.54	3.500	4.07	6.500	18.31
0.583	2.54	3.583	4.07	6.583	18.31
0.667	2.54	3.667	4.07	6.667	18.31
0.750	2.54	3.750	4.07	6.750	18.31
0.833	2.54	3.833	4.07	6.833	18.31
0.917	2.54	3.917	4.07	6.917	18.31
1.000	2.54	4.000	4.07	7.000	18.31
1.083	2.54	4.083	4.07	7.083	18.31
1.167	2.54	4.167	4.07	7.167	18.31
1.250	2.54	4.250	4.07	7.250	18.31
1.333	2.54	4.333	4.07	7.333	18.31
1.417	2.54	4.417	4.07	7.417	18.31
1.500	2.54	4.500	4.07	7.500	18.31
1.583	2.54	4.583	4.07	7.583	18.31
1.667	2.54	4.667	4.07	7.667	18.31
1.750	2.54	4.750	4.07	7.750	18.31
1.833	2.54	4.833	4.07	7.833	18.31
1.917	2.54	4.917	4.07	7.917	18.31
2.000	2.54	5.000	4.07	8.000	18.31
2.083	3.05	5.083	12.20	8.083	3.56
2.167	3.05	5.167	12.20	8.167	3.56
2.250	3.05	5.250	12.20	8.250	3.56
2.333	3.05	5.333	12.20	8.333	3.56
2.417	3.05	5.417	12.20	8.417	3.56
2.500	3.05	5.500	12.20	8.500	3.56
2.583	3.05	5.583	48.82	8.583	3.56

2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max. Eff. Inten. (mm/hr)= 134.24 over (min)= 5.00
Storage Coeff. (min)= 1.83 (ii) 6.62 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.32 0.14

PEAK FLOW (cms)= 0.19 0.06 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 93.75
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10031) Area (ha)= 0.38 Total Imp (%) = 66.00 Dir. Conn. (%) = 66.00

Surface Area (ha)= 0.25 IMPERVIOUS 0.11 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07	6.083	18.31
0.167	2.54	3.167	4.07	6.167	18.31
0.250	2.54	3.250	4.07	6.250	18.31
0.333	2.54	3.333	4.07	6.333	18.31
0.417	2.54	3.417	4.07	6.417	18.31
0.500	2.54	3.500	4.07	6.500	18.31
0.583	2.54	3.583	4.07	6.583	18.31
0.667	2.54	3.667	4.07	6.667	18.31
0.750	2.54	3.750	4.07	6.750	18.31
0.833	2.54	3.833	4.07	6.833	18.31
0.917	2.54	3.917	4.07	6.917	18.31
1.000	2.54	4.000	4.07	7.000	18.31
1.083	2.54	4.083	4.07	7.083	18.31
1.167	2.54	4.167	4.07	7.167	18.31
1.250	2.54	4.250	4.07	7.250	18.31
1.333	2.54	4.333	4.07	7.333	18.31
1.417	2.54	4.417	4.07	7.417	18.31
1.500	2.54	4.500	4.07	7.500	18.31
1.583	2.54	4.583	4.07	7.583	18.31
1.667	2.54	4.667	4.07	7.667	18.31
1.750	2.54	4.750	4.07	7.750	18.31
1.833	2.54	4.833	4.07	7.833	18.31
1.917	2.54	4.917	4.07	7.917	18.31
2.000	2.54	5.000	4.07	8.000	18.31
2.083	3.05	5.083	12.20	8.083	3.56
2.167	3.05	5.167	12.20	8.167	3.56
2.250	3.05	5.250	12.20	8.250	3.56
2.333	3.05	5.333	12.20	8.333	3.56
2.417	3.05	5.417	12.20	8.417	3.56
2.500	3.05	5.500	12.20	8.500	3.56
2.583	3.05	5.583	48.82	8.583	3.56
2.667	3.05	5.667	48.82	8.667	3.56
2.750	3.05	5.750	48.82	8.750	3.56
2.833	3.05	5.833	134.24	8.833	3.56
2.917	3.05	5.917	134.24	8.917	3.56
3.000	3.05	6.000	134.24	9.000	3.56

Max. Eff. Inten. (mm/hr)= 134.24 over (min)= 5.00
Storage Coeff. (min)= 1.47 (ii) 6.45 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.33 0.14

RUNOFF COEFFICIENT = 0.99 0.75 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10041) Area (ha)= 0.35 Total Imp (%) = 69.00 Dir. Conn. (%) = 69.00

Surface Area (ha)= 0.24 IMPERVIOUS 0.11 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.30 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07	6.083	18.31
0.167	2.54	3.167	4.07	6.167	18.31
0.250	2.54	3.250	4.07	6.250	18.31
0.333	2.54	3.333	4.07	6.333	18.31
0.417	2.54	3.417	4.07	6.417	18.31
0.500	2.54	3.500	4.07	6.500	18.31
0.583	2.54	3.583	4.07	6.583	18.31
0.667	2.54	3.667	4.07	6.667	18.31
0.750	2.54	3.750	4.07	6.750	18.31
0.833	2.54	3.833	4.07	6.833	18.31
0.917	2.54	3.917	4.07	6.917	18.31
1.000	2.54	4.000	4.07	7.000	18.31
1.083	2.54	4.083	4.07	7.083	18.31
1.167	2.54	4.167	4.07	7.167	18.31
1.250	2.54	4.250	4.07	7.250	18.31
1.333	2.54	4.333	4.07	7.333	18.31
1.417	2.54	4.417	4.07	7.417	18.31
1.500	2.54	4.500	4.07	7.500	18.31
1.583	2.54	4.583	4.07	7.583	18.31
1.667	2.54	4.667	4.07	7.667	18.31
1.750	2.54	4.750	4.07	7.750	18.31
1.833	2.54	4.833	4.07	7.833	18.31
1.917	2.54	4.917	4.07	7.917	18.31
2.000	2.54	5.000	4.07	8.000	18.31
2.083	3.05	5.083	12.20	8.083	3.56
2.167	3.05	5.167	12.20	8.167	3.56
2.250	3.05	5.250	12.20	8.250	3.56
2.333	3.05	5.333	12.20	8.333	3.56
2.417	3.05	5.417	12.20	8.417	3.56
2.500	3.05	5.500	12.20	8.500	3.56
2.583	3.05	5.583	48.82	8.583	3.56
2.667	3.05	5.667	48.82	8.667	3.56
2.750	3.05	5.750	48.82	8.750	3.56
2.833	3.05	5.833	134.24	8.833	3.56
2.917	3.05	5.917	134.24	8.917	3.56
3.000	3.05	6.000	134.24	9.000	3.56

Max. Eff. Inten. (mm/hr)= 134.24 over (min)= 5.00
Storage Coeff. (min)= 1.47 (ii) 6.45 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.33 0.14

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

Storage Coeff. (min)= 1.50 (ii) 8.73 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.33 0.12

PEAK FLOW (cms)= 0.09 0.03 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 93.75
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (10032) Area (ha)= 0.39 Total Imp (%) = 79.00 Dir. Conn. (%) = 79.00

Surface Area (ha)= 0.31 IMPERVIOUS 0.08 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07	6.083	18.31
0.167	2.54	3.167	4.07	6.167	18.31
0.250	2.54	3.250	4.07	6.250	18.31
0.333	2.54	3.333	4.07	6.333	18.31
0.417	2.54	3.417	4.07	6.417	18.31
0.500	2.54	3.500	4.07	6.500	18.31
0.583	2.54	3.583	4.07	6.583	18.31
0.667	2.54	3.667	4.07	6.667	18.31
0.750	2.54	3.750	4.07	6.750	18.31
0.833	2.54	3.833	4.07	6.833	18.31
0.917	2.54	3.917	4.07	6.917	18.31
1.000	2.54	4.000	4.07	7.000	18.31
1.083	2.54	4.083	4.07	7.083	18.31
1.167	2.54	4.167	4.07	7.167	18.31
1.250	2.54	4.250	4.07	7.250	18.31

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 2 columns: Parameter and Value. Includes CALIB, STANDHYD (10042), ID=1 DT=5.0 min, Area (ha)=0.38, Total Imp(%)=74.00, Dir. Conn.(%)=74.00.

Table with 2 columns: Parameter and Value. Includes IMPERVIOUS, PERVIOUS (i), Surface Area (ha)=0.28, Dep. Storage (mm)=1.00, Average Slope (%)=1.00, Length (m)=50.33, Mannings n=0.290.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Includes summary statistics like Max. Eff. Inten., Storage Coeff., etc.

Summary statistics table for the first hyetograph, including Max. Eff. Inten., Storage Coeff., Unit Hyd. Tpeak, and PEAK FLOW.

Summary statistics table for the first hyetograph, including TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, and RUNOFF COEFFICIENT.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 2 columns: Parameter and Value. Includes CALIB, STANDHYD (10012), ID=1 DT=5.0 min, Area (ha)=0.23, Total Imp(%)=78.00, Dir. Conn.(%)=78.00.

Table with 2 columns: Parameter and Value. Includes IMPERVIOUS, PERVIOUS (i), Surface Area (ha)=0.23, Dep. Storage (mm)=1.00, Average Slope (%)=1.00, Length (m)=49.67, Mannings n=0.290.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table for the second hyetograph, including summary statistics.

Summary statistics table for the second hyetograph.

Summary statistics table for the second hyetograph.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 2 columns: Parameter and Value. Includes CALIB, STANDHYD (10052), ID=1 DT=5.0 min, Area (ha)=0.36, Total Imp(%)=62.00, Dir. Conn.(%)=62.00.

Table with 2 columns: Parameter and Value. Includes IMPERVIOUS, PERVIOUS (i), Surface Area (ha)=0.22, Dep. Storage (mm)=1.00, Average Slope (%)=1.00, Length (m)=48.99, Mannings n=0.290.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table for the third hyetograph, including summary statistics.

Summary statistics table for the third hyetograph.

Table with 2 columns: Parameter and Value. Includes Surface Area (ha)=0.18, Dep. Storage (mm)=1.00, Average Slope (%)=1.00, Length (m)=39.16, Mannings n=0.290.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table for the fourth hyetograph, including summary statistics.

Summary statistics table for the fourth hyetograph.

Summary statistics table for the fourth hyetograph.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 2 columns: Parameter and Value. Includes CALIB, STANDHYD (10051), ID=1 DT=5.0 min, Area (ha)=0.37, Total Imp(%)=66.00, Dir. Conn.(%)=66.00.

Table with 2 columns: Parameter and Value. Includes IMPERVIOUS, PERVIOUS (i), Surface Area (ha)=0.24, Dep. Storage (mm)=1.00, Average Slope (%)=1.00, Length (m)=49.67, Mannings n=0.290.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table for the fifth hyetograph, including summary statistics.

Summary statistics table for the fifth hyetograph.

Summary statistics table for the fifth hyetograph.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 2 columns: Parameter and Value. Includes CALIB, STANDHYD (10062), ID=1 DT=5.0 min, Area (ha)=0.66, Total Imp(%)=62.00, Dir. Conn.(%)=62.00.

Table with 2 columns: Parameter and Value. Includes IMPERVIOUS, PERVIOUS (i), Surface Area (ha)=0.41, Dep. Storage (mm)=1.00, Average Slope (%)=1.00, Length (m)=66.33, Mannings n=0.290.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table for the sixth hyetograph, including summary statistics.

Summary statistics table for the sixth hyetograph.

1.000	2.54	4.000	4.07	7.000	8.14	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	6.10	7.583	6.10	10.58	2.03
1.667	2.54	4.667	6.10	7.667	6.10	10.67	2.03
1.750	2.54	4.750	6.10	7.750	6.10	10.75	2.03
1.833	2.54	4.833	6.10	7.833	6.10	10.83	2.03
1.917	2.54	4.917	6.10	7.917	6.10	10.92	2.03
2.000	2.54	5.000	6.10	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.78 (ii) 9.00 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.15 0.06 *TOTALS*
over (min)= 5.00 10.00 0.217 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 91.60
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (10061)	Area (ha)= 0.65
ID= 1 DT= 5.0 min	Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	= 0.42	0.23	
Dep. Storage (mm)	= 1.00	1.00	
Average Slope (%)	= 1.00	2.00	
Length (m)	= 65.83	40.00	
Mannings n	= 0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	6.10	7.583	6.10	10.58	2.03

2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.84 (ii) 9.05 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.14

PEAK FLOW (cms)= 0.20 0.06 *TOTALS*
over (min)= 5.00 10.00 0.235 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 93.75
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (10071)	Area (ha)= 0.72
ID= 1 DT= 5.0 min	Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	= 0.43	0.29	
Dep. Storage (mm)	= 1.00	1.00	
Average Slope (%)	= 1.00	2.00	
Length (m)	= 69.28	40.00	
Mannings n	= 0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	6.10	7.583	6.10	10.58	2.03
1.667	2.54	4.667	6.10	7.667	6.10	10.67	2.03
1.750	2.54	4.750	6.10	7.750	6.10	10.75	2.03
1.833	2.54	4.833	6.10	7.833	6.10	10.83	2.03
1.917	2.54	4.917	6.10	7.917	6.10	10.92	2.03
2.000	2.54	5.000	6.10	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03

1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.77 (ii) 8.99 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.16 0.06 *TOTALS*
over (min)= 5.00 10.00 0.216 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 93.75
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (10072)	Area (ha)= 0.74
ID= 1 DT= 5.0 min	Total Imp(%)= 71.00 Dir. Conn.(%)= 71.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	= 0.53	0.21	
Dep. Storage (mm)	= 1.00	1.00	
Average Slope (%)	= 1.00	2.00	
Length (m)	= 70.24	40.00	
Mannings n	= 0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750			

PEAK FLOW (cms)= 0.04 0.03 0.067 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 100.70 76.77 89.19
 TOTAL RAINFALL (mm)= 101.70 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (10082)
 ID= 1 DT= 5.0 min Area (ha)= 0.21
 Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.18	0.03
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	37.42	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	4.07	7.083	18.31	10.08	2.03
1.167	2.54	4.167	4.07	7.167	18.31	10.17	2.03
1.250	2.54	4.250	4.07	7.250	18.31	10.25	2.03
1.333	2.54	4.333	4.07	7.333	18.31	10.33	2.03
1.417	2.54	4.417	4.07	7.417	18.31	10.42	2.03
1.500	2.54	4.500	4.07	7.500	18.31	10.50	2.03
1.583	2.54	4.583	4.07	7.583	18.31	10.58	2.03
1.667	2.54	4.667	4.07	7.667	18.31	10.67	2.03
1.750	2.54	4.750	4.07	7.750	18.31	10.75	2.03
1.833	2.54	4.833	4.07	7.833	18.31	10.83	2.03
1.917	2.54	4.917	4.07	7.917	18.31	10.92	2.03
2.000	2.54	5.000	4.07	8.000	18.31	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
 over (min)= 5.00 10.00
 Storage Coeff.(min)= 1.26 (ii) 4.46 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.23
 PEAK FLOW (cms)= 0.07 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.077 (iii)
 RUNOFF VOLUME (mm)= 100.70 76.77 6.00
 TOTAL RAINFALL (mm)= 101.70 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

CALIB STANHYD (10091)
 ID= 1 DT= 5.0 min Area (ha)= 0.50
 Total Imp(%)= 74.00 Dir. Conn.(%)= 74.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.17	0.33
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	57.74	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	4.07	7.083	18.31	10.08	2.03
1.167	2.54	4.167	4.07	7.167	18.31	10.17	2.03
1.250	2.54	4.250	4.07	7.250	18.31	10.25	2.03
1.333	2.54	4.333	4.07	7.333	18.31	10.33	2.03
1.417	2.54	4.417	4.07	7.417	18.31	10.42	2.03
1.500	2.54	4.500	4.07	7.500	18.31	10.50	2.03
1.583	2.54	4.583	4.07	7.583	18.31	10.58	2.03
1.667	2.54	4.667	4.07	7.667	18.31	10.67	2.03
1.750	2.54	4.750	4.07	7.750	18.31	10.75	2.03
1.833	2.54	4.833	4.07	7.833	18.31	10.83	2.03
1.917	2.54	4.917	4.07	7.917	18.31	10.92	2.03
2.000	2.54	5.000	4.07	8.000	18.31	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
 over (min)= 5.00 10.00
 Storage Coeff.(min)= 1.63 (ii) 6.15 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.15
 PEAK FLOW (cms)= 0.14 0.04 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.175 (iii)
 RUNOFF VOLUME (mm)= 100.70 76.77 94.47
 TOTAL RAINFALL (mm)= 101.70 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (1101)
 ID= 1 DT= 5.0 min Area (ha)= 0.91
 Total Imp(%)= 55.00 Dir. Conn.(%)= 55.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.50	0.41
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (10092)
 ID= 1 DT= 5.0 min Area (ha)= 0.63
 Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.51	0.12
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	64.81	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	4.07	7.083	18.31	10.08	2.03
1.167	2.54	4.167	4.07	7.167	18.31	10.17	2.03
1.250	2.54	4.250	4.07	7.250	18.31	10.25	2.03
1.333	2.54	4.333	4.07	7.333	18.31	10.33	2.03
1.417	2.54	4.417	4.07	7.417	18.31	10.42	2.03
1.500	2.54	4.500	4.07	7.500	18.31	10.50	2.03
1.583	2.54	4.583	4.07	7.583	18.31	10.58	2.03
1.667	2.54	4.667	4.07	7.667	18.31	10.67	2.03
1.750	2.54	4.750	4.07	7.750	18.31	10.75	2.03
1.833	2.54	4.833	4.07	7.833	18.31	10.83	2.03
1.917	2.54	4.917	4.07	7.917	18.31	10.92	2.03
2.000	2.54	5.000	4.07	8.000	18.31	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 1

hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	6.10	10.58	2.03
1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max. Eff. Inten. (mm/hr)=	134.24	117.80	
over (min)	5.00	10.00	
Storage Coeff (min)=	1.94 (ii)	6.63 (ii)	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)=	0.31	0.14	
PEAK FLOW (cms)=	0.24	0.07	*TOTALS* 0.305 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	
RUNOFF VOLUME (mm)=	100.70	76.77	94.00
TOTAL RAINFALL (mm)=	101.70	101.70	101.70
RUNOFF COEFFICIENT =	0.99	0.75	0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN² = 89.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (1111)	Area (ha)= 1.06	Dir. Conn.(%)= 59.00
ID= 1 DT= 5.0 min	Total Imp(%)= 59.00	
Surface Area (ha)=	0.63	PERVIOUS (i) 0.43
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	84.06	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56

1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	6.10	10.58	2.03
1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max. Eff. Inten. (mm/hr)=	134.24	117.80	
over (min)	5.00	10.00	
Storage Coeff (min)=	1.99 (ii)	6.13 (ii)	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)=	0.31	0.15	
PEAK FLOW (cms)=	0.28	0.06	*TOTALS* 0.343 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	
RUNOFF VOLUME (mm)=	100.70	76.77	95.43
TOTAL RAINFALL (mm)=	101.70	101.70	101.70
RUNOFF COEFFICIENT =	0.99	0.75	0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN² = 89.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (1122)	Area (ha)= 1.22	Dir. Conn.(%)= 69.00
ID= 1 DT= 5.0 min	Total Imp(%)= 69.00	
Surface Area (ha)=	0.84	PERVIOUS (i) 0.38
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	90.18	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	6.10	10.58	2.03
1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03

0.667	2.54	3.667	4.07	6.667	8.14	9.67	3.56
0.750	2.54	3.750	4.07	6.750	8.14	9.75	3.56
0.833	2.54	3.833	4.07	6.833	8.14	9.83	3.56
0.917	2.54	3.917	4.07	6.917	8.14	9.92	3.56
1.000	2.54	4.000	4.07	7.000	8.14	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	6.10	10.58	2.03
1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.						

2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max. Eff. Inten. (mm/hr) = 134.24 over (min) = 5.00
 Storage Coeff. (min) = 2.13 (ii) 9.35 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.31 0.12
 TOTALS
 PEAK FLOW (cms) = 0.31 0.10 0.405 (iii)
 TIME TO PEAK (hrs) = 6.00 6.00 6.00
 RUNOFF VOLUME (mm) = 100.70 76.77 93.04
 TOTAL RAINFALL (mm) = 101.70 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

V	V	I	SSSS	U	U	A	L	(v 6.1.2002)	
V	V	I	SS	U	U	A	A	L	
V	V	I	SS	U	U	A	A	A	L
V	V	I	SS	U	U	A	A	L	
V	V	I	SSSS	UUUU	A	A	LLLL		

OOO TTTT TTTT H H Y Y M M O O O TM
 O O T T H H Y Y M M O O O
 O O T T H H Y Y M M O O O
 Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2020 Smart City Water Inc
 All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHM 6.1\VO2\voindat
 Output filename: C:\Users\caef070146\AppData\Local\Civica\WHS\50564c49-8c07-4086-8157-8729f3caba3\710b9e08
 Summary filename: C:\Users\caef070146\AppData\Local\Civica\WHS\50564c49-8c07-4086-8157-8729f3caba3\710b9e08

DATE: 05-18-2022 TIME: 12:41:46
 USER:

COMMENTS:

***** SIMULATION : Run 18 *****
 READ STORM File: C:\Users\caef070146\AppData\Local\Civica\WHS\50564c49-8c07-4086-8157-8729f3caba3\710b9e08
 Ptotal=112.50 mm
 Comments: 100-yr, 12-hr SCS, city of Barrie

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.81	3.25	4.50	6.25	20.25	9.25	3.94
0.50	2.81	4.50	6.00	9.25	20.25	9.50	3.94
0.75	2.81	3.75	4.50	6.75	9.00	9.75	3.94
1.00	2.81	4.00	4.50	7.00	9.00	10.00	3.94
1.25	2.81	4.25	6.75	7.25	6.75	10.25	2.25
1.50	2.81	4.50	6.75	7.50	6.75	10.50	2.25
1.75	2.81	4.75	9.00	7.75	6.75	10.75	2.25
2.00	2.81	5.00	9.00	8.00	6.75	11.00	2.25
2.25	3.37	5.25	13.50	8.25	3.94	11.25	2.25

[STANDHYD (10022)] Area (ha) = 0.83 Dir. Conn. (%) = 75.00
 ID= 1 DT= 5.0 min Total Imp (%) = 75.00

Surface Area (ha) = 0.62 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 74.39 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
 Storage Coeff. (min) = 1.83 (ii) 6.07 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.32 0.15
 TOTALS
 PEAK FLOW (cms) = 0.26 0.07 0.324 (iii)
 TIME TO PEAK (hrs) = 6.00 6.00 6.00
 RUNOFF VOLUME (mm) = 111.50 87.00 105.37
 TOTAL RAINFALL (mm) = 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.99 0.77 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

[CALIB STANDHYD (10021)] Area (ha) = 0.73 Dir. Conn. (%) = 71.00
 ID= 1 DT= 5.0 min Total Imp (%) = 71.00

Surface Area (ha) = 0.52 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 69.76 40.00

2.50	3.37	5.50	13.50	8.50	3.94	11.50	2.25
2.75	3.37	5.75	54.00	8.75	3.94	11.75	2.25
3.00	3.37	6.00	148.50	9.00	3.94	12.00	2.25

[STANDHYD (10011)] Area (ha) = 0.21 Dir. Conn. (%) = 79.00
 ID= 1 DT= 5.0 min Total Imp (%) = 79.00

Surface Area (ha) = 0.17 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 37.42 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
 Storage Coeff. (min) = 1.21 (ii) 5.09 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.33 0.16
 TOTALS
 PEAK FLOW (cms) = 0.07 0.01 0.083 (iii)
 TIME TO PEAK (hrs) = 6.00 6.00 6.00
 RUNOFF VOLUME (mm) = 111.50 87.00 106.34
 TOTAL RAINFALL (mm) = 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.99 0.77 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

[CALIB STANDHYD (10031)] Area (ha) = 0.38 Dir. Conn. (%) = 66.00
 ID= 1 DT= 5.0 min Total Imp (%) = 66.00

Surface Area (ha) = 0.25 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 50.33 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250					

Hydrology data table with columns for time, rain, and flow. Includes a 'TRANSFORMED HYETOGRAPH' section.

Max. Eff. Inten. (mm/hr) = 148.50
Storage Coeff. (min) = 1.44 (ii)
Unit Hyd. Tpeak (min) = 5.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CALIB STANDHYD (10032) ID= 1 DT= 5.0 min Area (ha) = 0.39 Total Imp (%) = 79.00 Dir. Conn. (%) = 79.00

Surface Area (ha) = 0.31
Dep. Storage (mm) = 1.00
Average Slope (%) = 50.99
Length (m) = 0.013
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns for TIME, RAIN, and HRS.

Hydrology data table with columns for time, rain, and flow. Includes a 'TRANSFORMED HYETOGRAPH' section.

Max. Eff. Inten. (mm/hr) = 148.50
Storage Coeff. (min) = 1.44 (ii)
Unit Hyd. Tpeak (min) = 5.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CALIB STANDHYD (10042) ID= 1 DT= 5.0 min Area (ha) = 0.38 Total Imp (%) = 74.00 Dir. Conn. (%) = 74.00

Surface Area (ha) = 0.28
Dep. Storage (mm) = 1.00
Average Slope (%) = 50.33
Length (m) = 0.013
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns for TIME, RAIN, and HRS.

Hydrology data table with columns for time, rain, and flow. Includes a 'TRANSFORMED HYETOGRAPH' section.

Max. Eff. Inten. (mm/hr) = 148.50
Storage Coeff. (min) = 1.46 (ii)
Unit Hyd. Tpeak (min) = 5.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CALIB STANDHYD (10041) ID= 1 DT= 5.0 min Area (ha) = 0.35 Total Imp (%) = 69.00 Dir. Conn. (%) = 69.00

Surface Area (ha) = 0.24
Dep. Storage (mm) = 1.00
Average Slope (%) = 48.30
Length (m) = 0.013
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns for TIME, RAIN, and HRS.

Hydrology data table with columns for time, rain, and flow. Includes a 'TRANSFORMED HYETOGRAPH' section.

Max. Eff. Inten. (mm/hr) = 148.50
Storage Coeff. (min) = 1.44 (ii)
Unit Hyd. Tpeak (min) = 5.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CALIB STANDHYD (10012) ID= 1 DT= 5.0 min Area (ha) = 0.23 Total Imp (%) = 78.00 Dir. Conn. (%) = 78.00

Surface Area (ha) = 0.18
Dep. Storage (mm) = 1.00
Average Slope (%) = 39.16
Length (m) = 0.013
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns for TIME, RAIN, and HRS.

2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
 over (min) 5.00 10.00
 Storage Coeff. (min)= 1.24 (ii) 5.20 (iii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.07 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 111.50 87.00 106.09
 TOTAL RAINFALL (mm)= 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.99 0.77 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10051)
 ID= 1 DT= 5.0 min Area (ha)= 0.37 Dir. Conn.(%)= 66.00
 Total Imp(%)= 66.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
0.24	0.13	0.11
1.00	1.00	1.00
1.00	1.00	2.00
49.67	40.00	40.00
0.013	0.290	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
 over (min) 5.00 10.00
 Storage Coeff. (min)= 1.43 (ii) 8.32 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10062)
 ID= 1 DT= 5.0 min Area (ha)= 0.66 Dir. Conn.(%)= 62.00
 Total Imp(%)= 62.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
0.41	0.23	0.21
1.00	1.00	1.00
1.00	1.00	2.00
66.33	40.00	40.00
0.013	0.290	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
 over (min) 5.00 10.00
 Storage Coeff. (min)= 1.70 (ii) 8.59 (iii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.12

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10072)
 ID= 1 DT= 5.0 min Area (ha)= 0.74 Dir. Conn.(%)= 71.00
 Total Imp(%)= 71.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
0.17	0.07	0.241 (iii)
1.00	1.00	1.00
1.00	1.00	6.00
111.50	87.00	102.18
112.50	112.50	112.50
0.99	0.77	0.91

Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.13

PEAK FLOW (cms)= 0.10 0.04 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 111.50 87.00 103.16
 TOTAL RAINFALL (mm)= 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.99 0.77 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (10052)
 ID= 1 DT= 5.0 min Area (ha)= 0.36 Dir. Conn.(%)= 62.00
 Total Imp(%)= 62.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
0.22	0.14	0.11
1.00	1.00	1.00
1.00	1.00	2.00
48.99	40.00	40.00
0.013	0.290	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25

Table with 10 columns of numerical data, likely representing rainfall intensity and duration for various scenarios.

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
Storage Coeff. (min) = 1.68 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.21
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 111.50
TOTAL RAINFALL (mm) = 112.50
RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)

CALIB STANDBYD (10091) Area (ha) = 0.50
Total Imp (%) = 74.00 Dir. Conn. (%) = 74.00
Surface Area (ha) = 0.50
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 57.74
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.
--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Contains transformed hyetograph data.

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
Storage Coeff. (min) = 1.88 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.21
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 111.50
TOTAL RAINFALL (mm) = 112.50
RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)

CALIB STANDBYD (1102) Area (ha) = 0.88
Total Imp (%) = 72.00 Dir. Conn. (%) = 72.00
Surface Area (ha) = 0.63
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 76.59
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.
--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Contains transformed hyetograph data.

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
Storage Coeff. (min) = 1.86 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.26
TOTAL RAINFALL (mm) = 112.50
RUNOFF COEFFICIENT = 0.99

Table with 10 columns of numerical data, similar to the first page.

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
Storage Coeff. (min) = 1.57 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.33

PEAK FLOW (cms) = 0.15
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 111.50
TOTAL RAINFALL (mm) = 112.50
RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)

CALIB STANDBYD (1101) Area (ha) = 0.91
Total Imp (%) = 55.00 Dir. Conn. (%) = 55.00
Surface Area (ha) = 0.50
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 77.89
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.
--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Contains transformed hyetograph data.

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
Storage Coeff. (min) = 1.88 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.21
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 111.50
TOTAL RAINFALL (mm) = 112.50
RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)

CALIB STANDBYD (1111) Area (ha) = 1.06
Total Imp (%) = 59.00 Dir. Conn. (%) = 59.00
Surface Area (ha) = 0.63
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 84.06
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.
--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Contains transformed hyetograph data.

Max. Eff. Inten. (mm/hr) = 148.50 over (min) = 5.00
Storage Coeff. (min) = 1.97 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.26
TOTAL RAINFALL (mm) = 112.50
RUNOFF COEFFICIENT = 0.99

CN^e = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1112) Area (ha)= 0.97
ID= 1 DT= 5.0 min Total Imp (%) = 78.00 Dir. Conn.(%) = 78.00

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)= 0.76	0.21	
Dep. Storage (mm)= 1.00	1.00	
Average Slope (%)= 1.00	2.00	
Length (m)= 80.42	40.00	
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	4.50	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.91 (ii) 5.88 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.15
PEAK FLOW (cms)= 0.31 0.12 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.382 (iii)
RUNOFF VOLUME (mm)= 111.50 87.00 106.11
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
CN^e = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	4.50	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 10.00
Storage Coeff. (min)= 2.04 (ii) 6.92 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.14
PEAK FLOW (cms)= 0.34 0.12 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.461 (iii)
RUNOFF VOLUME (mm)= 111.50 87.00 103.66
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
CN^e = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

FINISH

STANDHYD (1122) Area (ha)= 1.22
ID= 1 DT= 5.0 min Total Imp (%) = 69.00 Dir. Conn.(%) = 69.00

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)= 0.84	0.38	
Dep. Storage (mm)= 1.00	1.00	
Average Slope (%)= 1.00	2.00	
Length (m)= 90.18	40.00	
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

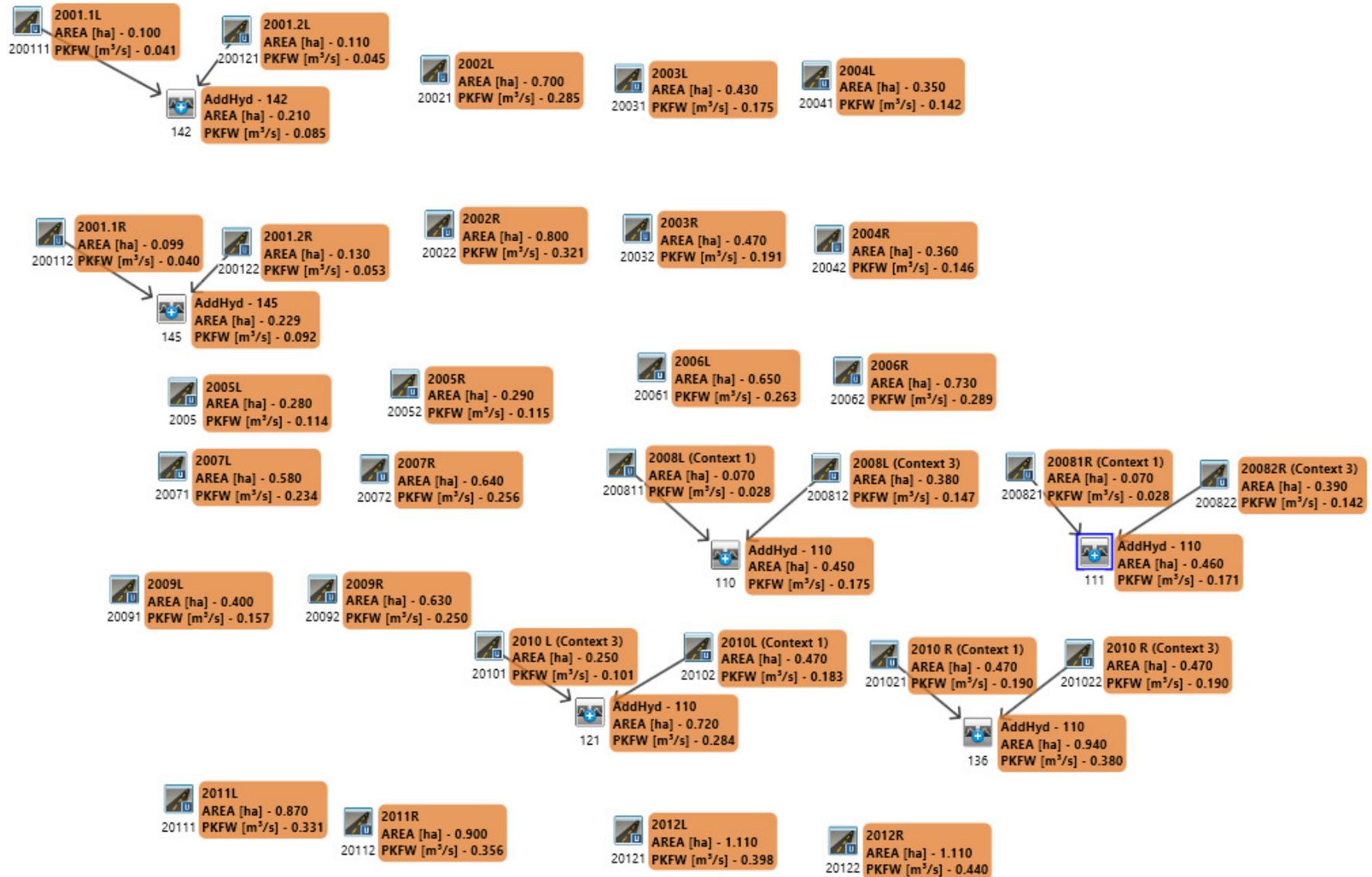
Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 10.00
Storage Coeff. (min)= 2.05 (ii) 6.83 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.14 *TOTALS*
PEAK FLOW (cms)= 0.35 0.12 0.466 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 103.90
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
CN^e = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1121) Area (ha)= 1.21
ID= 1 DT= 5.0 min Total Imp (%) = 68.00 Dir. Conn.(%) = 68.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 0.82	0.39
Dep. Storage (mm)= 1.00	1.00
Average Slope (%)= 1.00	2.00
Length (m)= 89.81	40.00

100-yr Post-developmet Roadway Drainage Area Flows (12-hr SCS II)



V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U A A L
W I SSSSS UUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\visual OTTHMO 6.1\VO2\voin.dat
Output filename: C:\Users\caef070146\AppData\Local\Civica\VH5\30564c49-8c07-4086-8157-8729fb3caba3\4f62ffc
Summary filename: C:\Users\caef070146\AppData\Local\Civica\VH5\30564c49-8c07-4086-8157-8729fb3caba3\4f62ffc

DATE: 05-18-2022 TIME: 12:50:56

USER:

COMMENTS: _____

***** SIMULATION *****

READ STORM Filename: C:\Users\caef070146\AppData\Local\Temp\3388b910-d519-48ad-be84-5b446daecc3\39b42806
Ptotal= 46.70 mm Comments: 2-vr,12-hr SCS, City of Barrie

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a hyetograph with peaks at 0.083, 0.167, 0.333, 0.500, 0.750, 1.000, 1.500, 2.000, 2.500, 3.000 hours.

CALIB STANDBYD (20022) ID= 1 DT= 5.0 min Area (ha)= 0.80 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.18 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 73.03 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a transformed hyetograph with peaks at 0.083, 0.167, 0.333, 0.500, 0.750, 1.000 hours.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a second hyetograph with peaks at 1.000, 1.083, 1.167, 1.250, 1.333, 1.417, 1.500, 1.583, 1.667, 1.750, 1.833, 1.917, 2.000, 2.083, 2.167, 2.250, 2.333, 2.417, 2.500, 2.583, 2.667, 2.750, 2.833, 2.917, 3.000 hours.

Max. Eff. Inten. (mm/hr)= 61.64 over (min)= 5.00
Storage Coeff. (min)= 2.47 (ii) 6.52 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.14

PEAK FLOW (cms)= 0.11 0.01 *TOTALS* (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 43.70 27.09 43.83
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20031) ID= 1 DT= 5.0 min Area (ha)= 0.43 Total Imp(%)= 90.00 Dir. Conn.(%)= 90.00

Surface Area (ha)= 0.39 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 53.54 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a transformed hyetograph with peaks at 0.083, 0.167, 0.333, 0.500, 0.750, 1.000 hours.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a second transformed hyetograph with peaks at 1.000, 1.083, 1.167, 1.250, 1.333, 1.417, 1.500, 1.583, 1.667, 1.750, 1.833, 1.917, 2.000, 2.083, 2.167, 2.250, 2.333, 2.417, 2.500, 2.583, 2.667, 2.750, 2.833, 2.917, 3.000 hours.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a third hyetograph with peaks at 0.333, 0.417, 0.500, 0.583, 0.667, 0.750, 0.833, 0.917, 1.000, 1.083, 1.167, 1.250, 1.333, 1.417, 1.500, 1.583, 1.667, 1.750, 1.833, 1.917, 2.000, 2.083, 2.167, 2.250, 2.333, 2.417, 2.500, 2.583, 2.667, 2.750, 2.833, 2.917, 3.000 hours.

Max. Eff. Inten. (mm/hr)= 61.64 over (min)= 5.00
Storage Coeff. (min)= 2.13 (ii) 7.25 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.29 0.14

PEAK FLOW (cms)= 0.12 0.01 *TOTALS* (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 42.90
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20021) ID= 1 DT= 5.0 min Area (ha)= 0.70 Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

Surface Area (ha)= 0.62 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 68.31 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a transformed hyetograph with peaks at 0.083, 0.167, 0.333, 0.500, 0.750, 1.000 hours.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a second transformed hyetograph with peaks at 1.000, 1.083, 1.167, 1.250, 1.333, 1.417, 1.500, 1.583, 1.667, 1.750, 1.833, 1.917, 2.000, 2.083, 2.167, 2.250, 2.333, 2.417, 2.500, 2.583, 2.667, 2.750, 2.833, 2.917, 3.000 hours.

Max. Eff. Inten. (mm/hr)= 61.64 over (min)= 5.00
Storage Coeff. (min)= 2.13 (ii) 6.52 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.15

PEAK FLOW (cms)= 0.07 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 43.83
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20032) ID= 1 DT= 5.0 min Area (ha)= 0.47 Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00

Surface Area (ha)= 0.41 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a transformed hyetograph with peaks at 0.083, 0.167, 0.333, 0.500, 0.750, 1.000 hours.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows a second transformed hyetograph with peaks at 1.000, 1.083, 1.167, 1.250, 1.333, 1.417, 1.500, 1.583, 1.667, 1.750, 1.833, 1.917, 2.000, 2.083, 2.167, 2.250, 2.333, 2.417, 2.500, 2.583, 2.667, 2.750, 2.833, 2.917, 3.000 hours.

Hydrology data table with 13 columns of values representing flow and time parameters.

Max. Eff. Inten. (mm/hr)= 61.64 43.41
Storage Coeff. (min)= 2.19 (ii) 6.57 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.14

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (2004I) Area (ha)= 0.35 Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

Surface Area (ha)= 0.31 IMPERVIOUS 0.04
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.30 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr.

PEAK FLOW (cms)= 0.05 0.01 0.058 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 46.70 46.70 46.70
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20052) Area (ha)= 0.29 Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

Surface Area (ha)= 0.23 IMPERVIOUS 0.06
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 43.97 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr.

Max. Eff. Inten. (mm/hr)= 61.64 41.44
Storage Coeff. (min)= 1.89 (ii) 12.87 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.08

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Hydrology data table with 13 columns of values.

Max. Eff. Inten. (mm/hr)= 61.64 43.41
Storage Coeff. (min)= 2.00 (ii) 6.00 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.15

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20042) Area (ha)= 0.36 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.31 IMPERVIOUS 0.05
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr.

Max. Eff. Inten. (mm/hr)= 61.64 43.41
Storage Coeff. (min)= 2.02 (ii) 6.70 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.14

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20062) Area (ha)= 0.73 Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

Surface Area (ha)= 0.58 IMPERVIOUS 0.15
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.76 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr.

Max. Eff. Inten. (mm/hr)= 61.64 41.44
Storage Coeff. (min)= 2.50 (ii) 13.47 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.08

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2006I)
ID= 1 DT= 5.0 min

Area (ha)= 0.65
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
(ha)= 0.55 0.10
Surface Area (mm)= 1.00 1.00
Dep. Storage (%)= 1.00 2.00
Average Slope (m)= 65.83 40.00
Length (m)= 0.013 0.290
Mannings n =

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 43.41
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.41 (ii) 7.09 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.14

TOTALS
PEAK FLOW (cms)= 0.09 0.01 0.104 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 46.70 27.09 46.70
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2007I)
ID= 1 DT= 5.0 min

Area (ha)= 0.64
Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

IMPERVIOUS PERVIOUS (i)
(ha)= 0.54 0.10
Surface Area (mm)= 1.00 1.00
Dep. Storage (%)= 1.00 2.00
Average Slope (m)= 62.18 40.00
Length (m)= 0.013 0.290
Mannings n =

Length (m)= 65.32 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 43.41
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.40 (ii) 7.22 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.14

TOTALS
PEAK FLOW (cms)= 0.09 0.01 0.102 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 46.70
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2007I)
ID= 1 DT= 5.0 min

Area (ha)= 0.58
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
(ha)= 0.49 0.09
Surface Area (mm)= 1.00 1.00
Dep. Storage (%)= 1.00 2.00
Average Slope (m)= 62.18 40.00
Length (m)= 0.013 0.290
Mannings n =

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.833	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.917	1.17	3.833	1.87	6.833	3.74	9.83	1.63
1.000	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.083	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.167	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.250	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.333	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.417	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.500	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.583	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.667	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.750	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.833	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.917	1.17	4.833	3.74	7.833	2.80	10.83	0.93
2.000	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.39 (ii) 13.36 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.30 0.08

TOTALS
PEAK FLOW (cms)= 0.09 0.01 0.095 (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 46.70
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2009I)
ID= 1 DT= 5.0 min

Area (ha)= 0.40
Total Imp(%)= 77.00 Dir. Conn.(%)= 77.00

IMPERVIOUS PERVIOUS (i)
(ha)= 0.31 0.09
Surface Area (mm)= 1.00 1.00
Dep. Storage (%)= 1.00 2.00
Average Slope (m)= 51.64 40.00
Length (m)= 0.013 0.290
Mannings n =

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08</	

1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min) = 5.00 15.00
Storage Coeff. (min)= 2.09 (ii) 13.06 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

PEAK FLOW (cms)= 0.05 0.01 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.059 (iii)
RUNOFF VOLUME (mm)= 45.70 27.09 41.40
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200812) ID= 1 DT= 5.0 min	Area (ha)= 0.38 Total Imp(%)= 70.00	Dir. Conn.(%)= 70.00
Surface Area (ha)= 0.27 Dep. Storage (mm)= 1.00 Average Slope (%)= 1.00 Length (m)= 50.33 Mannings n = 0.013	IMPERVIOUS PERVIOUS (i)	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	1.87	7.083	3.74	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93

2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 43.41
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.24 (ii) 6.06 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.01 0.00 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.011 (iii)
RUNOFF VOLUME (mm)= 45.70 27.09 42.71
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (200811):	0.07	0.011	6.00	42.71
+ ID2= 2 (200812):	0.38	0.053	6.00	40.10
ID = 3 (0110):	0.45	0.064	6.00	40.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200822) ID= 1 DT= 5.0 min	Area (ha)= 0.39 Total Imp(%)= 59.00	Dir. Conn.(%)= 59.00
Surface Area (ha)= 0.23 Dep. Storage (mm)= 1.00 Average Slope (%)= 1.00 Length (m)= 50.99 Mannings n = 0.013	IMPERVIOUS PERVIOUS (i)	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93

2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min) = 5.00 15.00
Storage Coeff. (min)= 2.05 (ii) 13.03 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

PEAK FLOW (cms)= 0.05 0.01 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.053 (iii)
RUNOFF VOLUME (mm)= 45.70 27.09 41.40
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200811) ID= 1 DT= 5.0 min	Area (ha)= 0.07 Total Imp(%)= 84.00	Dir. Conn.(%)= 84.00
Surface Area (ha)= 0.06 Dep. Storage (mm)= 1.00 Average Slope (%)= 1.00 Length (m)= 21.60 Mannings n = 0.013	IMPERVIOUS PERVIOUS (i)	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93

2.333	1.40	5.333	5.60	8.333	1.6
-------	------	-------	------	-------	-----

3.000 1.40 | 6.000 61.64 | 9.000 1.63 | 12.00 0.93

Max.Eff.Inten.(mm/hr)= 61.64 43.41
over (min) = 5.00 10.00
Storage Coeff (min)= 0.33 (ii) 6.23 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15

TOTALS

PEAK FLOW (cms)= 0.01 0.00 0.01 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 42.52
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD (0111), L + 2 = 3, AREA (ha), OPEAK (cms), TPEAK (hrs), R.V. (mm). Rows for ID1=1, ID2=2, and ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 3 columns: CALIB STANDHYD (20102), Area (ha), Total Imp(%). Values: Area=0.47, Total Imp=74.00.

Surface Area (ha)= 0.35
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 55.98
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

2.667 1.40 | 5.667 22.42 | 8.667 1.63 | 11.67 0.93
2.750 1.40 | 5.750 22.42 | 8.750 1.63 | 11.75 0.93
2.833 1.40 | 5.833 61.64 | 8.833 1.63 | 11.83 0.93
2.917 1.40 | 5.917 61.64 | 8.917 1.63 | 11.92 0.93
3.000 1.40 | 6.000 61.64 9.000 1.63 | 12.00 0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min) = 5.00 15.00
Storage Coeff (min)= 2.19 (ii) 13.16 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

TOTALS

PEAK FLOW (cms)= 0.06 0.01 0.06 (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 40.84
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 3 columns: CALIB STANDHYD (20101), Area (ha), Total Imp(%). Values: Area=0.25, Total Imp=82.00.

Surface Area (ha)= 0.05
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 40.82
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min) = 5.00 15.00

Storage Coeff. (min)= 1.81 (ii) 12.78 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.08

TOTALS

PEAK FLOW (cms)= 0.04 0.00 0.038 (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 42.32
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD (0121), L + 2 = 3, AREA (ha), OPEAK (cms), TPEAK (hrs), R.V. (mm). Rows for ID1=1, ID2=2, and ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 3 columns: CALIB STANDHYD (20111), Area (ha), Total Imp(%). Values: Area=0.87, Total Imp=67.00.

Surface Area (ha)= 0.58
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 76.16
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

3.000 1.40 | 6.000 61.64 | 9.000 1.63 | 12.00 0.93

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min) = 5.00 15.00
Storage Coeff. (min)= 2.63 (ii) 13.61 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.08

TOTALS

PEAK FLOW (cms)= 0.10 0.02 0.118 (iii)
TIME TO PEAK (hrs)= 6.00 6.08 6.00
RUNOFF VOLUME (mm)= 45.70 27.09 39.55
TOTAL RAINFALL (mm)= 46.70 46.70 46.70
RUNOFF COEFFICIENT = 0.98 0.58 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 3 columns: CALIB STANDHYD (20112), Area (ha), Total Imp(%). Values: Area=0.90, Total Imp=80.00.

Surface Area (ha)= 0.72
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 77.46
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr).

Max.Eff.Inten.(mm/hr)= 61.64 41.44
over (min) = 5.00 15.00
Storage Coeff. (min)= 2.66 (ii) 13.63 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.08

TOTALS

PEAK FLOW (cms)= 0.12 0.01 0.135 (iii)
 TIME TO PEAK (hrs)= 6.00 6.08 6.00
 RUNOFF VOLUME (mm)= 45.70 27.09 41.97
 TOTAL RAINFALL (mm)= 46.70 46.70 46.70
 RUNOFF COEFFICIENT = 0.98 0.58 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN[#] = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20122) ID= 1 DT= 5.0 min	Area (ha)= 1.11 Total Imp(%)= 81.00	Dir. Conn.(%)= 81.00
--	--	----------------------

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.90	0.21	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	86.02	40.00	
Mannings n	= 0.013	= 0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max. Eff. Inten. (mm/hr)= 61.64 41.44
 over (min) = 5.00 15.00
 Storage Coeff. (min)= 2.83 (ii) 13.81 (ii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.28 0.08

PEAK FLOW (cms)= 0.15 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.08 0.167 (iii)
 RUNOFF VOLUME (mm)= 45.70 27.09 6.00
 TOTAL RAINFALL (mm)= 46.70 46.70 42.16
 RUNOFF COEFFICIENT = 0.98 0.58 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

CALIB STANDHYD (201022) ID= 1 DT= 5.0 min	Area (ha)= 0.47 Total Imp(%)= 85.00	Dir. Conn.(%)= 85.00
---	--	----------------------

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.40	0.07	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	55.98	40.00	
Mannings n	= 0.013	= 0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max. Eff. Inten. (mm/hr)= 61.64 43.41
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 2.19 (ii) 6.87 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.31 0.14

PEAK FLOW (cms)= 0.07 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.075 (iii)
 RUNOFF VOLUME (mm)= 45.70 27.09 6.00
 TOTAL RAINFALL (mm)= 46.70 46.70 42.90
 RUNOFF COEFFICIENT = 0.98 0.58 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN[#] = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136) ID= 2 (201022) ID= 3 (0136)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	0.47	0.075	6.00	42.90
	0.47	0.075	6.00	42.90
	0.94	0.151	6.00	42.90

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN[#] = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (201021) ID= 1 DT= 5.0 min	Area (ha)= 0.47 Total Imp(%)= 85.00	Dir. Conn.(%)= 85.00
---	--	----------------------

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.40	0.07	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	55.98	40.00	
Mannings n	= 0.013	= 0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max. Eff. Inten. (mm/hr)= 61.64 43.41

Surface Area (ha)=	0.62	PERVIOUS (i)	0.49
Dep. Storage (mm)=	1.00		1.00
Average Slope (%)=	1.00		2.00
Length (m)=	86.02		40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max. Eff. Inten. (mm/hr)=	61.64	41.44	
over (min)	5.00	15.00	
Storage Coeff. (min)=	2.83 (ii)	13.81 (ii)	
Unit Hyd. Tpeak (min)=	5.00	15.00	
Unit Hyd. peak (cms)=	0.28	0.08	
PEAK FLOW (cms)=	0.11	0.04	*TOTALS* (iii)
TIME TO PEAK (hrs)=	6.00	6.08	6.00
RUNOFF VOLUME (mm)=	45.70	27.09	43.74
TOTAL RAINFALL (mm)=	46.70	46.70	46.70
RUNOFF COEFFICIENT =	0.98	0.58	0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)=	0.10	
STANDHYD (200111)	Total Imp(%)=	89.50	Dir. Conn.(%)= 89.50
ID= 1 DT= 5.0 min			

Surface Area (ha)=	0.09	PERVIOUS (i)	0.01
Dep. Storage (mm)=	1.00		1.00
Average Slope (%)=	1.00		2.00
Length (m)=	25.82		40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max. Eff. Inten. (mm/hr)=	61.64	43.41	
over (min)	5.00	10.00	
Storage Coeff. (min)=	1.42 (ii)	5.79 (ii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.33	0.15	
PEAK FLOW (cms)=	0.02	0.00	*TOTALS* (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	45.70	27.09	43.74
TOTAL RAINFALL (mm)=	46.70	46.70	46.70
RUNOFF COEFFICIENT =	0.98	0.58	0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142)	Area (ha)=	0.10	
1 + 2 = 3	Total Imp(%)=	79.00	Dir. Conn.(%)= 79.00

CALIB	Area (ha)=	0.10	
STANDHYD (200111)	Total Imp(%)=	79.00	Dir. Conn.(%)= 79.00
ID= 1 DT= 5.0 min			

Surface Area (ha)=	0.08	PERVIOUS (i)	0.02
Dep. Storage (mm)=	1.00		1.00
Average Slope (%)=	1.00		2.00
Length (m)=	25.69		40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63

----- TRANSFORMED HYETOGRAPH -----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.17	3.083	1.87	6.083	8.41	9.08	1.63
0.167	1.17	3.167	1.87	6.167	8.41	9.17	1.63
0.250	1.17	3.250	1.87	6.250	8.41	9.25	1.63
0.333	1.17	3.333	1.87	6.333	8.41	9.33	1.63
0.417	1.17	3.417	1.87	6.417	8.41	9.42	1.63
0.500	1.17	3.500	1.87	6.500	8.41	9.50	1.63
0.583	1.17	3.583	1.87	6.583	3.74	9.58	1.63
0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	2.80	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1		

0.667	1.17	3.667	1.87	6.667	3.74	9.67	1.63
0.750	1.17	3.750	1.87	6.750	3.74	9.75	1.63
0.833	1.17	3.833	1.87	6.833	3.74	9.83	1.63
0.917	1.17	3.917	1.87	6.917	3.74	9.92	1.63
1.000	1.17	4.000	1.87	7.000	3.74	10.00	1.63
1.083	1.17	4.083	2.80	7.083	2.80	10.08	0.93
1.167	1.17	4.167	2.80	7.167	2.80	10.17	0.93
1.250	1.17	4.250	2.80	7.250	2.80	10.25	0.93
1.333	1.17	4.333	2.80	7.333	2.80	10.33	0.93
1.417	1.17	4.417	2.80	7.417	2.80	10.42	0.93
1.500	1.17	4.500	2.80	7.500	2.80	10.50	0.93
1.583	1.17	4.583	3.74	7.583	3.74	10.58	0.93
1.667	1.17	4.667	3.74	7.667	2.80	10.67	0.93
1.750	1.17	4.750	3.74	7.750	2.80	10.75	0.93
1.833	1.17	4.833	3.74	7.833	2.80	10.83	0.93
1.917	1.17	4.917	3.74	7.917	2.80	10.92	0.93
2.000	1.17	5.000	3.74	8.000	2.80	11.00	0.93
2.083	1.40	5.083	5.60	8.083	1.63	11.08	0.93
2.167	1.40	5.167	5.60	8.167	1.63	11.17	0.93
2.250	1.40	5.250	5.60	8.250	1.63	11.25	0.93
2.333	1.40	5.333	5.60	8.333	1.63	11.33	0.93
2.417	1.40	5.417	5.60	8.417	1.63	11.42	0.93
2.500	1.40	5.500	5.60	8.500	1.63	11.50	0.93
2.583	1.40	5.583	22.42	8.583	1.63	11.58	0.93
2.667	1.40	5.667	22.42	8.667	1.63	11.67	0.93
2.750	1.40	5.750	22.42	8.750	1.63	11.75	0.93
2.833	1.40	5.833	61.64	8.833	1.63	11.83	0.93
2.917	1.40	5.917	61.64	8.917	1.63	11.92	0.93
3.000	1.40	6.000	61.64	9.000	1.63	12.00	0.93

Max.Eff.Inten.(mm/hr)= 61.64 43.41
 over (min) 5.00 10.00
 Storage Coeff. (min)= 1.49 (ii) 6.02 (iii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.02 0.00 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.021 (iii)
 RUNOFF VOLUME (mm)= 45.70 27.09 43.08
 TOTAL RAINFALL (mm)= 46.70 46.70 46.70
 RUNOFF COEFFICIENT = 0.98 0.58 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (200112):	0.10	0.015	6.00	41.75
+ ID2= 2 (200122):	0.13	0.021	6.00	43.08
ID = 3 (0145):	0.23	0.036	6.00	42.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 6.1.2002)
 V V I SS U U A A A L
 V V I SS U U A A A L
 W I SSSSS UUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M O O TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2020 Smart City Water Inc
 All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.1\VO2\voin.dat

2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)= 84.88 66.77
 over (min) 5.00 10.00
 Storage Coeff. (min)= 2.26 (ii) 6.38 (iii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.30 0.15

PEAK FLOW (cms)= 0.16 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.179 (iii)
 RUNOFF VOLUME (mm)= 63.30 42.31 60.15
 TOTAL RAINFALL (mm)= 64.30 64.30 64.30
 RUNOFF COEFFICIENT = 0.98 0.66 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20021)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
ID= 1 DT= 5.0 min	0.70	89.00	89.00

Surface Area (ha)= 0.62 0.08
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 68.31 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29

Output filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\0f24e9d2
 Summary filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\0f24e9d2

DATE: 05-18-2022 TIME: 12:50:57

USER:

COMMENTS:

** SIMULATION : Run 14 **

FILENAME: C:\Users\caef070146\AppData\Local\Temp\338b910-df19-48ad-be84-5b44b6daecc3\5b7ccddc
 Ptotal= 64.30 mm
 Comments: 2-Yr,12-hr SCS, City of Barrie

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.61	3.25	2.57	6.25	11.57	9.25	2.25
0.50	1.61	3.50	2.57	6.50	11.57	9.50	2.25
0.75	1.61	3.75	2.57	6.75	5.14	9.75	2.25
1.00	1.61	4.00	2.57	7.00	5.14	10.00	2.25
1.25	1.61	4.25	3.86	7.25	3.86	10.25	1.29
1.50	1.61	4.50	3.86	7.50	3.86	10.50	1.29
1.75	1.61	4.75	5.14	7.75	3.86	10.75	1.29
2.00	1.61	5.00	5.14	8.00	3.86	11.00	1.29
2.25	1.93	5.25	7.72	8.25	2.25	11.25	1.29
2.50	1.93	5.50	7.72	8.50	2.25	11.50	1.29
2.75	1.93	5.75	30.86	8.75	2.25	11.75	1.29
3.00	1.93	6.00	84.88	9.00	2.25	12.00	1.29

CALIB STANDBYD (20022)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
ID= 1 DT= 5.0 min	0.80	85.00	85.00

Surface Area (ha)= 0.68 0.12
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 73.33 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29

2.750	1.93	5.750	30.86
-------	------	-------	-------

Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16
PEAK FLOW (cms)= 0.09 0.01
TIME TO PEAK (hrs)= 6.00 6.00
RUNOFF VOLUME (mm)= 63.30 42.31
TOTAL RAINFALL (mm)= 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66

TOTALS
0.098 (iii)
6.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20032) Area (ha)= 0.47
Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00

Surface Area (ha)= 0.41 0.06
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

IMPERVIOUS PERVIOUS (i)

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

Max.Eff.Inten.(mm/hr)= 84.88 66.77
Storage Coeff.(min)= 1.93 (ii) 5.78 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.15

TOTALS
0.106 (iii)
6.00
63.30 42.31 60.56
64.30 64.30 64.30
0.98 0.66 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20042) Area (ha)= 0.36
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.31 0.05
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

IMPERVIOUS PERVIOUS (i)

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

Max.Eff.Inten.(mm/hr)= 84.88 66.77
Storage Coeff.(min)= 1.78 (ii) 6.63 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

TOTALS
0.081 (iii)
6.00
63.30 42.31 60.14
64.30 64.30 64.30
0.98 0.66 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20052) Area (ha)= 0.29
Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

IMPERVIOUS PERVIOUS (i)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20041) Area (ha)= 0.35
Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

Surface Area (ha)= 0.31 0.04
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.30 40.00
Mannings n = 0.013 0.290

IMPERVIOUS PERVIOUS (i)

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

Max.Eff.Inten.(mm/hr)= 84.88 66.77
Storage Coeff.(min)= 1.76 (ii) 5.33 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

TOTALS
0.07 0.01 0.080 (iii)
6.00 6.00 6.00
63.30 42.31 60.99
64.30 64.30 64.30
0.98 0.66 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20062) Area (ha)= 0.73
Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

Surface Area (ha)= 0.23 0.06
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 43.97 40.00
Mannings n = 0.013 0.290

IMPERVIOUS PERVIOUS (i)

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

Max.Eff.Inten.(mm/hr)= 84.88 66.77
Storage Coeff.(min)= 1.67 (ii) 6.63 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.14

TOTALS
0.05 0.01 0.063 (iii)
6.00 6.00 6.00
63.30 42.31 58.67
64.30 64.30 64.30
0.98 0.66 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20062) Area (ha)= 0.73
Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

Surface Area (ha)= 0.58 0.15
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 69.76 40.00
Mannings n = 0.013 0.290

IMPERVIOUS PERVIOUS (i)

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max.Eff.Inten.(mm/hr)= 84.88 over (min)= 5.00 Storage Coeff.(min)= 2.20 Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.30 PEAK FLOW (cms)= 0.14 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 63.30 TOTAL RAINFALL (mm)= 64.30 RUNOFF COEFFICIENT = 0.98

TOTALS 0.160 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20061) Area (ha)= 0.65 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.55 IMPERVIOUS (%)= 1.00 Average Slope (m)= 65.83 Length (m)= 0.013 Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max.Eff.Inten.(mm/hr)= 84.88 over (min)= 5.00 Storage Coeff.(min)= 2.11 Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.31 PEAK FLOW (cms)= 0.13 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 63.30 TOTAL RAINFALL (mm)= 64.30 RUNOFF COEFFICIENT = 0.98

TOTALS 0.143 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20071) Area (ha)= 0.58 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.49 IMPERVIOUS (%)= 1.00 Average Slope (m)= 62.18 Length (m)= 0.013 Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 1.667 hours.

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.417 to 3.000 hours.

Max.Eff.Inten.(mm/hr)= 84.88 over (min)= 5.00 Storage Coeff.(min)= 2.12 Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.31 PEAK FLOW (cms)= 0.13 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 63.30 TOTAL RAINFALL (mm)= 64.30 RUNOFF COEFFICIENT = 0.98

TOTALS 0.146 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20072) Area (ha)= 0.64 Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

Surface Area (ha)= 0.54 IMPERVIOUS (%)= 1.00 Average Slope (m)= 65.32 Length (m)= 0.013 Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max.Eff.Inten.(mm/hr)= 84.88 over (min)= 5.00 Storage Coeff.(min)= 2.05 Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.31 PEAK FLOW (cms)= 0.12 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 63.30 TOTAL RAINFALL (mm)= 64.30 RUNOFF COEFFICIENT = 0.98

TOTALS 0.130 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20092) Area (ha)= 0.63 Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

Surface Area (ha)= 0.51 IMPERVIOUS (%)= 1.00 Average Slope (m)= 64.81 Length (m)= 0.013 Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 2.333 hours.

2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)=	84.88	66.77
over (min)	5.00	10.00
Storage Coeff (min)=	6.72 (ii)	10.00 (iii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.31	0.14

PEAK FLOW (cms)=	0.12	0.02	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.139 (iii)
RUNOFF VOLUME (mm)=	63.30	42.31	6.00
TOTAL RAINFALL (mm)=	64.30	64.30	59.31
RUNOFF COEFFICIENT	0.98	0.66	0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20091)	Area (ha)= 0.40	Dir. Conn.(%)= 77.00
ID= 1 DT= 5.0 min	Total Imp(%)= 77.00	

Surface Area (ha)=	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)=	0.31	0.00
Average Slope (%)=	1.00	2.00
Length (m)=	51.64	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)=	84.88	66.77	
over (min)	5.00	15.00	
Storage Coeff (min)=	1.83 (ii)	10.90 (iii)	
Unit Hyd. Tpeak (min)=	5.00	15.00	
Unit Hyd. peak (cms)=	0.32	0.09	
PEAK FLOW (cms)=	0.07	0.01	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.08	0.083 (iii)
RUNOFF VOLUME (mm)=	63.30	42.31	6.00
TOTAL RAINFALL (mm)=	64.30	64.30	58.46
RUNOFF COEFFICIENT	0.98	0.66	0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200812)	Area (ha)= 0.38	Dir. Conn.(%)= 70.00
ID= 1 DT= 5.0 min	Total Imp(%)= 70.00	

Surface Area (ha)=	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)=	0.27	0.11
Average Slope (%)=	1.00	2.00
Length (m)=	50.33	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)=	84.88	66.77	
over (min)	5.00	15.00	
Storage Coeff (min)=	1.81 (ii)	10.87 (iii)	
Unit Hyd. Tpeak (min)=	5.00	15.00	
Unit Hyd. peak (cms)=	0.32	0.09	
PEAK FLOW (cms)=	0.06	0.01	*TOTALS*
			0.076 (iii)

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID= 1 (200811):	0.07	0.016	6.00	59.92
+ ID= 2 (200812):	0.38	0.076	6.00	56.99
ID= 3 (0110):	0.45	0.091	6.00	57.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (200822)	Area (ha)= 0.39	Dir. Conn.(%)= 59.00
ID= 1 DT= 5.0 min	Total Imp(%)= 59.00	

Surface Area (ha)=	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)=	0.23	0.16
Average Slope (%)=	1.00	2.00
Length (m)=	50.99	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	3.86	7.667	3.86	10.67	1.29
1.750	1.61	4.750	3.86	7.750	3.86	10.75	1.29
1.833	1.61	4.833	3.86	7.833	3.86	10.83	1.29
1.917	1.61	4.917	3.86	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250</							

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200821) Area (ha)= 0.07 Total Imp(%)= 83.00 Dir. Conn.(%)= 83.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.06 0.01 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 21.60 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH --- TIME RAIN TIME RAIN TIME RAIN TIME RAIN hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max. Eff. Inten. (mm/hr)= 84.88 66.77 over (min)= 5.00 10.00 Storage Coeff. (min)= 1.09 (ii) 5.46 (ii) Unit Hyd. Tpeak (min)= 5.00 10.00 Unit Hyd. peak (cms)= 0.34 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20101) Area (ha)= 0.25 Total Imp(%)= 82.00 Dir. Conn.(%)= 82.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.20 0.05 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 40.82 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH --- TIME RAIN TIME RAIN TIME RAIN TIME RAIN hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max. Eff. Inten. (mm/hr)= 84.88 66.77 over (min)= 5.00 10.00 Storage Coeff. (min)= 1.59 (ii) 6.09 (ii) Unit Hyd. Tpeak (min)= 5.00 10.00 Unit Hyd. peak (cms)= 0.33 0.15

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0111) 1 + 2 = 3 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (20102) Area (ha)= 0.47 Total Imp(%)= 74.00 Dir. Conn.(%)= 74.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.35 0.12 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 55.98 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH --- TIME RAIN TIME RAIN TIME RAIN TIME RAIN hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max. Eff. Inten. (mm/hr)= 84.88 66.77 over (min)= 5.00 15.00 Storage Coeff. (min)= 1.93 (ii) 10.99 (ii) Unit Hyd. Tpeak (min)= 5.00 15.00 Unit Hyd. peak (cms)= 0.31 0.09

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

ADD HYD (0121) 1 + 2 = 3 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (20111) Area (ha)= 0.87 Total Imp(%)= 67.00 Dir. Conn.(%)= 67.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.58 0.29 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 76.16 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH --- TIME RAIN TIME RAIN TIME RAIN TIME RAIN hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max. Eff. Inten. (mm/hr)= 84.88 66.77 over (min)= 5.00 15.00 Storage Coeff. (min)= 2.32 (ii) 15.00 (ii) Unit Hyd. Tpeak (min)= 5.00 15.00 Unit Hyd. peak (cms)= 0.30 0.09

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CALIB
STANDHYD (20112)
ID= 1 DT= 5.0 min

Area (ha)= 0.90
Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.72 0.18
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 77.46 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGRAP ---															
TIME	RAIN		RAIN		RAIN		TIME	RAIN		RAIN					
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr				
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25	0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25	0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25	0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25	0.583	1.61	3.583	2.57	6.583	11.57	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25	0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25	0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25	1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29	1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29	1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29	1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.58	1.29	1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29	1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29	2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29	2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.25	1.29	2.417	1.93	5.417	7.72	8.417	2.25	11.33	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29	2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.58	1.29	2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	30.86	8.833	2.25	11.83	1.29	2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29								

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.34 (ii) 7.07 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.14
PEAK FLOW (cms)= 0.17 0.03 0.197 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 63.30 42.31 59.10
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^N = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20122)
ID= 1 DT= 5.0 min

Area (ha)= 1.11
Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

IMPERVIOUS PERVIOUS (i)

--- TRANSFORMED HETOGRAP ---															
TIME	RAIN		RAIN		RAIN		TIME	RAIN		RAIN					
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr				
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25	0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25	0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25	0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25	0.583	1.61	3.583	2.57	6.583	11.57	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25	0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25	0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25	1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29	1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29	1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29	1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.58	1.29	1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29	1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29	2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29	2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.25	1.29	2.417	1.93	5.417	7.72	8.417	2.25	11.33	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29	2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29	2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	30.86	8.833	2.25	11.83	1.29	2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29								

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.93 (ii) 6.04 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.15
PEAK FLOW (cms)= 0.09 0.01 0.105 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 63.30 42.31 60.14
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^N = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (201022)
ID= 1 DT= 5.0 min

Area (ha)= 0.47
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.40 0.07
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGRAP ---															
TIME	RAIN		RAIN		RAIN		TIME	RAIN		RAIN					
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr				
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25	0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25	0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25

Surface Area (ha)= 0.90 0.21
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 86.02 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGRAP ---															
TIME	RAIN		RAIN		RAIN		TIME	RAIN		RAIN					
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr				
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25	0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25	0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25	0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	11.57	9.58	2.25	0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25	0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25	1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29	1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29	1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29	1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29	1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29	1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29	2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29	2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29	2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29	2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29	2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29	2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29	3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.49 (ii) 7.10 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.29 0.14
PEAK FLOW (cms)= 0.21 0.03 0.244 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 63.30 42.31 59.31
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^N = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (201021)
ID= 1 DT= 5.0 min

Area (ha)= 0.47
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.40 0.07
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.

0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	3.86	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min) 5.00 10.00
Storage Coeff.(min)= 1.65 (ii) 5.07 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.06 0.00 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.064 (iii)
RUNOFF VOLUME (mm)= 63.30 42.31 61.20
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 1.11	Dir. Conn.(%)= 56.00
STANDHYD (20212)	Total Imp(%)= 56.00	
ID= 1 DT= 5.0 min		

Surface Area (ha)= 0.62	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)= 1.00		
Average Slope (%)= 1.00		
Length (m)= 86.02		
Mannings n = 0.013		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25		
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25		
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25		
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25		
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25		
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25		
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25		
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25		

1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min) 5.00 10.00
Storage Coeff.(min)= 1.21 (ii) 4.71 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.02 0.00 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.023 (iii)
RUNOFF VOLUME (mm)= 63.30 42.31 61.09
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 0.11	Dir. Conn.(%)= 87.00
STANDHYD (200121)	Total Imp(%)= 87.00	
ID= 1 DT= 5.0 min		

Surface Area (ha)= 0.10	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)= 1.21		
Average Slope (%)= 1.00		
Length (m)= 27.08		
Mannings n = 0.013		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25		
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25		
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25		
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25		
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25		
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25		
0.583	1.61	3.583	2.57	6.583	5.14	9.58	2.25		
0.667	1.61	3.667	2.57	6.667	5.14	9.67	2.25		
0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25		
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25		
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25		
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25		
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29		
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29		
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29		
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29		
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29		
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29		
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29		
1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29		
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29		
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29		
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29		
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29		

0.750	1.61	3.750	2.57	6.750	5.14	9.75	2.25
0.833	1.61	3.833	2.57	6.833	5.14	9.83	2.25
0.917	1.61	3.917	2.57	6.917	5.14	9.92	2.25
1.000	1.61	4.000	2.57	7.000	5.14	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max.Eff.Inten.(mm/hr)= 84.88 66.77
over (min) 5.00 15.00
Storage Coeff.(min)= 2.49 (ii) 11.56 (iii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.09

PEAK FLOW (cms)= 0.15 0.06 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.20 (iii)
RUNOFF VOLUME (mm)= 63.30 42.31 54.06
TOTAL RAINFALL (mm)= 64.30 64.30 64.30
RUNOFF COEFFICIENT = 0.98 0.66 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 0.10	Dir. Conn.(%)= 89.50
STANDHYD (200111)	Total Imp(%)= 89.50	
ID= 1 DT= 5.0 min		

Surface Area (ha)= 0.09	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)= 1.00		
Average Slope (%)= 1.00		
Length (m)= 25.82		
Mannings n = 0.013		

1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29
2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max. Eff. Inten. (mm/hr)= 84.88 over (min)= 5.00
Storage Coeff (min)= 1.21 (ii) Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.33

TOTALS
PEAK FLOW (cms)= 0.02
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 63.30
TOTAL RAINFALL (mm)= 64.30
RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200122) Area (ha)= 0.13 Total Imp(%)= 86.00 Dir. Conn.(%)= 86.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.13
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 29.44
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.61	3.083	2.57	6.083	11.57	9.08	2.25
0.167	1.61	3.167	2.57	6.167	11.57	9.17	2.25
0.250	1.61	3.250	2.57	6.250	11.57	9.25	2.25
0.333	1.61	3.333	2.57	6.333	11.57	9.33	2.25
0.417	1.61	3.417	2.57	6.417	11.57	9.42	2.25
0.500	1.61	3.500	2.57	6.500	11.57	9.50	2.25
0.583	1.61	3.583	2.57	6.583	11.57	9.58	2.25
0.667	1.61	3.667	2.57	6.667	11.57	9.67	2.25
0.750	1.61	3.750	2.57	6.750	11.57	9.75	2.25
0.833	1.61	3.833	2.57	6.833	11.57	9.83	2.25
0.917	1.61	3.917	2.57	6.917	11.57	9.92	2.25
1.000	1.61	4.000	2.57	7.000	11.57	10.00	2.25
1.083	1.61	4.083	3.86	7.083	3.86	10.08	1.29
1.167	1.61	4.167	3.86	7.167	3.86	10.17	1.29
1.250	1.61	4.250	3.86	7.250	3.86	10.25	1.29
1.333	1.61	4.333	3.86	7.333	3.86	10.33	1.29
1.417	1.61	4.417	3.86	7.417	3.86	10.42	1.29
1.500	1.61	4.500	3.86	7.500	3.86	10.50	1.29
1.583	1.61	4.583	5.14	7.583	3.86	10.58	1.29
1.667	1.61	4.667	5.14	7.667	3.86	10.67	1.29
1.750	1.61	4.750	5.14	7.750	3.86	10.75	1.29
1.833	1.61	4.833	5.14	7.833	3.86	10.83	1.29
1.917	1.61	4.917	5.14	7.917	3.86	10.92	1.29
2.000	1.61	5.000	5.14	8.000	3.86	11.00	1.29
2.083	1.93	5.083	7.72	8.083	2.25	11.08	1.29
2.167	1.93	5.167	7.72	8.167	2.25	11.17	1.29
2.250	1.93	5.250	7.72	8.250	2.25	11.25	1.29
2.333	1.93	5.333	7.72	8.333	2.25	11.33	1.29

CALIB STANDHYD (20022) Area (ha)= 0.80 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.68
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 75.03
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)= 100.32 over (min)= 5.00
Storage Coeff (min)= 2.11 (ii) Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.31

TOTALS
PEAK FLOW (cms)= 0.19
TIME TO PEAK (hrs)= 6.00

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)

2.417	1.93	5.417	7.72	8.417	2.25	11.42	1.29
2.500	1.93	5.500	7.72	8.500	2.25	11.50	1.29
2.583	1.93	5.583	30.86	8.583	2.25	11.58	1.29
2.667	1.93	5.667	30.86	8.667	2.25	11.67	1.29
2.750	1.93	5.750	30.86	8.750	2.25	11.75	1.29
2.833	1.93	5.833	84.88	8.833	2.25	11.83	1.29
2.917	1.93	5.917	84.88	8.917	2.25	11.92	1.29
3.000	1.93	6.000	84.87	9.000	2.25	12.00	1.29

Max. Eff. Inten. (mm/hr)= 84.88 over (min)= 5.00
Storage Coeff (min)= 1.31 (ii) Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.33

TOTALS
PEAK FLOW (cms)= 0.03
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 63.30
TOTAL RAINFALL (mm)= 64.30
RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (200112):	0.10	0.022	6.00	58.86
+ ID= 2 (200122):	0.13	0.029	6.00	60.34

ID = 3 (0145):	0.23	0.051	6.00	59.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U A A L
V V I SSSSS UUUU A LLLLL

000 TTTT TTTT H H Y Y M M O O TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M O O

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.1\VO2\voindat
Output filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\73cb36f5
Summary filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\73cb36f5

DATE: 05-18-2022 TIME: 12:50:58

USER:

COMMENTS:

***** SIMULATION: Run 15 *****

READ STORM Filename: C:\Users\caef070146\AppData

CALIB STANDHYD (20021) Area (ha)= 0.70 Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.62
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 68.31
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

CALIB STANDHYD (20021) Area (ha)= 0.70 Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.62
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 68.31
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000					

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20031) Area (ha)= 0.43
ID= 1 DT= 5.0 min Total Imp(%)= 90.00 Dir. Conn.(%)= 90.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.39 0.06
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 53.54 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	6.08	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)= 100.32 82.63
over (min) = 5.00 5.00
Storage Coeff. (min)= 1.75 (ii) 4.99 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.11 0.01 0.17 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 75.00 52.87 72.78
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CNⁿ = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20032) Area (ha)= 0.47

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	6.08	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)= 100.32 82.63
over (min) = 5.00 5.00
Storage Coeff. (min)= 1.65 (ii) 4.99 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.09 0.01 0.09 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 75.00 52.87 72.78
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CNⁿ = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20042) Area (ha)= 0.36
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.31 0.05
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66

ID= 1 DT= 5.0 min | Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00

Surface Area (ha)= 0.41 0.06
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 53.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)= 100.32 82.63
over (min) = 5.00 5.00
Storage Coeff. (min)= 1.50 (ii) 5.40 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.11 0.01 0.17 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 75.00 52.87 72.78
TOTAL RAINFALL (mm)= 76.00 76.00 76.00
RUNOFF COEFFICIENT = 0.99 0.70 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CNⁿ = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20041) Area (ha)= 0.35
ID= 1 DT= 5.0 min Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.31 0.04
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.30 40.00
Mannings n = 0.013 0.290

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	

0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)=	100.32	82.63	
over (min)	5.00	10.00	
Storage Coeff. (min)=	1.56 (ii)	6.20 (iii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.33	0.15	
PEAK FLOW (cms)=	0.06	0.01	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.076 (iii)
RUNOFF VOLUME (mm)=	75.00	52.87	6.00
TOTAL RAINFALL (mm)=	76.00	76.00	76.00
RUNOFF COEFFICIENT	0.99	0.70	0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (2006J)	Area (ha)= 0.73
ID= 1 DT= 5.0 min	Total Imp (%) = 80.00	Dir. Conn. (%) = 80.00

Surface Area (ha)=	0.58	0.15
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	69.76	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED ---		HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66				
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66				
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66				
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66				
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66				
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66				
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66				
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66				
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66				
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66				
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66				
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66				
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52				
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52				
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52				
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52				
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52				

2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)=	100.32	82.63	
over (min)	5.00	10.00	
Storage Coeff. (min)=	1.99 (ii)	5.84 (iii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.31	0.15	
PEAK FLOW (cms)=	0.15	0.02	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.174 (iii)
RUNOFF VOLUME (mm)=	75.00	52.87	6.00
TOTAL RAINFALL (mm)=	76.00	76.00	76.00
RUNOFF COEFFICIENT	0.99	0.70	0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (2007J)	Area (ha)= 0.64
ID= 1 DT= 5.0 min	Total Imp (%) = 84.00	Dir. Conn. (%) = 84.00

Surface Area (ha)=	0.54	0.10
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	65.32	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED ---		HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66				
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66				
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66				
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66				
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66				
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66				
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66				
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66				
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66				
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66				
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66				
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66				
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52				
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52				
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52				
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52				
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52				
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52				
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52				
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52				
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52				
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52				
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52				
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52				
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52				
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52				
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52				
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52				
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52				
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52				
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52				
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52				
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52				

1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)=	100.32	82.63	
over (min)	5.00	10.00	
Storage Coeff. (min)=	2.06 (ii)	6.48 (iii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.31	0.14	
PEAK FLOW (cms)=	0.16	0.03	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.191 (iii)
RUNOFF VOLUME (mm)=	75.00	52.87	6.00
TOTAL RAINFALL (mm)=	76.00	76.00	76.00
RUNOFF COEFFICIENT	0.99	0.70	0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (2006J)	Area (ha)= 0.65
ID= 1 DT= 5.0 min	Total Imp (%) = 85.00	Dir. Conn. (%) = 85.00

Surface Area (ha)=	0.55	0.15
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	65.83	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED ---		HYETOGRAPH ---	
------	--	------	--	---------------------	--	----------------	--

Unit Hyd. peak (cms)= 0.31 0.15
 PEAK FLOW (cms)= 0.14 0.02 0.155 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 75.00 52.87 71.68
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN¹ = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20092) Area (ha)= 0.63
 ID= 1 DT= 5.0 min Total Imp (%) = 81.00 Dir. Conn. (%) = 81.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.51 0.12
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 64.81 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	3.04	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)= 100.32 82.63
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.97 (ii) 6.28 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.31 0.15

PEAK FLOW (cms)= 0.14 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.166 (iii)
 RUNOFF VOLUME (mm)= 75.00 52.87 6.00
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.93

CALIB
 STANDHYD (200812) Area (ha)= 0.38
 ID= 1 DT= 5.0 min Total Imp (%) = 70.00 Dir. Conn. (%) = 70.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.27 0.11
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 50.33 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)= 100.32 82.63
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.69 (ii) 10.02 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.10

PEAK FLOW (cms)= 0.07 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.091 (iii)
 RUNOFF VOLUME (mm)= 75.00 52.87 68.35
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN¹ = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (200811) Area (ha)= 0.07
 ID= 1 DT= 5.0 min Total Imp (%) = 84.00 Dir. Conn. (%) = 84.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.06 0.01

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN¹ = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20091) Area (ha)= 0.40
 ID= 1 DT= 5.0 min Total Imp (%) = 77.00 Dir. Conn. (%) = 77.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.31 0.05
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 51.64 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	6.08	9.58	2.66
0.667	1.90	3.667	3.04	6.667	6.08	9.67	2.66
0.750	1.90	3.750	3.04	6.750	6.08	9.75	2.66
0.833	1.90	3.833	3.04	6.833	6.08	9.83	2.66
0.917	1.90	3.917	3.04	6.917	6.08	9.92	2.66
1.000	1.90	4.000	3.04	7.000	6.08	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max. Eff. Inten. (mm/hr)= 100.32 82.63
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.92 (ii) 6.07 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.14

PEAK FLOW (cms)= 0.09 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.104 (iii)
 RUNOFF VOLUME (mm)= 75.00 52.87 69.90
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN¹ = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Dep. Storage (mm)= 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 21.60 40.00
 Mannings n = 0.013 0.290

NOTE: R

ID= 1 DT= 5.0 min | Total Imp(%)= 59.00 Dir. Conn.(%)= 59.00

Surface Area (ha)= 0.23 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 0.16
Average Slope (%)= 1.00 2.00
Length (m)= 50.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 10 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr)= 100.32 82.63
Storage Coeff. (min)= 1.70 (ii) 10.03 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.10

TOTALS*
PEAK FLOW (cms)= 0.06 0.03
TIME TO PEAK (hrs)= 6.00 6.08

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (200821) Area (ha)= 0.07 Dir. Conn.(%)= 83.00
Total Imp(%)= 83.00

Surface Area (ha)= 0.06 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 0.01
Average Slope (%)= 1.00 2.00
Length (m)= 21.60 40.00
Mannings n = 0.013 0.290

Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 56.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 10 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr)= 100.32 82.63
Storage Coeff. (min)= 1.80 (ii) 10.13 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.10

TOTALS*
PEAK FLOW (cms)= 0.10 0.02
TIME TO PEAK (hrs)= 6.00 6.08

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20101) Area (ha)= 0.25 Dir. Conn.(%)= 82.00
Total Imp(%)= 82.00

Surface Area (ha)= 0.20 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 0.05
Average Slope (%)= 1.00 2.00
Length (m)= 40.82 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 10 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr)= 100.32 82.63
Storage Coeff. (min)= 1.02 (ii) 5.10 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.34 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD, AREA, QPEAK, TPEAK, R.V. Rows show hydrograph additions.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (20102) Area (ha)= 0.47 Dir. Conn.(%)= 74.00
Total Imp(%)= 74.00

Surface Area (ha)= 0.35 IMPERVIOUS PERVIOUS (i)
0.12

Table with 10 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH, TIME, RAIN. Rows show time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr)= 100.32 82.63
Storage Coeff. (min)= 1.49 (ii) 5.69 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD, AREA, QPEAK, TPEAK, R.V. Rows show hydrograph additions.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (20111) Area (ha)= 0.87 Dir. Conn.(%)= 67.00
Total Imp(%)= 67.00

Surface Area (ha)= 0.58 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 76.16 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000.

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) 5.00 15.00
Storage Coeff. (min)= 2.17 (ii) 10.49 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.09

TOTALS
0.204 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20112) ID= 1 DT= 5.0 min Area (ha)= 0.90 Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

Surface Area (ha)= 0.72 0.18
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 77.46 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000.

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) 5.00 10.00
Storage Coeff. (min)= 2.33 (ii) 6.65 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.14

TOTALS
0.291 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (201021) ID= 1 DT= 5.0 min Area (ha)= 0.47 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.40 0.07
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 1.417.

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.167 to 3.000.

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) 5.00 10.00
Storage Coeff. (min)= 2.19 (ii) 6.61 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.14

TOTALS
0.236 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20122) ID= 1 DT= 5.0 min Area (ha)= 1.11 Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

Surface Area (ha)= 0.90 0.21
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 86.02 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 3.000.

Max.Eff.Inten.(mm/hr)= 100.32 82.63
over (min) 5.00 10.00
Storage Coeff. (min)= 1.80 (ii) 5.65 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

TOTALS
0.126 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (201022) ID= 1 DT= 5.0 min Area (ha)= 0.47 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.40 0.07
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 10 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time from 0.083 to 2.083.

2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
 over (min) 5.00 10.00
 Storage Coeff. (min)= 1.80 (ii) 5.65 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.15
 PEAK FLOW (cms)= 0.11 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.126 (iii)
 RUNOFF VOLUME (mm)= 75.00 52.87 71.67
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (201021)	0.47	0.126	6.00	71.67
+ ID2= 2 (201022)	0.47	0.126	6.00	71.67
ID = 3 (0136)	0.94	0.251	6.00	71.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2005)	Area (ha)= 0.28
ID= 1 DT= 5.0 min	Total Imp(%)= 90.00 Dir. Conn.(%)= 90.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.25 0.03
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 45.20 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52

1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
 over (min) 5.00 5.00
 Storage Coeff. (min)= 1.54 (ii) 4.74 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.33 0.22
 PEAK FLOW (cms)= 0.07 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.00
 RUNOFF VOLUME (mm)= 75.00 52.87 72.78
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2012)	Area (ha)= 1.11
ID= 1 DT= 5.0 min	Total Imp(%)= 56.00 Dir. Conn.(%)= 56.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.62 0.49
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 86.02 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52

2.500	2.28	5.500	9.12	8.500	2.66	11.50	1.52
2.583	2.28	5.583	36.48	8.583	2.66	11.58	1.52
2.667	2.28	5.667	36.48	8.667	2.66	11.67	1.52
2.750	2.28	5.750	36.48	8.750	2.66	11.75	1.52
2.833	2.28	5.833	100.32	8.833	2.66	11.83	1.52
2.917	2.28	5.917	100.32	8.917	2.66	11.92	1.52
3.000	2.28	6.000	100.32	9.000	2.66	12.00	1.52

Max.Eff.Inten.(mm/hr)= 100.32 82.63
 over (min) 5.00 5.00
 Storage Coeff. (min)= 2.33 (ii) 10.66 (ii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.30 0.09
 PEAK FLOW (cms)= 0.17 0.08 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.243 (iii)
 RUNOFF VOLUME (mm)= 75.00 52.87 72.78
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200111)	Area (ha)= 0.10
ID= 1 DT= 5.0 min	Total Imp(%)= 89.50 Dir. Conn.(%)= 89.50

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.09 0.01
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 25.82 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	4.56	7.083	4.56	10.08	1.52
1.167	1.90	4.167	4.56	7.167	4.56	10.17	1.52
1.250	1.90	4.250	4.56	7.250	4.56	10.25	1.52
1.333	1.90	4.333	4.56	7.333	4.56	10.33	1.52
1.417	1.90	4.417	4.56	7.417	4.56	10.42	1.52
1.500	1.90	4.500	4.56	7.500	4.56	10.50	1.52
1.583	1.90	4.583	6.08	7.583	4.56	10.58	1.52
1.667	1.90	4.667	6.08	7.667	4.56	10.67	1.52
1.750	1.90	4.750	6.08	7.750	4.56	10.75	1.52
1.833	1.90	4.833	6.08	7.833	4.56	10.83	1.52
1.917	1.90	4.917	6.08	7.917	4.56	10.92	1.52
2.000	1.90	5.000	6.08	8.000	4.56	11.00	1.52
2.083	2.28	5.083	9.12	8.083	2.66	11.08	1.52
2.167	2.28	5.167	9.12	8.167	2.66	11.17	1.52
2.250	2.28	5.250	9.12	8.250	2.66	11.25	1.52
2.333	2.28	5.333	9.12	8.333	2.66	11.33	1.52
2.417	2.28	5.417	9.12	8.417	2.66	11.42	1.52
2.500	2.2						

RUNOFF VOLUME (mm)= 75.00 52.87 72.11
 TOTAL RAINFALL (mm)= 76.00 76.00 76.00
 RUNOFF COEFFICIENT = 0.99 0.70 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (20011):	0.10	0.027	6.00	72.67
+ ID2= 2 (20012):	0.11	0.030	6.00	72.11
ID = 3 (0142):	0.21	0.057	6.00	72.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (20012)	Area (ha)= 0.10	Dir. Conn.(%)= 79.00
ID= 1 DT= 5.0 min	Total Imp (%)= 79.00	

Surface Area (ha)=	0.08	PERVIOUS (i)
Dep. Storage (mm)=	1.00	0.02
Average Slope (%)=	1.00	2.00
Length (m)=	25.69	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	3.04	7.083	13.68	10.08	1.52
1.167	1.90	4.167	3.04	7.167	13.68	10.17	1.52
1.250	1.90	4.250	3.04	7.250	13.68	10.25	1.52
1.333	1.90	4.333	3.04	7.333	13.68	10.33	1.52
1.417	1.90	4.417	3.04	7.417	13.68	10.42	1.52
1.500	1.90	4.500	3.04	7.500	13.68	10.50	1.52
1.583	1.90	4.583	3.04	7.583	13.68	10.58	1.52
1.667	1.90	4.667	3.04	7.667	13.68	10.67	1.52
1.750	1.90	4.750	3.04	7.750	13.68	10.75	1.52
1.833	1.90	4.833	3.04	7.833	13.68	10.83	1.52
1.917	1.90	4.917	3.04	7.917	13.68	10.92	1.52
2.000	1.90	5.000	3.04	8.000	13.68	11.00	1.52
2.083	2.28	5.083	3.04	8.083	13.68	11.08	1.52
2.167	2.28	5.167	3.04	8.167	13.68	11.17	1.52
2.250	2.28	5.250	3.04	8.250	13.68	11.25	1.52
2.333	2.28	5.333	3.04	8.333	13.68	11.33	1.52
2.417	2.28	5.417	3.04	8.417	13.68	11.42	1.52
2.500	2.28	5.500	3.04	8.500	13.68	11.50	1.52
2.583	2.28	5.583	3.04	8.583	13.68	11.58	1.52
2.667	2.28	5.667	3.04	8.667	13.68	11.67	1.52
2.750	2.28	5.750	3.04	8.750	13.68	11.75	1.52
2.833	2.28	5.833	3.04	8.833	13.68	11.83	1.52
2.917	2.28	5.917	3.04	8.917	13.68	11.92	1.52
3.000	2.28	6.000	3.04	9.000	13.68	12.00	1.52

Max. Eff. Inten. (mm/hr)=	100.32	82.63
over (min)=	5.00	10.00
Storage Coeff. (min)=	1.13 (ii)	5.66 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00

Unit Hyd. peak (cms)=	0.34	0.15	*TOTALS*
PEAK FLOW (cms)=	0.02	0.00	0.026 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	75.00	52.87	70.32
TOTAL RAINFALL (mm)=	76.00	76.00	76.00
RUNOFF COEFFICIENT =	0.99	0.70	0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20012)	Area (ha)= 0.13	Dir. Conn.(%)= 86.00
ID= 1 DT= 5.0 min	Total Imp (%)= 86.00	

Surface Area (ha)=	0.11	PERVIOUS (i)
Dep. Storage (mm)=	1.00	0.02
Average Slope (%)=	1.00	2.00
Length (m)=	29.44	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.90	3.083	3.04	6.083	13.68	9.08	2.66
0.167	1.90	3.167	3.04	6.167	13.68	9.17	2.66
0.250	1.90	3.250	3.04	6.250	13.68	9.25	2.66
0.333	1.90	3.333	3.04	6.333	13.68	9.33	2.66
0.417	1.90	3.417	3.04	6.417	13.68	9.42	2.66
0.500	1.90	3.500	3.04	6.500	13.68	9.50	2.66
0.583	1.90	3.583	3.04	6.583	13.68	9.58	2.66
0.667	1.90	3.667	3.04	6.667	13.68	9.67	2.66
0.750	1.90	3.750	3.04	6.750	13.68	9.75	2.66
0.833	1.90	3.833	3.04	6.833	13.68	9.83	2.66
0.917	1.90	3.917	3.04	6.917	13.68	9.92	2.66
1.000	1.90	4.000	3.04	7.000	13.68	10.00	2.66
1.083	1.90	4.083	3.04	7.083	13.68	10.08	1.52
1.167	1.90	4.167	3.04	7.167	13.68	10.17	1.52
1.250	1.90	4.250	3.04	7.250	13.68	10.25	1.52
1.333	1.90	4.333	3.04	7.333	13.68	10.33	1.52
1.417	1.90	4.417	3.04	7.417	13.68	10.42	1.52
1.500	1.90	4.500	3.04	7.500	13.68	10.50	1.52
1.583	1.90	4.583	3.04	7.583	13.68	10.58	1.52
1.667	1.90	4.667	3.04	7.667	13.68	10.67	1.52
1.750	1.90	4.750	3.04	7.750	13.68	10.75	1.52
1.833	1.90	4.833	3.04	7.833	13.68	10.83	1.52
1.917	1.90	4.917	3.04	7.917	13.68	10.92	1.52
2.000	1.90	5.000	3.04	8.000	13.68	11.00	1.52
2.083	2.28	5.083	3.04	8.083	13.68	11.08	1.52
2.167	2.28	5.167	3.04	8.167	13.68	11.17	1.52
2.250	2.28	5.250	3.04	8.250	13.68	11.25	1.52
2.333	2.28	5.333	3.04	8.333	13.68	11.33	1.52
2.417	2.28	5.417	3.04	8.417	13.68	11.42	1.52
2.500	2.28	5.500	3.04	8.500	13.68	11.50	1.52
2.583	2.28	5.583	3.04	8.583	13.68	11.58	1.52
2.667	2.28	5.667	3.04	8.667	13.68	11.67	1.52
2.750	2.28	5.750	3.04	8.750	13.68	11.75	1.52
2.833	2.28	5.833	3.04	8.833	13.68	11.83	1.52
2.917	2.28	5.917	3.04	8.917	13.68	11.92	1.52
3.000	2.28	6.000	3.04	9.000	13.68	12.00	1.52

Max. Eff. Inten. (mm/hr)=	100.32	82.63
over (min)=	5.00	10.00
Storage Coeff. (min)=	1.23 (ii)	4.95 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.33	0.22

PEAK FLOW (cms)=	0.03	0.00	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.035 (iii)
RUNOFF VOLUME (mm)=	75.00	52.87	71.89
TOTAL RAINFALL (mm)=	76.00	76.00	76.00
RUNOFF COEFFICIENT =	0.99	0.70	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (20012):	0.10	0.026	6.00	70.32
+ ID2= 2 (20012):	0.13	0.035	6.00	71.89
ID = 3 (0145):	0.23	0.061	6.00	71.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L	(v 6.1.2002)
V V I SS U U A A L	
V V I SS U U A A A L	
V V I SSS UUUU A A LLLL	
OOO TTTT TTTT H H Y Y M M OOO TM	
O O T T H H Y Y M M O O	
O O T T H H Y Y M M O O	
OOO T T H H Y Y M M OOO	

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2020 Smart City Water Inc
 All rights reserved.

***** DETAILED OUTPUT *****

Input filename: c:\Program Files (x86)\Visual OTTHM 6.1\VO2\vo1n.dat
 Output filename: c:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\d5d7529a
 Summary filename: c:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\d5d7529a

DATE: 05-18-2022 TIME: 12:51:00
 USER:
 COMMENTS:

** SIMULATION : Run 16 **

READ STORM	Filename: C:\Users\caef070146\AppData\Local\Temp\83a8b910-d519-48ad-be84-5b44b6daecc3\2d33b8b1						
Ptotal= 90.70 mm	Comments: 25-yr,12-hr SCS, City of Barrie						
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	2.27	3.25	3.63	6.25	16.33	9.25	3.17
0.50	2.27	3.50	3.63	6.50	16.33	9.50	3.17
0.75	2.27	3.75	3.63	6.75	16.33	9.75	3.17
1.00	2.27	4.00	3.63	7.00	16.33	10.00	3.17
1.25	2.27	4.25	3.63	7.25	16.33	10.25	1.81
1.50	2.27	4.50	3.63	7.50	16.33	10.50	1.81
1.75	2.27	4.75	3.63	7.75	16.33	10.75	1.81
2.00	2.27	5.00	3.63	8.00	16.33	11.00	1.81
2.25	2.27	5.25	3.63	8.25	16.33	11.25	1.81
2.50	2.27	5.50	3.63	8.50	16.33	11.50	1.81
2.75	2.27	5.75	3.63	8.75	16.33	11.75	1.81
3.00	2.27	6.00	3.63	9.00	16.33	12.00	1.81

CALIB STANDBYD (20022)	Area (ha)= 0.80	Dir. Conn.(%)= 85.00
ID= 1 DT= 5.0 min	Total Imp (%)= 85.00	

Surface Area (ha)=	0.68	PERVIOUS (i)
Dep. Storage (mm)=	1.00	0.12
Average Slope (%)=	1.00	1.00
Length (m)=	73.03	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.3		

Average Slope (%)= 1.00 2.00
 Length (m)= 68.31 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.083	3.63	6.083	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	3.63	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.72	5.583	10.88	8.583	3.17	11.58	1.81
2.667	2.72	5.667	10.88	8.667	3.17	11.67	1.81
2.750	2.72	5.750	10.88	8.750	3.17	11.75	1.81
2.833	2.72	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.72	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.72	6.000	119.72	9.000	3.17	12.00	1.81

Max.Eff.Inten.(mm/hr)= 119.72 102.72
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.89 (ii) 5.00 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.21 0.02 *TOTALS* 0.227 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 89.70 66.45 87.14
 TOTAL RAINFALL (mm)= 90.70 90.70 90.70
 RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20031)	Area (ha)=	Dir. Conn.(%)=
		0.43	90.00
	ID= 1 DT= 5.0 min	Total Imp(%)= 90.00	

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	0.39	0.04
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	53.54	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.167	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.250	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.333	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.417	2.27	3.917	3.63	6.917	7.26	9.92	3.17
0.500	2.27	4.000	3.63	7.000	7.26	10.00	3.17
0.583	2.27	4.083	3.63	7.083	5.44	10.08	1.81
0.667	2.27	4.167	5.44	7.167	5.44	10.17	1.81
0.750	2.27	4.250	5.44	7.250	5.44	10.25	1.81
0.833	2.27	4.333	5.44	7.333	5.44	10.33	1.81
0.917	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.000	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.083	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.167	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.250	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.333	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.417	2.27	4.917	7.26	7.917	5.44	10.92	1.81
1.500	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.72	5.583	10.88	8.583	3.17	11.58	1.81
2.667	2.72	5.667	10.88	8.667	3.17	11.67	1.81
2.750	2.72	5.750	10.88	8.750	3.17	11.75	1.81
2.833	2.72	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.72	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.72	6.000	119.72	9.000	3.17	12.00	1.81

Max.Eff.Inten.(mm/hr)= 119.72 102.72
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.68 (ii) 5.03 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.14 0.02 *TOTALS* 0.152 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 89.70 66.45 86.67
 TOTAL RAINFALL (mm)= 90.70 90.70 90.70
 RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20041)	Area (ha)=	Dir. Conn.(%)=
		0.35	89.00
	ID= 1 DT= 5.0 min	Total Imp(%)= 89.00	

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	0.31	0.04
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	48.30	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.72	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.72	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.72	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.72	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.72	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.72	6.000	119.72	9.000	3.17	12.00	1.81

Max.Eff.Inten.(mm/hr)= 119.72 102.72
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.63 (ii) 4.62 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.13 0.01 *TOTALS* 0.141 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 89.70 66.45

1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.72	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.72	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.72	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.72	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.72	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.72	6.000	119.72	9.000	3.17	12.00	1.81

Max.Eff.Inten.(mm/hr)= 119.72 102.72
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.55 (ii) 5.14 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.10 0.01 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.116 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 86.20
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)=	0.29
STANDHYD (20052)	Total Imp (%)=	78.00
ID= 1 DT= 5.0 min	Dir. Conn. (%)=	78.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.23	0.06	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	43.97	40.00	
Mannings n =	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	3.63	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81

Storage Coeff. (min)=	5.00	10.00
Unit Hyd. Tpeak (min)=	1.92 (ii)	6.04 (ii)
Unit Hyd. peak (cms)=	5.00	10.00
Unit Hyd. peak (cms)=	0.31	0.15

PEAK FLOW (cms)= 0.19 0.04 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.231 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 85.05
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)=	0.65
STANDHYD (20061)	Total Imp (%)=	85.00
ID= 1 DT= 5.0 min	Dir. Conn. (%)=	85.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.55	0.10	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	65.83	40.00	
Mannings n =	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.72	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.72	5.500	10.88	8.500	3.17	11.50	1.81

Storage Coeff. (min)=	1.85 (ii)	5.44 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.32	0.16

PEAK FLOW (cms)= 0.18 0.02 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.209 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 86.21

2.583	2.72	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.72	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.72	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.72	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.72	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.72	6.000	119.72	9.000	3.17	12.00	1.81

Max.Eff.Inten.(mm/hr)= 119.72 102.72
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.45 (ii) 5.78 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.08 0.02 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.00 (iii)
RUNOFF VOLUME (mm)= 89.70 66.45 84.57
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)=	0.73
STANDHYD (20062)	Total Imp (%)=	80.00
ID= 1 DT= 5.0 min	Dir. Conn. (%)=	80.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.58	0.15	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	69.76	40.00	
Mannings n =	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.72	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.72	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.72	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.72	5.333	10.88	8.333	3.17		

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20071)
ID= 1 DT= 5.0 min

Area (ha)= 0.58
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.49 0.09
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.18 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TRANSFORMED (hrs, mm/hr), HYETOGRAPH (hrs, mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data from 0.083 to 3.000 hrs.

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) 5.00 10.00
Storage Coeff. (min)= 1.79 (ii) 5.37 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

TOTALS

PEAK FLOW (cms)= 0.16 0.02 0.186 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 89.70 66.45 86.21
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20092)
ID= 1 DT= 5.0 min

Area (ha)= 0.63
Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.31 0.09
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 51.64 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TRANSFORMED (hrs, mm/hr), HYETOGRAPH (hrs, mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data from 0.083 to 3.000 hrs.

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) 5.00 10.00
Storage Coeff. (min)= 6.03 (ii) 6.03 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

TOTALS

PEAK FLOW (cms)= 0.10 0.02 0.125 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 89.70 66.45 84.34
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (200812)
ID= 1 DT= 5.0 min

Area (ha)= 0.38
Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.27 0.1
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TRANSFORMED (hrs, mm/hr), HYETOGRAPH (hrs, mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data from 0.083 to 0.167 hrs.

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.51 0.12
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 64.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TRANSFORMED (hrs, mm/hr), HYETOGRAPH (hrs, mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data from 0.083 to 3.000 hrs.

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) 5.00 10.00
Storage Coeff. (min)= 1.83 (ii) 5.85 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

TOTALS

PEAK FLOW (cms)= 0.17 0.03 0.200 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 89.70 66.45 85.27
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20091)
ID= 1 DT= 5.0 min

Area (ha)= 0.40
Total Imp(%)= 77.00 Dir. Conn.(%)= 77.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.31 0.09
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 51.64 40.00
Mannings n = 0.013 0.290

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TRANSFORMED (hrs, mm/hr), HYETOGRAPH (hrs, mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data from 0.250 to 3.000 hrs.

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) 5.00 10.00
Storage Coeff. (min)= 1.57 (ii) 9.21 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.12

TOTALS

PEAK FLOW (cms)= 0.09 0.03 0.114 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 89.70 66.45 82.71
TOTAL RAINFALL (mm)= 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (200811)
ID= 1 DT= 5.0 min

Area (ha)= 0.07
Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.06 0.01
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 21.60 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TRANSFORMED (hrs, mm/hr), HYETOGRAPH (hrs, mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data from 0.083 to 0.833 hrs.

Hydrology data table with columns for time (hrs), rain (mm/hr), and various flow metrics (0.917, 1.000, 1.083, etc.).

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 5.00
Storage Coeff. (min)= 0.95 (ii) 4.76 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.22
TOTALS
PEAK FLOW (cms)= 0.02 0.00 0.023 (iii)
TIME TO PEAK (hrs)= 5.92 6.00 6.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110) table with columns for ID, AREA, QPEAK, TPEAK, R.V. (mm) and values for ID1, ID2, ID3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200822) table with Area (ha)= 0.39, Total Imp(%)= 59.00, Dir. Conn.(%)= 59.00.

Surface Area (ha)= IMPERVIOUS 0.23 PERVIOUS (i) 0.16
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns for TIME, RAIN, and multiple rows of data.

Hydrology data table with columns for time (hrs), rain (mm/hr), and various flow metrics (1.250, 1.333, 1.500, etc.).

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 5.00
Storage Coeff. (min)= 0.95 (ii) 4.76 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.22
TOTALS
PEAK FLOW (cms)= 0.02 0.00 0.023 (iii)
TIME TO PEAK (hrs)= 5.92 6.00 6.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0111) table with columns for ID, AREA, QPEAK, TPEAK, R.V. (mm) and values for ID1, ID2, ID3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (20102) table with Area (ha)= 0.47, Total Imp(%)= 74.00, Dir. Conn.(%)= 74.00.

Surface Area (ha)= IMPERVIOUS 0.35 PERVIOUS (i) 0.12
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns for TIME, RAIN, and multiple rows of data.

Hydrology data table with columns for time (hrs), rain (mm/hr), and various flow metrics (0.583, 0.667, 0.750, etc.).

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.59 (ii) 9.22 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.12
TOTALS
PEAK FLOW (cms)= 0.08 0.04 0.112 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200821) table with Area (ha)= 0.07, Total Imp(%)= 83.00, Dir. Conn.(%)= 83.00.

Surface Area (ha)= IMPERVIOUS 0.06 PERVIOUS (i) 0.01
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 21.60 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns for TIME, RAIN, and multiple rows of data.

Hydrology data table with columns for time (hrs), rain (mm/hr), and various flow metrics (0.917, 1.000, 1.083, etc.).

Max. Eff. Inten. (mm/hr)= 119.72 102.72
over (min) = 5.00 10.00
Storage Coeff. (min)= 1.68 (i) 6.40 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.14
TOTALS
PEAK FLOW (cms)= 0.12 0.03 0.146 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20101) table with Area (ha)= 0.25, Total Imp(%)= 82.00, Dir. Conn.(%)= 82.00.

Surface Area (ha)= IMPERVIOUS 0.20 PERVIOUS (i) 0.05
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 40.82 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns for TIME, RAIN, and multiple rows of data.

1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72
over (min)= 5.00
Storage Coeff. (min)= 1.39 (ii)
Unit Hyd. Tpeak (min)= 5.30 (ii)
Unit Hyd. peak (cms)= 0.33

PEAK FLOW (cms)= 0.07
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 89.70
TOTAL RAINFALL (mm)= 90.70
RUNOFF COEFFICIENT = 0.99

TOTALS
0.080 (iii)
6.00
85.50
90.70
0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0121)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (20101):	0.25	0.080	6.00	85.50
+ ID2 = 2 (20102):	0.47	0.146	6.00	83.64
ID = 3 (0121):	0.72	0.225	6.00	84.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2011)	Area (ha)= 0.87
ID= 1 DT= 5.0 min	Total Imp(%) = 67.00 Dir. Conn.(%) = 67.00

Surface Area (ha)= 0.58
Dep. Storage (mm)= 1.00
Average Slope (%) = 1.00
Length (m) = 76.16
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17
0.667	2.27	3.667	3.63	6.667	16.33	9.67	3.17
0.750	2.27	3.750	3.63	6.750	16.33	9.75	3.17
0.833	2.27	3.833	3.63	6.833	16.33	9.83	3.17
0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81

1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.933	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72
over (min)= 5.00
Storage Coeff. (min)= 2.04 (ii)
Unit Hyd. Tpeak (min)= 6.16 (ii)
Unit Hyd. peak (cms)= 0.31

PEAK FLOW (cms)= 0.24
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 89.70
TOTAL RAINFALL (mm)= 90.70
RUNOFF COEFFICIENT = 0.99

TOTALS
0.284 (iii)
6.00
85.05
90.70
0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20122)	Area (ha)= 1.11
ID= 1 DT= 5.0 min	Total Imp(%) = 81.00 Dir. Conn.(%) = 81.00

Surface Area (ha)= 0.90
Dep. Storage (mm)= 1.00
Average Slope (%) = 1.00
Length (m) = 80.42
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17
0.667	2.27	3.667	3.63	6.667	16.33	9.67	3.17
0.750	2.27	3.750	3.63	6.750	16.33	9.75	3.17
0.833	2.27	3.833	3.63	6.833	16.33	9.83	3.17
0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81

1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr)= 119.72
over (min)= 5.00
Storage Coeff. (min)= 2.02 (ii)
Unit Hyd. Tpeak (min)= 6.19 (ii)
Unit Hyd. peak (cms)= 0.31

PEAK FLOW (cms)= 0.19
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 89.70
TOTAL RAINFALL (mm)= 90.70
RUNOFF COEFFICIENT = 0.99

TOTALS (iii)
0.216
6.00
82.02
90.70
0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20112)	Area (ha)= 0.90
ID= 1 DT= 5.0 min	Total Imp(%) = 80.00 Dir. Conn.(%) = 80.00

Surface Area (ha)= 0.72
Dep. Storage (mm)= 1.00
Average Slope (%) = 1.00
Length (m) = 77.46
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	16.33	9.58	3.17
0.667	2.27	3.667	3.63	6.667	16.33	9.67	3.17
0.750	2.27	3.750	3.63	6.750	16.33	9.75	3.17
0.833	2.27	3.833	3.63	6.833	16.33	9.83	3.17
0.917	2.27	3.917	3.63	6.917	16.33	9.92	3.17
1.000	2.27	4.000	3.63	7.000	16.33	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27						

over (min) = 5.00 10.00
Storage Coeff. (min) = 1.68 (ii) 5.27 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.16

TOTALS
PEAK FLOW (cms) = 0.13 0.02 0.151 (iii)
TIME TO PEAK (hrs) = 6.00 6.00 6.00
RUNOFF VOLUME (mm) = 89.70 66.45 86.21
TOTAL RAINFALL (mm) = 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (201022)
ID= 1 DT= 5.0 min
Area (ha) = 0.47
Total Imp (%) = 85.00 Dir. Conn. (%) = 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.40 0.07
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr) = 119.72 102.72 5.00
over (min) = 5.00 10.00
Storage Coeff. (min) = 1.68 (ii) 5.27 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.16

TOTALS
PEAK FLOW (cms) = 0.13 0.02 0.151 (iii)
TIME TO PEAK (hrs) = 6.00 6.00 6.00
RUNOFF VOLUME (mm) = 89.70 66.45 86.21

PEAK FLOW (cms) = 0.08 0.01 0.092 (iii)
TIME TO PEAK (hrs) = 6.00 6.00 6.00
RUNOFF VOLUME (mm) = 89.70 66.45 87.36
TOTAL RAINFALL (mm) = 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20121)
ID= 1 DT= 5.0 min
Area (ha) = 1.11
Total Imp (%) = 56.00 Dir. Conn. (%) = 56.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.62 0.49
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 86.02 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr) = 119.72 102.72 5.00
over (min) = 5.00 10.00
Storage Coeff. (min) = 2.17 (ii) 9.80 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.31 0.11

TOTALS
PEAK FLOW (cms) = 0.21 0.11 0.312 (iii)
TIME TO PEAK (hrs) = 6.00 6.00 6.00
RUNOFF VOLUME (mm) = 89.70 66.45 79.46
TOTAL RAINFALL (mm) = 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

TOTAL RAINFALL (mm) = 90.70 90.70 90.70
RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136)
1 + 2 = 3
AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1 = 1 (201021): 0.47 0.151 6.00 86.21
+ ID2 = 2 (201022): 0.47 0.151 6.00 86.21
===== ID = 3 (0136): 0.94 0.302 6.00 86.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2005)
ID= 1 DT= 5.0 min
Area (ha) = 0.28
Total Imp (%) = 90.00 Dir. Conn. (%) = 90.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.25 0.03
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 43.20 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17
0.167	2.27	3.167	3.63	6.167	16.33	9.17	3.17
0.250	2.27	3.250	3.63	6.250	16.33	9.25	3.17
0.333	2.27	3.333	3.63	6.333	16.33	9.33	3.17
0.417	2.27	3.417	3.63	6.417	16.33	9.42	3.17
0.500	2.27	3.500	3.63	6.500	16.33	9.50	3.17
0.583	2.27	3.583	3.63	6.583	7.26	9.58	3.17
0.667	2.27	3.667	3.63	6.667	7.26	9.67	3.17
0.750	2.27	3.750	3.63	6.750	7.26	9.75	3.17
0.833	2.27	3.833	3.63	6.833	7.26	9.83	3.17
0.917	2.27	3.917	3.63	6.917	7.26	9.92	3.17
1.000	2.27	4.000	3.63	7.000	7.26	10.00	3.17
1.083	2.27	4.083	5.44	7.083	5.44	10.08	1.81
1.167	2.27	4.167	5.44	7.167	5.44	10.17	1.81
1.250	2.27	4.250	5.44	7.250	5.44	10.25	1.81
1.333	2.27	4.333	5.44	7.333	5.44	10.33	1.81
1.417	2.27	4.417	5.44	7.417	5.44	10.42	1.81
1.500	2.27	4.500	5.44	7.500	5.44	10.50	1.81
1.583	2.27	4.583	7.26	7.583	5.44	10.58	1.81
1.667	2.27	4.667	7.26	7.667	5.44	10.67	1.81
1.750	2.27	4.750	7.26	7.750	5.44	10.75	1.81
1.833	2.27	4.833	7.26	7.833	5.44	10.83	1.81
1.917	2.27	4.917	7.26	7.917	5.44	10.92	1.81
2.000	2.27	5.000	7.26	8.000	5.44	11.00	1.81
2.083	2.27	5.083	10.88	8.083	3.17	11.08	1.81
2.167	2.27	5.167	10.88	8.167	3.17	11.17	1.81
2.250	2.27	5.250	10.88	8.250	3.17	11.25	1.81
2.333	2.27	5.333	10.88	8.333	3.17	11.33	1.81
2.417	2.27	5.417	10.88	8.417	3.17	11.42	1.81
2.500	2.27	5.500	10.88	8.500	3.17	11.50	1.81
2.583	2.27	5.583	43.54	8.583	3.17	11.58	1.81
2.667	2.27	5.667	43.54	8.667	3.17	11.67	1.81
2.750	2.27	5.750	43.54	8.750	3.17	11.75	1.81
2.833	2.27	5.833	119.72	8.833	3.17	11.83	1.81
2.917	2.27	5.917	119.72	8.917	3.17	11.92	1.81
3.000	2.27	6.000	119.72	9.000	3.17	12.00	1.81

Max. Eff. Inten. (mm/hr) = 119.72 102.72 5.00
over (min) = 5.00 10.00
Storage Coeff. (min) = 1.44 (ii) 4.42 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.33 0.23

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (200111)
ID= 1 DT= 5.0 min
Area (ha) = 0.10
Total Imp (%) = 89.50 Dir. Conn. (%) = 89.50

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.09 0.01
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 25.82 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.27	3.083	3.63	6.083	16.33	9.08	3.17

CALIB STANDHYD (200121) ID=1 DT= 5.0 min Area (ha)= 0.11 Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.10 0.01 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 27.08 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 12 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 119.72 over (min)= 5.00 Storage Coeff. (min)= 1.09 (ii) Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.34 PEAK FLOW (cms)= 0.03 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 89.70 TOTAL RAINFALL (mm)= 90.70 RUNOFF COEFFICIENT = 0.99

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.11 0.02 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 29.44 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 12 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 119.72 over (min)= 5.00 Storage Coeff. (min)= 1.14 (ii) Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.34 PEAK FLOW (cms)= 0.04 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 89.70 TOTAL RAINFALL (mm)= 90.70 RUNOFF COEFFICIENT = 0.99

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) ID1= 1 (200112): 0.10 0.031 6.00 84.78 ID2= 2 (200122): 0.13 0.042 6.00 86.43 ID = 3 (0145): 0.23 0.074 6.00 85.72

ID = 3 (0142): 0.21 0.069 6.00 86.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200112) ID=1 DT= 5.0 min Area (ha)= 0.10 Total Imp(%)= 79.00 Dir. Conn.(%)= 79.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.08 0.02 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 25.69 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 12 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show rainfall intensity over time from 0.083 to 3.000 hours.

Max. Eff. Inten. (mm/hr)= 119.72 over (min)= 5.00 Storage Coeff. (min)= 1.05 (ii) Unit Hyd. Tpeak (min)= 5.00 Unit Hyd. peak (cms)= 0.34 PEAK FLOW (cms)= 0.03 TIME TO PEAK (hrs)= 6.00 RUNOFF VOLUME (mm)= 89.70 TOTAL RAINFALL (mm)= 90.70 RUNOFF COEFFICIENT = 0.99

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Calibration summary for ID=1 DT= 5.0 min, Area=0.13, Dir. Conn.=86.00. Includes a table for ADD HYD (0142) with 1+2=3.

DETAILED OUTPUT: Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.1\VO2\voin.dat. Output filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\664943f0. Summary filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\664943f0.

READ STORM: Filename: C:\Users\caef070146\AppData\Local\Temp\ata\Local\Temp\3a8b910-d519-48ad-be84-5b44b6deacc3\9894c1cd. Ptotal=101.70 mm. Comments: 50-Yr, 12-hr SCS, City of Barrie.

CALIB STANDHYD (20022) ID=1 DT= 5.0 min Area (ha)= 0.80 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00. IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.68 0.12 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 73.03 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

Table with 12 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show rainfall intensity over time from 0.083 to 0.250 hours.

0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.166	2.54	4.166	6.10	7.166	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	8.14	10.58	2.03
1.667	2.54	4.667	8.14	7.667	8.14	10.67	2.03
1.750	2.54	4.750	8.14	7.750	8.14	10.75	2.03
1.833	2.54	4.833	8.14	7.833	8.14	10.83	2.03
1.917	2.54	4.917	8.14	7.917	8.14	10.92	2.03
2.000	2.54	5.000	8.14	8.000	8.14	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min) 5.00 10.00
Storage Coeff (min)= 0.88 (ii) 5.31 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.25 0.04 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.257 (iii)
RUNOFF VOLUME (mm)= 100.70 76.77 97.11
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20021)	Area (ha)= 0.70
ID= 1	DT= 5.0 min	Total Imp(%)= 89.00

Surface Area (ha)=	0.62	PERVIOUS (i)
Dep. Storage (mm)=	1.00	0.08
Average Slope (%)=	1.00	1.00
Length (m)=	68.31	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56

1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min) 5.00 5.00
Storage Coeff (min)= 1.56 (ii) 4.44 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.23

PEAK FLOW (cms)= 0.14 0.01 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 98.30
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20032)	Area (ha)= 0.47
ID= 1	DT= 5.0 min	Total Imp(%)= 87.00

Surface Area (ha)=	0.41	PERVIOUS (i)
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	55.98	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56

1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03

1.000	2.54	4.000	4.07	7.000	8.14	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	8.14	10.58	2.03
1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min) 5.00 5.00
Storage Coeff (min)= 1.81 (ii) 4.78 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.23 0.02 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.257 (iii)
RUNOFF VOLUME (mm)= 100.70 76.77 98.06
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20031)	Area (ha)= 0.43
ID= 1	DT= 5.0 min	Total Imp(%)= 90.00

Surface Area (ha)=	0.39	PERVIOUS (i)
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	53.54	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56

2.333	3.05	5.333	12.20	8.333	3.56	11.33</
-------	------	-------	-------	-------	------	---------

3.000 3.05 | 6.000 134.24 | 9.000 3.56 | 12.00 2.03

Max. Eff. Inten. (mm/hr)= 134.24 117.80
over (min) 5.00 5.00
Storage Coeff (min)= 1.47 (ii) 4.44 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.23

TOTALS
PEAK FLOW (cms)= 0.12 0.01 0.128 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 98.06
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20042) Area (ha)= 0.36
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.31 IMPERVIOUS 0.01 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 4 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH. Rows show time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr)= 134.24 117.80
over (min) 5.00 5.00
Storage Coeff (min)= 1.48 (ii) 4.90 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.22

TOTALS

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20062) Area (ha)= 0.73
ID= 1 DT= 5.0 min Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

Surface Area (ha)= 0.58 IMPERVIOUS 0.15 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 69.76 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 4 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH. Rows show time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr)= 134.24 117.80
over (min) 5.00 10.00
Storage Coeff (min)= 1.83 (ii) 5.77 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

TOTALS

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

PEAK FLOW (cms)= 0.11 0.02 0.133 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 97.11
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20052) Area (ha)= 0.29
ID= 1 DT= 5.0 min Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

Surface Area (ha)= 0.23 IMPERVIOUS 0.06 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 43.97 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 4 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH. Rows show time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr)= 134.24 117.80
over (min) 5.00 10.00
Storage Coeff (min)= 1.39 (ii) 5.52 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

TOTALS

- PEAK FLOW (cms)= 0.08 0.02 0.103 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 95.43
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

CALIB STANDBYD (20061) Area (ha)= 0.65
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.55 IMPERVIOUS 0.15 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 65.83 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 4 columns: TIME, RAIN, TRANSFORMED, HYETOGRAPH. Rows show time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr)= 134.24 117.80
over (min) 5.00 10.00
Storage Coeff (min)= 1.77 (ii) 5.19 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

TOTALS

- PEAK FLOW (cms)= 0.21 0.03 0.235 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 97.11
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20072) Area (ha)= 0.64
ID= 1 DT= 5.0 min Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

Surface Area (ha)= 0.54 IMPERVIOUS 0.15 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00

Length (m) = 65.32 40.00
Mannings n = 0.13 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes data for various time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr) over (min) = 134.24 10.00 117.80 5.00
Storage Coeff. (min) = 1.76 (ii) 5.29 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20071) Area (ha) = 0.58 Total Imp (%) = 85.00 Dir. Conn. (%) = 85.00

Surface Area (ha) = 0.49 IMPERVIOUS 0.09 PERVIOUS (i) 0.00
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 64.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes data for various time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr) over (min) = 134.24 10.00 117.80 5.00
Storage Coeff. (min) = 1.75 (ii) 5.59 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20091) Area (ha) = 0.40 Total Imp (%) = 77.00 Dir. Conn. (%) = 77.00

Surface Area (ha) = 0.31 IMPERVIOUS 0.09 PERVIOUS (i) 0.00
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 51.64 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes data for various time intervals and rainfall amounts.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes data for various time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr) over (min) = 134.24 10.00 117.80 5.00
Storage Coeff. (min) = 1.71 (ii) 5.13 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20092) Area (ha) = 0.63 Total Imp (%) = 81.00 Dir. Conn. (%) = 81.00

Surface Area (ha) = 0.51 IMPERVIOUS 0.11 PERVIOUS (i) 0.00
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 64.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes data for various time intervals and rainfall amounts.

Max. Eff. Inten. (mm/hr) over (min) = 134.24 10.00 117.80 5.00
Storage Coeff. (min) = 1.53 (ii) 5.76 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.33 0.15

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200812) Area (ha) = 0.38 Total Imp (%) = 70.00 Dir. Conn. (%) = 70.00

Surface Area (ha) = 0.27 IMPERVIOUS 0.10 PERVIOUS (i) 0.00
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 50.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes data for various time intervals and rainfall amounts.

2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max. Eff. Inten. (mm/hr)= 134.24 117.80
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.50 (ii) 6.39 (iii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.10 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.131 (iii)
 RUNOFF VOLUME (mm)= 100.70 76.77 6.00
 TOTAL RAINFALL (mm)= 101.70 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 0.07
STANDHYD (200811)	Total Imp(%)= 84.00
ID= 1 DT= 5.0 min	Dir. Conn.(%)= 84.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.06	0.01	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	21.60	40.00	
Mannings n =	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56				
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56				
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56				
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56				
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56				
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56				
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56				
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56				
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56				
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56				
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56				
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56				
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03				
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03				
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03				
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03				
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03				
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03				
1.583	2.54	4.583	6.10	7.583	6.10	10.58	2.03				
1.667	2.54	4.667	6.10	7.667	6.10	10.67	2.03				
1.750	2.54	4.750	6.10	7.750	6.10	10.75	2.03				
1.833	2.54	4.833	6.10	7.833	6.10	10.83	2.03				
1.917	2.54	4.917	6.10	7.917	6.10	10.92	2.03				
2.000	2.54	5.000	6.10	8.000	6.10	11.00	2.03				
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03				
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03				
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03				
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03				
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03				
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03				
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03				

2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max. Eff. Inten. (mm/hr)= 134.24 117.80
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.52 (ii) 8.74 (iii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.12

PEAK FLOW (cms)= 0.09 0.04 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.127 (iii)
 RUNOFF VOLUME (mm)= 100.70 76.77 90.88
 TOTAL RAINFALL (mm)= 101.70 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 0.07
STANDHYD (200821)	Total Imp(%)= 83.00
ID= 1 DT= 5.0 min	Dir. Conn.(%)= 83.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.06	0.01	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	21.60	40.00	
Mannings n =	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56				
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56				
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56				
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56				
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56				
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56				
0.583	2.54	3.583	4.07	6.583	18.31	9.58	3.56				
0.667	2.54	3.667	4.07	6.667	18.31	9.67	3.56				
0.750	2.54	3.750	4.07	6.750	18.31	9.75	3.56				
0.833	2.54	3.833	4.07	6.833	18.31	9.83	3.56				
0.917	2.54	3.917	4.07	6.917	18.31	9.92	3.56				
1.000	2.54	4.000	4.07	7.000	18.31	10.00	3.56				
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03				
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03				
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03				
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03				
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03				
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03				
1.583	2.54	4.583	6.10	7.583	6.10	10.58	2.03				
1.667	2.54	4.667	6.10	7.667	6.10	10.67	2.03				
1.750	2.54	4.750	6.10	7.750	6.10	10.75	2.03				
1.833	2.54	4.833	6.10	7.833	6.10	10.83	2.03				
1.917	2.54	4.917	6.10	7.917	6.10	10.92	2.03				
2.000	2.54	5.000	6.10	8.000	6.10	11.00	2.03				
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03				
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03				
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03				
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03				
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03				
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03				
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03				
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03				
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03				
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03				
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03				

2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03

2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max. Eff. Inten. (mm/hr)= 134.24 117.80
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 0.91 (ii) 4.44 (iii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.34 0.23

PEAK FLOW (cms)= 0.02 0.00 *TOTALS*
 TIME TO PEAK (hrs)= 5.92 6.00 0.025 (iii)
 RUNOFF VOLUME (mm)= 100.70 76.77 96.62
 TOTAL RAINFALL (mm)= 101.70 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 0.39
STANDHYD (200822)	Total Imp(%)= 59.00
ID= 1 DT= 5.0 min	Dir. Conn.(%)= 59.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.23	0.16	
Dep. Storage (mm)=	1.00	1.00	
Average Slope (%)=</			

2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)=	134.24	117.80	
over (min)	5.00	10.00	
Storage Coeff. (min)=	1.60 (ii)	6.12 (ii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.32	0.15	
PEAK FLOW (cms)=	0.13	0.03	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.291 (iii)
RUNOFF VOLUME (mm)=	100.70	76.77	92.80
TOTAL RAINFALL (mm)=	101.70	101.70	101.70
RUNOFF COEFFICIENT =	0.99	0.75	0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20101)	Area (ha)= 0.25	Dir. Conn.(%)= 82.00
ID= 1 DT= 5.0 min	Total Imp(%)= 82.00	

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.20	0.05
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	40.82	40.00
Manning's n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07
0.167	2.54	3.167	4.07
0.250	2.54	3.250	4.07
0.333	2.54	3.333	4.07
0.417	2.54	3.417	4.07
0.500	2.54	3.500	4.07
0.583	2.54	3.583	4.07
0.667	2.54	3.667	4.07
0.750	2.54	3.750	4.07
0.833	2.54	3.833	4.07
0.917	2.54	3.917	4.07
1.000	2.54	4.000	4.07
1.083	2.54	4.083	6.10
1.167	2.54	4.167	6.10
1.250	2.54	4.250	6.10
1.333	2.54	4.333	6.10
1.417	2.54	4.417	6.10
1.500	2.54	4.500	6.10
1.583	2.54	4.583	6.10
1.667	2.54	4.667	6.10
1.750	2.54	4.750	6.10
1.833	2.54	4.833	6.10
1.917	2.54	4.917	6.10
2.000	2.54	5.000	6.10
2.083	3.05	5.083	12.20
2.167	3.05	5.167	12.20
2.250	3.05	5.250	12.20
2.333	3.05	5.333	12.20
2.417	3.05	5.417	12.20
2.500	3.05	5.500	12.20
2.583	3.05	5.583	48.82
2.667	3.05	5.667	48.82
2.750	3.05	5.750	48.82
2.833	3.05	5.833	134.24
2.917	3.05	5.917	134.24
3.000	3.05	6.000	134.24

Max.Eff.Inten.(mm/hr)=	134.24	117.80
over (min)	5.00	10.00

3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03
-------	------	-------	--------	-------	------	-------	------

Max.Eff.Inten.(mm/hr)=	134.24	117.80	
over (min)	5.00	10.00	
Storage Coeff. (min)=	1.93 (ii)	9.15 (ii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.31	0.12	
PEAK FLOW (cms)=	0.22	0.07	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.291 (iii)
RUNOFF VOLUME (mm)=	100.70	76.77	92.80
TOTAL RAINFALL (mm)=	101.70	101.70	101.70
RUNOFF COEFFICIENT =	0.99	0.75	0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20112)	Area (ha)= 0.90	Dir. Conn.(%)= 80.00
ID= 1 DT= 5.0 min	Total Imp(%)= 80.00	

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.72	0.18
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	77.46	40.00
Manning's n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07
0.167	2.54	3.167	4.07
0.250	2.54	3.250	4.07
0.333	2.54	3.333	4.07
0.417	2.54	3.417	4.07
0.500	2.54	3.500	4.07
0.583	2.54	3.583	4.07
0.667	2.54	3.667	4.07
0.750	2.54	3.750	4.07
0.833	2.54	3.833	4.07
0.917	2.54	3.917	4.07
1.000	2.54	4.000	4.07
1.083	2.54	4.083	6.10
1.167	2.54	4.167	6.10
1.250	2.54	4.250	6.10
1.333	2.54	4.333	6.10
1.417	2.54	4.417	6.10
1.500	2.54	4.500	6.10
1.583	2.54	4.583	6.10
1.667	2.54	4.667	6.10
1.750	2.54	4.750	6.10
1.833	2.54	4.833	6.10
1.917	2.54	4.917	6.10
2.000	2.54	5.000	6.10
2.083	3.05	5.083	12.20
2.167	3.05	5.167	12.20
2.250	3.05	5.250	12.20
2.333	3.05	5.333	12.20
2.417	3.05	5.417	12.20
2.500	3.05	5.500	12.20
2.583	3.05	5.583	48.82
2.667	3.05	5.667	48.82
2.750	3.05	5.750	48.82
2.833	3.05	5.833	134.24
2.917	3.05	5.917	134.24
3.000	3.05	6.000	134.24

Max.Eff.Inten.(mm/hr)=	134.24	117.80
over (min)	5.00	10.00

Storage Coeff. (min)=	1.95 (ii)	5.89 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.31	0.15

TOTALS

Storage Coeff. (min)=	1.33 (ii)	5.07 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.33	0.16

PEAK FLOW (cms)=	0.08	0.01	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.090 (iii)
RUNOFF VOLUME (mm)=	100.70	76.77	96.38
TOTAL RAINFALL (mm)=	101.70	101.70	101.70
RUNOFF COEFFICIENT =	0.99	0.75	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0121)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID= 1 (20101):	0.25	0.090	6.00	96.38
+ ID2= 2 (20102):	0.47	0.165	6.00	94.47
ID = 3 (0121):	0.72	0.255	6.00	95.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (20111)	Area (ha)= 0.87	Dir. Conn.(%)= 67.00
ID= 1 DT= 5.0 min	Total Imp(%)= 67.00	

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.58	0.29
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	76.16	40.00
Manning's n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07
0.167	2.54	3.167	4.07
0.250	2.54	3.250	4.07
0.333	2.54	3.333	4.07
0.417	2.54	3.417	4.07
0.500	2.54	3.500	4.07
0.583	2.54	3.583	4.07
0.667	2.54	3.667	4.07
0.750	2.54	3.750	4.07
0.833	2.54	3.833	4.07
0.917	2.54	3.917	4.07
1.000	2.54	4.000	4.07
1.083	2.54	4.083	6.10
1.167	2.54	4.167	6.10
1.250	2.54	4.250	6.10
1.333	2.54	4.333	6.10
1.417	2.54	4.417	6.10
1.500	2.54	4.500	6.10
1.583	2.54	4.583	6.10
1.667	2.54	4.667	6.10
1.750	2.54	4.750	6.10
1.833	2.54	4.833	6.10
1.917	2.54	4.917	6.10
2.000	2.54	5.000	6.10
2.083	3.05	5.083	12.20
2.167	3.05	5.167	12.20
2.250	3.05	5.250	12.20
2.333	3.05	5.333	12.20
2.417	3.05	5.417	12.20
2.500	3.05	5.500	12.20
2.583	3.05	5.583	48.82
2.667	3.05	5.667	48.82
2.750	3.05	5.750	48.82
2.833	3.05	5.833	134.24
2.917	3.05	5.917	134.24
3.000	3.05	6.000	134.24

Max.Eff.Inten.(mm/hr)=	134.24	117.80
over (min)	5.00	10.00

PEAK FLOW (cms)=	0.27	0.05	0.320 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	100.70	76.77	96.38
TOTAL RAINFALL (mm)=	101.70	101.70	101.70
RUNOFF COEFFICIENT =	0.99	0.75	0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20122)	Area (ha)= 1.11	Dir. Conn.(%)= 81.00
ID= 1 DT= 5.0 min	Total Imp(%)= 81.00	

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.90	0.21
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	86.02	40.00
Manning's n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.54	3.083	4.07
0.167	2.54	3.167	4.07
0.250	2.54	3.250	4.07
0.333	2.54	3.333	4.07
0.417	2.54	3.417	4.07
0.500	2.54	3.500	4.07
0.583	2.54	3.583	4.07
0.667	2.54	3.667	4.07
0.750	2.54	3.750	4.07
0.833	2.54	3.833	4.07
0.917	2.54	3.917	4.07
1.000	2.54	4.000	4.07
1.083	2.54	4.083	6.10
1.167	2.54	4.167	6.10
1.250	2.54	4.250	6.10
1.333	2.54	4.333	6.10
1.417	2.54	4.417	6.10
1.500	2.54	4.500	6.10
1.583	2.54	4.583	6.10
1.667	2.54	4.667	6.10
1.750	2.54	4.750	6.10
1.833	2.54	4.833	6.10
1.917	2.54	4.917	6.10
2.000	2.54	5.000	6.10
2.083	3.05	5.083	12.20
2.167	3.05	5.167	12.20
2.250	3.05	5.250	12.20
2.333	3.05	5.333	12.20
2.417	3.05	5.417	12.20
2.500	3.05	5.500	12.20
2.583	3.05	5.583	48.82
2.667	3.05	5.667	48.82
2.750	3.05	5.750	48.82
2.833	3.05	5.833	134.24
2.917	3.05	5.917	134.24
3.000	3.05	6.000	134.24

Max.Eff.Inten.(mm/hr)=	134.24	117.80
over (min)	5.00	10.00

Storage Coeff. (min)=	2.07 (ii)	5.91 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.31	0.15

PEAK FLOW (cms)=	0.34	0.06	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	0.396 (iii)
RUNOFF VOLUME (mm)=	100.70	76.77	96.17
TOTAL RAINFALL (mm)=	101.70	101.70	101.70
RUNOFF COEFFICIENT =	0.99	0.75	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (201021) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.40 0.07
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	---	TRANSFORMED	HYETOGRAPH	---	TIME	RAIN
hrs	mm/hr		mm/hr			hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	8.14	9.58	3.56
0.667	2.54	3.667	4.07	6.667	8.14	9.67	3.56
0.750	2.54	3.750	4.07	6.750	8.14	9.75	3.56
0.833	2.54	3.833	4.07	6.833	8.14	9.83	3.56
0.917	2.54	3.917	4.07	6.917	8.14	9.92	3.56
1.000	2.54	4.000	4.07	7.000	8.14	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	6.10	10.58	2.03
1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.60 (ii) 5.03 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.15 0.02
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 100.70 76.77
 TOTAL RAINFALL (mm)= 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75

TOTALS
 (iii)
 0.103 (iii)
 97.11
 101.70
 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2005) Area (ha)= 0.28
 ID= 1 DT= 5.0 min Total Imp(%)= 90.00 Dir. Conn.(%)= 90.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.25 0.03
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 43.30 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	---	TRANSFORMED	HYETOGRAPH	---	TIME	RAIN
hrs	mm/hr		mm/hr			hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	8.14	9.58	3.56
0.667	2.54	3.667	4.07	6.667	8.14	9.67	3.56
0.750	2.54	3.750	4.07	6.750	8.14	9.75	3.56
0.833	2.54	3.833	4.07	6.833	8.14	9.83	3.56
0.917	2.54	3.917	4.07	6.917	8.14	9.92	3.56
1.000	2.54	4.000	4.07	7.000	8.14	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	6.10	10.58	2.03
1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.37 (ii) 4.22 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.24

PEAK FLOW (cms)= 0.09 0.01
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 100.70 76.77
 TOTAL RAINFALL (mm)= 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75

TOTALS
 (iii)
 0.103 (iii)
 97.11
 101.70
 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20121) Area (ha)= 1.11
 ID= 1 DT= 5.0 min Total Imp(%)= 56.00 Dir. Conn.(%)= 56.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.09 0.01
 Dep. Storage (mm)= 1.00 2.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 25.82 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

CALIB
 STANDHYD (201022) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.40 0.07
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	---	TRANSFORMED	HYETOGRAPH	---	TIME	RAIN
hrs	mm/hr		mm/hr			hrs	mm/hr
0.083	2.54	3.083	4.07	6.083	18.31	9.08	3.56
0.167	2.54	3.167	4.07	6.167	18.31	9.17	3.56
0.250	2.54	3.250	4.07	6.250	18.31	9.25	3.56
0.333	2.54	3.333	4.07	6.333	18.31	9.33	3.56
0.417	2.54	3.417	4.07	6.417	18.31	9.42	3.56
0.500	2.54	3.500	4.07	6.500	18.31	9.50	3.56
0.583	2.54	3.583	4.07	6.583	8.14	9.58	3.56
0.667	2.54	3.667	4.07	6.667	8.14	9.67	3.56
0.750	2.54	3.750	4.07	6.750	8.14	9.75	3.56
0.833	2.54	3.833	4.07	6.833	8.14	9.83	3.56
0.917	2.54	3.917	4.07	6.917	8.14	9.92	3.56
1.000	2.54	4.000	4.07	7.000	8.14	10.00	3.56
1.083	2.54	4.083	6.10	7.083	6.10	10.08	2.03
1.167	2.54	4.167	6.10	7.167	6.10	10.17	2.03
1.250	2.54	4.250	6.10	7.250	6.10	10.25	2.03
1.333	2.54	4.333	6.10	7.333	6.10	10.33	2.03
1.417	2.54	4.417	6.10	7.417	6.10	10.42	2.03
1.500	2.54	4.500	6.10	7.500	6.10	10.50	2.03
1.583	2.54	4.583	8.14	7.583	6.10	10.58	2.03
1.667	2.54	4.667	8.14	7.667	6.10	10.67	2.03
1.750	2.54	4.750	8.14	7.750	6.10	10.75	2.03
1.833	2.54	4.833	8.14	7.833	6.10	10.83	2.03
1.917	2.54	4.917	8.14	7.917	6.10	10.92	2.03
2.000	2.54	5.000	8.14	8.000	6.10	11.00	2.03
2.083	3.05	5.083	12.20	8.083	3.56	11.08	2.03
2.167	3.05	5.167	12.20	8.167	3.56	11.17	2.03
2.250	3.05	5.250	12.20	8.250	3.56	11.25	2.03
2.333	3.05	5.333	12.20	8.333	3.56	11.33	2.03
2.417	3.05	5.417	12.20	8.417	3.56	11.42	2.03
2.500	3.05	5.500	12.20	8.500	3.56	11.50	2.03
2.583	3.05	5.583	48.82	8.583	3.56	11.58	2.03
2.667	3.05	5.667	48.82	8.667	3.56	11.67	2.03
2.750	3.05	5.750	48.82	8.750	3.56	11.75	2.03
2.833	3.05	5.833	134.24	8.833	3.56	11.83	2.03
2.917	3.05	5.917	134.24	8.917	3.56	11.92	2.03
3.000	3.05	6.000	134.24	9.000	3.56	12.00	2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.60 (ii) 5.03 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.15 0.02
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 100.70 76.77
 TOTAL RAINFALL (mm)= 101.70 101.70
 RUNOFF COEFFICIENT = 0.99 0.75

TOTALS
 (iii)
 0.103 (iii)
 97.11
 101.70
 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136)
 1 + 2 = 3
 AREA (ha) PPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID= 1 (201021): 0.47 0.170 6.00 97.11
 + ID= 2 (201022): 0.47 0.170 6.00

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
0.083 2.54 3.083 4.07 6.083 18.31 9.08 3.56
0.167 2.54 3.167 4.07 6.167 18.31 9.17 3.56
0.250 2.54 3.250 4.07 6.250 18.31 9.25 3.56
0.333 2.54 3.333 4.07 6.333 18.31 9.33 3.56
0.417 2.54 3.417 4.07 6.417 18.31 9.42 3.56
0.500 2.54 3.500 4.07 6.500 18.31 9.50 3.56
0.583 2.54 3.583 4.07 6.583 18.31 9.58 3.56
0.667 2.54 3.667 4.07 6.667 18.31 9.67 3.56
0.750 2.54 3.750 4.07 6.750 18.31 9.75 3.56
0.833 2.54 3.833 4.07 6.833 18.31 9.83 3.56
0.917 2.54 3.917 4.07 6.917 18.31 9.92 3.56
1.000 2.54 4.000 4.07 7.000 18.31 10.00 3.56
1.083 2.54 4.083 6.10 7.083 6.10 10.08 2.03
1.167 2.54 4.167 6.10 7.167 6.10 10.17 2.03
1.250 2.54 4.250 6.10 7.250 6.10 10.25 2.03
1.333 2.54 4.333 6.10 7.333 6.10 10.33 2.03
1.417 2.54 4.417 6.10 7.417 6.10 10.42 2.03
1.500 2.54 4.500 6.10 7.500 6.10 10.50 2.03
1.583 2.54 4.583 8.14 7.583 6.10 10.58 2.03
1.667 2.54 4.667 8.14 7.667 6.10 10.67 2.03
1.750 2.54 4.750 8.14 7.750 6.10 10.75 2.03
1.833 2.54 4.833 8.14 7.833 6.10 10.83 2.03
1.917 2.54 4.917 8.14 7.917 6.10 10.92 2.03
2.000 2.54 5.000 8.14 8.000 6.10 11.00 2.03
2.083 3.05 5.083 12.20 8.083 3.56 11.08 2.03
2.167 3.05 5.167 12.20 8.167 3.56 11.17 2.03
2.250 3.05 5.250 12.20 8.250 3.56 11.25 2.03
2.333 3.05 5.333 12.20 8.333 3.56 11.33 2.03
2.417 3.05 5.417 12.20 8.417 3.56 11.42 2.03
2.500 3.05 5.500 12.20 8.500 3.56 11.50 2.03
2.583 3.05 5.583 48.82 8.583 3.56 11.58 2.03
2.667 3.05 5.667 48.82 8.667 3.56 11.67 2.03
2.750 3.05 5.750 48.82 8.750 3.56 11.75 2.03
2.833 3.05 5.833 134.24 8.833 3.56 11.83 2.03
2.917 3.05 5.917 134.24 8.917 3.56 11.92 2.03
3.000 3.05 6.000 134.24 9.000 3.56 12.00 2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.01 (ii) 5.04 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.24
TOTALS*
PEAK FLOW (cms)= 0.03 0.00 0.037 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 98.18
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200121) Area (ha)= 0.11 Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.10
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 27.08 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
0.083 2.54 3.083 4.07 6.083 18.31 9.08 3.56
0.167 2.54 3.167 4.07 6.167 18.31 9.17 3.56
0.250 2.54 3.250 4.07 6.250 18.31 9.25 3.56

hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 2.54 3.083 4.07 6.083 18.31 9.08 3.56
0.167 2.54 3.167 4.07 6.167 18.31 9.17 3.56
0.250 2.54 3.250 4.07 6.250 18.31 9.25 3.56
0.333 2.54 3.333 4.07 6.333 18.31 9.33 3.56
0.417 2.54 3.417 4.07 6.417 18.31 9.42 3.56
0.500 2.54 3.500 4.07 6.500 18.31 9.50 3.56
0.583 2.54 3.583 4.07 6.583 18.31 9.58 3.56
0.667 2.54 3.667 4.07 6.667 18.31 9.67 3.56
0.750 2.54 3.750 4.07 6.750 18.31 9.75 3.56
0.833 2.54 3.833 4.07 6.833 18.31 9.83 3.56
0.917 2.54 3.917 4.07 6.917 18.31 9.92 3.56
1.000 2.54 4.000 4.07 7.000 18.31 10.00 3.56
1.083 2.54 4.083 6.10 7.083 6.10 10.08 2.03
1.167 2.54 4.167 6.10 7.167 6.10 10.17 2.03
1.250 2.54 4.250 6.10 7.250 6.10 10.25 2.03
1.333 2.54 4.333 6.10 7.333 6.10 10.33 2.03
1.417 2.54 4.417 6.10 7.417 6.10 10.42 2.03
1.500 2.54 4.500 6.10 7.500 6.10 10.50 2.03
1.583 2.54 4.583 8.14 7.583 6.10 10.58 2.03
1.667 2.54 4.667 8.14 7.667 6.10 10.67 2.03
1.750 2.54 4.750 8.14 7.750 6.10 10.75 2.03
1.833 2.54 4.833 8.14 7.833 6.10 10.83 2.03
1.917 2.54 4.917 8.14 7.917 6.10 10.92 2.03
2.000 2.54 5.000 8.14 8.000 6.10 11.00 2.03
2.083 3.05 5.083 12.20 8.083 3.56 11.08 2.03
2.167 3.05 5.167 12.20 8.167 3.56 11.17 2.03
2.250 3.05 5.250 12.20 8.250 3.56 11.25 2.03
2.333 3.05 5.333 12.20 8.333 3.56 11.33 2.03
2.417 3.05 5.417 12.20 8.417 3.56 11.42 2.03
2.500 3.05 5.500 12.20 8.500 3.56 11.50 2.03
2.583 3.05 5.583 48.82 8.583 3.56 11.58 2.03
2.667 3.05 5.667 48.82 8.667 3.56 11.67 2.03
2.750 3.05 5.750 48.82 8.750 3.56 11.75 2.03
2.833 3.05 5.833 134.24 8.833 3.56 11.83 2.03
2.917 3.05 5.917 134.24 8.917 3.56 11.92 2.03
3.000 3.05 6.000 134.24 9.000 3.56 12.00 2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.00 (ii) 5.04 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.16
TOTALS*
PEAK FLOW (cms)= 0.03 0.01 0.035 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 95.63
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200122) Area (ha)= 0.13 Total Imp(%)= 86.00 Dir. Conn.(%)= 86.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.11
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 29.44 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
0.083 2.54 3.083 4.07 6.083 18.31 9.08 3.56
0.167 2.54 3.167 4.07 6.167 18.31 9.17 3.56
0.250 2.54 3.250 4.07 6.250 18.31 9.25 3.56
0.333 2.54 3.333 4.07 6.333 18.31 9.33 3.56
0.417 2.54 3.417 4.07 6.417 18.31 9.42 3.56
0.500 2.54 3.500 4.07 6.500 18.31 9.50 3.56
0.583 2.54 3.583 4.07 6.583 8.14 9.58 3.56

0.333 2.54 3.333 4.07 6.333 18.31 9.33 3.56
0.417 2.54 3.417 4.07 6.417 18.31 9.42 3.56
0.500 2.54 3.500 4.07 6.500 18.31 9.50 3.56
0.583 2.54 3.583 4.07 6.583 18.31 9.58 3.56
0.667 2.54 3.667 4.07 6.667 18.31 9.67 3.56
0.750 2.54 3.750 4.07 6.750 18.31 9.75 3.56
0.833 2.54 3.833 4.07 6.833 18.31 9.83 3.56
0.917 2.54 3.917 4.07 6.917 18.31 9.92 3.56
1.000 2.54 4.000 4.07 7.000 18.31 10.00 3.56
1.083 2.54 4.083 6.10 7.083 6.10 10.08 2.03
1.167 2.54 4.167 6.10 7.167 6.10 10.17 2.03
1.250 2.54 4.250 6.10 7.250 6.10 10.25 2.03
1.333 2.54 4.333 6.10 7.333 6.10 10.33 2.03
1.417 2.54 4.417 6.10 7.417 6.10 10.42 2.03
1.500 2.54 4.500 6.10 7.500 6.10 10.50 2.03
1.583 2.54 4.583 8.14 7.583 6.10 10.58 2.03
1.667 2.54 4.667 8.14 7.667 6.10 10.67 2.03
1.750 2.54 4.750 8.14 7.750 6.10 10.75 2.03
1.833 2.54 4.833 8.14 7.833 6.10 10.83 2.03
1.917 2.54 4.917 8.14 7.917 6.10 10.92 2.03
2.000 2.54 5.000 8.14 8.000 6.10 11.00 2.03
2.083 3.05 5.083 12.20 8.083 3.56 11.08 2.03
2.167 3.05 5.167 12.20 8.167 3.56 11.17 2.03
2.250 3.05 5.250 12.20 8.250 3.56 11.25 2.03
2.333 3.05 5.333 12.20 8.333 3.56 11.33 2.03
2.417 3.05 5.417 12.20 8.417 3.56 11.42 2.03
2.500 3.05 5.500 12.20 8.500 3.56 11.50 2.03
2.583 3.05 5.583 48.82 8.583 3.56 11.58 2.03
2.667 3.05 5.667 48.82 8.667 3.56 11.67 2.03
2.750 3.05 5.750 48.82 8.750 3.56 11.75 2.03
2.833 3.05 5.833 134.24 8.833 3.56 11.83 2.03
2.917 3.05 5.917 134.24 8.917 3.56 11.92 2.03
3.000 3.05 6.000 134.24 9.000 3.56 12.00 2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.04 (ii) 4.24 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.24
TOTALS*
PEAK FLOW (cms)= 0.04 0.00 0.048 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 97.58
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142) AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (200111): 0.10 0.037 6.00 98.18
+ ID2= 2 (200121): 0.11 0.040 6.00 97.58

ID = 3 (0142): 0.21 0.077 6.00 97.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200112) Area (ha)= 0.10 Total Imp(%)= 79.00 Dir. Conn.(%)= 79.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.08
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 25.69 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
0.083 2.54 3.083 4.07 6.083 18.31 9.08 3.56
0.167 2.54 3.167 4.07 6.167 18.31 9.17 3.56
0.250 2.54 3.250 4.07 6.250 18.31 9.25 3.56

0.667 2.54 3.667 4.07 6.667 18.31 9.67 3.56
0.750 2.54 3.750 4.07 6.750 18.31 9.75 3.56
0.833 2.54 3.833 4.07 6.833 18.31 9.83 3.56
0.917 2.54 3.917 4.07 6.917 18.31 9.92 3.56
1.000 2.54 4.000 4.07 7.000 18.31 10.00 3.56
1.083 2.54 4.083 6.10 7.083 6.10 10.08 2.03
1.167 2.54 4.167 6.10 7.167 6.10 10.17 2.03
1.250 2.54 4.250 6.10 7.250 6.10 10.25 2.03
1.333 2.54 4.333 6.10 7.333 6.10 10.33 2.03
1.417 2.54 4.417 6.10 7.417 6.10 10.42 2.03
1.500 2.54 4.500 6.10 7.500 6.10 10.50 2.03
1.583 2.54 4.583 8.14 7.583 6.10 10.58 2.03
1.667 2.54 4.667 8.14 7.667 6.10 10.67 2.03
1.750 2.54 4.750 8.14 7.750 6.10 10.75 2.03
1.833 2.54 4.833 8.14 7.833 6.10 10.83 2.03
1.917 2.54 4.917 8.14 7.917 6.10 10.92 2.03
2.000 2.54 5.000 8.14 8.000 6.10 11.00 2.03
2.083 3.05 5.083 12.20 8.083 3.56 11.08 2.03
2.167 3.05 5.167 12.20 8.167 3.56 11.17 2.03
2.250 3.05 5.250 12.20 8.250 3.56 11.25 2.03
2.333 3.05 5.333 12.20 8.333 3.56 11.33 2.03
2.417 3.05 5.417 12.20 8.417 3.56 11.42 2.03
2.500 3.05 5.500 12.20 8.500 3.56 11.50 2.03
2.583 3.05 5.583 48.82 8.583 3.56 11.58 2.03
2.667 3.05 5.667 48.82 8.667 3.56 11.67 2.03
2.750 3.05 5.750 48.82 8.750 3.56 11.75 2.03
2.833 3.05 5.833 134.24 8.833 3.56 11.83 2.03
2.917 3.05 5.917 134.24 8.917 3.56 11.92 2.03
3.000 3.05 6.000 134.24 9.000 3.56 12.00 2.03

Max.Eff.Inten.(mm/hr)= 134.24 117.80
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.09 (ii) 4.41 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.23
TOTALS*
PEAK FLOW (cms)= 0.04 0.01 0.048 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 100.70 76.77 97.34
TOTAL RAINFALL (mm)= 101.70 101.70 101.70
RUNOFF COEFFICIENT = 0.99 0.75 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145) AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (200112): 0.10 0.035 6.00 95.63
+ ID2= 2 (200122): 0.13 0.048 6.00 97.34

ID = 3 (0145): 0.23 0.083 6.00 96.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O O
O O T T H H Y Y M M O O O
OOO T T H H Y Y M M OOO
Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\visual OTTHYMO 6.1\VO2\voind.dat

Output filename: C:\Users\caef070146\AppData\Local\Civica\VH5\50564c49-8c07-4086-8157-8729fb3caba3\901f06fc
 Summary filename: C:\Users\caef070146\AppData\Local\Civica\VH5\50564c49-8c07-4086-8157-8729fb3caba3\901f06fc

DATE: 05-18-2022 TIME: 12:51:03

USER:

COMMENTS:

 ** SIMULATION : Run 18

 READ STORM Filename: C:\Users\caef070146\AppData\Local\Temp\ata\LocalTemp\43a8d910-d519-48ad-be84-5b44b6daecc3\13bf36c4
 Ptotal=112.50 mm Comments: 100-Yr,12-hr 5CS, City of Barrie

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	2.81	3.25	4.50	6.25	20.25	9.25	3.94
0.50	2.81	3.50	4.50	6.50	20.25	9.50	3.94
0.75	2.81	3.75	4.50	6.75	20.25	9.75	3.94
1.00	2.81	4.00	4.50	7.00	20.25	10.00	3.94
1.25	2.81	4.25	6.75	7.25	6.75	10.25	2.25
1.50	2.81	4.50	6.75	7.50	6.75	10.50	2.25
1.75	2.81	5.00	9.00	8.00	6.75	11.00	2.25
2.00	2.81	5.00	9.00	8.00	6.75	11.00	2.25
2.25	3.37	5.25	13.50	8.25	3.94	11.25	2.25
2.50	3.37	5.50	13.50	8.50	3.94	11.50	2.25
2.75	3.37	5.75	14.00	8.75	3.94	11.75	2.25
3.00	3.37	6.00	148.50	9.00	3.94	12.00	2.25

CALIB STANDBYD (20022)	Area (ha)	Dir. Conn.(%)
ID= 1 DT= 5.0 min	0.80	85.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 0.68	0.12
Dep. Storage (mm)= 1.00	1.00
Average Slope (%)= 1.00	2.00
Length (m)= 73.03	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25

Max. Eff. Inten. (mm/hr)= over (min)	148.50	132.61	5.00
Storage Coeff. (min)=	1.74 (ii)	4.59 (ii)	5.00
Unit Hyd. Tpeak (min)=	5.00	5.00	0.32
Unit Hyd. peak (cms)=	0.32	0.23	

PEAK FLOW (cms)=	0.26	0.03	0.285 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	111.50	87.00	108.80
TOTAL RAINFALL (mm)=	112.50	112.50	112.50
RUNOFF COEFFICIENT =	0.99	0.77	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20031)	Area (ha)	Dir. Conn.(%)
ID= 1 DT= 5.0 min	0.43	90.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 0.39	0.04
Dep. Storage (mm)= 1.00	1.00
Average Slope (%)= 53.54	40.00
Length (m)= 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= over (min)	148.50	132.61	5.00
Storage Coeff. (min)=	1.50 (ii)	4.23 (ii)	5.00

Max. Eff. Inten. (mm/hr)= over (min)	148.50	132.61	5.00
Storage Coeff. (min)=	1.81 (ii)	5.10 (ii)	5.00
Unit Hyd. Tpeak (min)=	5.00	10.00	0.32
Unit Hyd. peak (cms)=	0.32	0.16	

PEAK FLOW (cms)=	0.28	0.04	0.321 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	111.50	87.00	107.82
TOTAL RAINFALL (mm)=	112.50	112.50	112.50
RUNOFF COEFFICIENT =	0.99	0.77	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20021)	Area (ha)	Dir. Conn.(%)
ID= 1 DT= 5.0 min	0.70	89.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 0.62	0.08
Dep. Storage (mm)= 1.00	1.00
Average Slope (%)= 1.00	2.00
Length (m)= 68.31	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25

</

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (2004I) Area (ha)= 0.35
Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

Surface Area (ha)= 0.31
Dep. Storage (mm)= 1.00
Average Slope (%)= 48.99
Length (m)= 40.00
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 148.50 132.61
Storage Coeff.(min)= 1.41 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (2005I) Area (ha)= 0.29
Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

Surface Area (ha)= 0.23
Dep. Storage (mm)= 1.00
Average Slope (%)= 43.97
Length (m)= 40.00
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 148.50 132.61
Storage Coeff.(min)= 1.33 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (2006I) Area (ha)= 0.73
Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

Surface Area (ha)= 0.58
Dep. Storage (mm)= 1.00
Average Slope (%)= 69.76
Length (m)= 40.00
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

CALIB STANHYD (2004Z) Area (ha)= 0.36
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.31
Dep. Storage (mm)= 1.00
Average Slope (%)= 48.99
Length (m)= 40.00
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 148.50 132.61
Storage Coeff.(min)= 1.42 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (2005Z) Area (ha)= 0.29
Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

IMPERVIOUS PERVIOUS (i)

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 148.50 132.61
Storage Coeff.(min)= 1.76 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (2006Z) Area (ha)= 0.65
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.55
Dep. Storage (mm)= 1.00
Average Slope (%)= 65.83
Length (m)= 40.00
Mannings n = 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr

0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.666	2.81	3.666	4.50	6.666	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max.Eff.Inten.(mm/hr)= 148.50 132.61
over (min) 5.00 10.00
Storage Coeff. (min)= 1.70 (ii) 4.99 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.23 0.03 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.263 (iii)
RUNOFF VOLUME (mm)= 111.50 87.00 107.82
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2007) Area (ha)= 0.64
ID= 1 DT= 5.0 min Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.54 0.10
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 65.32 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94

1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max.Eff.Inten.(mm/hr)= 148.50 132.61
over (min) 5.00 10.00
Storage Coeff. (min)= 1.64 (ii) 4.93 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.20 0.03 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.224 (iii)
RUNOFF VOLUME (mm)= 111.50 87.00 107.82
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2009) Area (ha)= 0.63
ID= 1 DT= 5.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.51 0.12
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 64.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25

1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max.Eff.Inten.(mm/hr)= 148.50 132.61
over (min) 5.00 10.00
Storage Coeff. (min)= 1.69 (ii) 5.08 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.22 0.03 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.256 (iii)
RUNOFF VOLUME (mm)= 111.50 87.00 107.82
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2007) Area (ha)= 0.58
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.49 0.09
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.18 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50				

Max. Eff. Inten. (mm/hr) = 148.50 132.61
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 1.47 (ii) 1.53 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.33 0.16

PEAK FLOW (cms) = 0.13 0.03
 TIME TO PEAK (hrs) = 6.00 6.00
 RUNOFF VOLUME (mm) = 111.50 87.00
 TOTAL RAINFALL (mm) = 112.50 112.50
 RUNOFF COEFFICIENT = 0.99 0.77

TOTALS
 0.137 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200812) ID= 1 DT= 5.0 min	Area (ha) = 0.38 Total Imp (%) = 70.00	Dir. Conn. (%) = 70.00
---	---	------------------------

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	0.27	0.11	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	50.33	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGROPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	10.00	9.94	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	6.75	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr) = 148.50 132.61
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 1.44 (ii) 1.64 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.33 0.15

PEAK FLOW (cms) = 0.11 0.04

TOTALS
 0.147 (iii)

CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110) L = 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (200811):	0.07	0.028	6.00	107.57
+ ID2 = 2 (200812):	0.38	0.147	6.00	104.14
ID = 3 (0110):	0.45	0.175	6.00	104.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200822) ID= 1 DT= 5.0 min	Area (ha) = 0.39 Total Imp (%) = 59.00	Dir. Conn. (%) = 59.00
---	---	------------------------

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	0.23	0.16	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	50.99	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGROPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	10.00	9.94	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr) = 148.50 132.61
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 1.46 (ii) 8.35 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.33 0.12

PEAK FLOW (cms) = 0.09 0.05

TOTALS
 0.142 (iii)

CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TIME TO PEAK (hrs) = 6.00 6.00 6.00
 RUNOFF VOLUME (mm) = 111.50 87.00 104.14
 TOTAL RAINFALL (mm) = 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.99 0.77 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200811) ID= 1 DT= 5.0 min	Area (ha) = 0.07 Total Imp (%) = 84.00	Dir. Conn. (%) = 84.00
---	---	------------------------

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	0.06	0.01	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	21.60	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETOGROPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	10.00	9.94	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.91			

ADD HYD (0111)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (200812):	0.07	0.028	6.00	107.32
+ ID2= 2 (200822):	0.39	0.142	6.00	101.44
ID = 3 (0111):	0.46	0.171	6.00	102.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Total Imp (%)	Dir. Conn. (%)
STANDHYD (20102)	0.47		
ID= 1 DT= 5.0 min	74.00	74.00	74.00

Surface Area (ha)	IMPERVIOUS (%)	PERVIOUS (i)
0.35	0.12	
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	55.98	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	2.81	3.083	4.50
0.167	2.81	3.167	4.50
0.250	2.81	3.250	4.50
0.333	2.81	3.333	4.50
0.417	2.81	3.417	4.50
0.500	2.81	3.500	4.50
0.583	2.81	3.583	4.50
0.667	2.81	3.667	4.50
0.750	2.81	3.750	4.50
0.833	2.81	3.833	4.50
0.917	2.81	3.917	4.50
1.000	2.81	4.000	4.50
1.083	2.81	4.083	6.75
1.167	2.81	4.167	6.75
1.250	2.81	4.250	6.75
1.333	2.81	4.333	6.75
1.417	2.81	4.417	6.75
1.500	2.81	4.500	6.75
1.583	2.81	4.583	6.75
1.667	2.81	4.667	6.75
1.750	2.81	4.750	6.75
1.833	2.81	4.833	6.75
1.917	2.81	4.917	6.75
2.000	2.81	5.000	6.75
2.083	3.37	5.083	13.50
2.167	3.37	5.167	13.50
2.250	3.37	5.250	13.50
2.333	3.37	5.333	13.50
2.417	3.37	5.417	13.50
2.500	3.37	5.500	13.50
2.583	3.37	5.583	13.50
2.667	3.37	5.667	13.50
2.750	3.37	5.750	13.50
2.833	3.37	5.833	148.50
2.917	3.37	5.917	148.50
3.000	3.37	6.000	148.50

Max. Eff. Inten. (mm/hr)=	148.50	132.61
over (min)	5.00	10.00
Storage Coeff. (min)=	1.54 (ii)	5.87 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.33	0.15
PEAK FLOW (cms)=	0.14	0.04
TIME TO PEAK (hrs)	6.00	6.00
RUNOFF VOLUME (mm)	111.50	87.00
TOTAL RAINFALL (mm)	112.50	112.50
RUNOFF COEFFICIENT	0.99	0.77

TOTALS
0.183 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

ADD HYD (0121)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (20101):	0.25	0.101	6.00	107.08
+ ID2= 2 (20102):	0.47	0.183	6.00	105.13
ID = 3 (0121):	0.72	0.284	6.00	105.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Total Imp (%)	Dir. Conn. (%)
STANDHYD (20111)	0.87		
ID= 1 DT= 5.0 min	67.00	67.00	67.00

Surface Area (ha)	IMPERVIOUS (%)	PERVIOUS (i)
0.58	0.21	
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	76.16	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	2.81	3.083	4.50
0.167	2.81	3.167	4.50
0.250	2.81	3.250	4.50
0.333	2.81	3.333	4.50
0.417	2.81	3.417	4.50
0.500	2.81	3.500	4.50
0.583	2.81	3.583	4.50
0.667	2.81	3.667	4.50
0.750	2.81	3.750	4.50
0.833	2.81	3.833	4.50
0.917	2.81	3.917	4.50
1.000	2.81	4.000	4.50
1.083	2.81	4.083	6.75
1.167	2.81	4.167	6.75
1.250	2.81	4.250	6.75
1.333	2.81	4.333	6.75
1.417	2.81	4.417	6.75
1.500	2.81	4.500	6.75
1.583	2.81	4.583	6.75
1.667	2.81	4.667	6.75
1.750	2.81	4.750	6.75
1.833	2.81	4.833	6.75
1.917	2.81	4.917	6.75
2.000	2.81	5.000	6.75
2.083	3.37	5.083	13.50
2.167	3.37	5.167	13.50
2.250	3.37	5.250	13.50
2.333	3.37	5.333	13.50
2.417	3.37	5.417	13.50
2.500	3.37	5.500	13.50
2.583	3.37	5.583	54.00
2.667	3.37	5.667	54.00
2.750	3.37	5.750	54.00
2.833	3.37	5.833	148.50
2.917	3.37	5.917	148.50
3.000	3.37	6.000	148.50

Max. Eff. Inten. (mm/hr)=	148.50	132.61
over (min)	5.00	10.00
Storage Coeff. (min)=	1.85 (ii)	6.83 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.32	0.14
PEAK FLOW (cms)=	0.24	0.09
TIME TO PEAK (hrs)	6.00	6.00
RUNOFF VOLUME (mm)	111.50	87.00
TOTAL RAINFALL (mm)	112.50	112.50
RUNOFF COEFFICIENT	0.99	0.77

TOTALS
0.331 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Total Imp (%)	Dir. Conn. (%)
STANDHYD (20101)	0.25		
ID= 1 DT= 5.0 min	82.00	82.00	82.00

Surface Area (ha)	IMPERVIOUS (%)	PERVIOUS (i)
0.20	0.05	
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	40.82	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	2.81	3.083	4.50
0.167	2.81	3.167	4.50
0.250	2.81	3.250	4.50
0.333	2.81	3.333	4.50
0.417	2.81	3.417	4.50
0.500	2.81	3.500	4.50
0.583	2.81	3.583	4.50
0.667	2.81	3.667	4.50
0.750	2.81	3.750	4.50
0.833	2.81	3.833	4.50
0.917	2.81	3.917	4.50
1.000	2.81	4.000	4.50
1.083	2.81	4.083	6.75
1.167	2.81	4.167	6.75
1.250	2.81	4.250	6.75
1.333	2.81	4.333	6.75
1.417	2.81	4.417	6.75
1.500	2.81	4.500	6.75
1.583	2.81	4.583	6.75
1.667	2.81	4.667	6.75
1.750	2.81	4.750	6.75
1.833	2.81	4.833	6.75
1.917	2.81	4.917	6.75
2.000	2.81	5.000	6.75
2.083	3.37	5.083	13.50
2.167	3.37	5.167	13.50
2.250	3.37	5.250	13.50
2.333	3.37	5.333	13.50
2.417	3.37	5.417	13.50
2.500	3.37	5.500	13.50
2.583	3.37	5.583	54.00
2.667	3.37	5.667	54.00
2.750	3.37	5.750	54.00
2.833	3.37	5.833	148.50
2.917	3.37	5.917	148.50
3.000	3.37	6.000	148.50

Max. Eff. Inten. (mm/hr)=	148.50	132.61
over (min)	5.00	10.00
Storage Coeff. (min)=	1.27 (ii)	4.87 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.33	0.22
PEAK FLOW (cms)=	0.08	0.02
TIME TO PEAK (hrs)	6.00	6.00
RUNOFF VOLUME (mm)	111.50	87.00
TOTAL RAINFALL (mm)	112.50	112.50
RUNOFF COEFFICIENT	0.99	0.77

TOTALS
0.101 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Total Imp (%)	Dir. Conn. (%)
STANDHYD (20112)	0.90		
ID= 1 DT= 5.0 min	80.00	80.00	80.00

Surface Area (ha)	IMPERVIOUS (%)	PERVIOUS (i)
0.72	0.18	
Dep. Storage (mm)	1.00	1.00
Average Slope (%)	1.00	2.00
Length (m)	77.46	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	2.81	3.083	4.50
0.167	2.81	3.167	4.50
0.250	2.81	3.250	4.50
0.333	2.81	3.333	4.50
0.417	2.81	3.417	4.50
0.500	2.81	3.500	4.50
0.583	2.81	3.583	4.50
0.667	2.81	3.667	4.50
0.750	2.81	3.750	4.50
0.833	2.81	3.833	4.50
0.917	2.81	3.917	4.50
1.000	2.81	4.000	4.50
1.083	2.81	4.083	6.75
1.167	2.81	4.167	6.75
1.250	2.81	4.250	6.75
1.333	2.81	4.333	6.75
1.417	2.81	4.417	6.75
1.500	2.81	4.500	6.75
1.583	2.81	4.583	6.75
1.667	2.81	4.667	6.75
1.750	2.81	4.750	6.75
1.833	2.81	4.833	6.75
1.917	2.81	4.917	6.75
2.000	2.81	5.000	6.75
2.083	3.37	5.083	13.50
2.167	3.37	5.167	13.50
2.250	3.37	5.250	13.50
2.333	3.37	5.333	13.50
2.417	3.37	5.417	13.50
2.500	3.37	5.500	13.50
2.583	3.37	5.583	54.00
2.667	3.37	5.667	54.00
2.750	3.37	5.750	54.00
2.833	3.37	5.833	148.50
2.917	3.37	5.917	148.50
3.000	3.37	6.000	148.50

Max. Eff. Inten. (mm/hr)=	148.50	132.61
over (min)	5.00	10.00
Storage Coeff. (min)=	1.87 (ii)	5.65 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.32	0.15
PEAK FLOW (cms)=	0.30	0.06
TIME TO PEAK (hrs)	6.00	6.00
RUNOFF VOLUME (mm)	111.50	87.00
TOTAL RAINFALL (mm)	112.50	112.50
RUNOFF COEFFICIENT	0.99	0.77

</

Surface Area (ha)= 0.90 0.21
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 86.02 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
 over (min)= 5.00 10.00
 Storage Coeff (min)= 1.54 (ii) 4.83 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.15

TOTALS
 PEAK FLOW (cms)= 0.37 0.07 0.440 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 111.50 87.00 106.84
 TOTAL RAINFALL (mm)= 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.99 0.77 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (201021)	Area (ha)= 0.47	Total Imp(%)= 85.00	Dir. Conn.(%)= 85.00
ID= 1 DT= 5.0 min			

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.40	0.07
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	55.98	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
 over (min)= 5.00 10.00
 Storage Coeff (min)= 1.54 (ii) 4.83 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.22

TOTALS
 PEAK FLOW (cms)= 0.16 0.03 0.190 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 111.50 87.00 107.82
 TOTAL RAINFALL (mm)= 112.50 112.50 112.50
 RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (201021):	0.47	0.190	6.00	107.82
+ ID2= 2 (201022):	0.47	0.190	6.00	107.82
ID = 3 (0136):	0.94	0.380	6.00	107.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (2005) ID= 1 DT= 5.0 min	Area (ha)= 0.28	Total Imp(%)= 90.00	Dir. Conn.(%)= 90.00
--	-----------------	---------------------	----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.25	0.03
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	43.20	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.5				

0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min)= 1.99 (ii) 8.88 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.31 0.12

PEAK FLOW (cms)= 0.26 0.14 0.398 (iii)
TIME TO PEAK (hrs)= 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 100.72
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200111) ID= 1 DT= 5.0 min	Area (ha)= 0.10 Total Imp(%)= 89.50	IMPERVIOUS (%)= 0.09	PERVIOUS (i) (%)= 0.01
Surface Area (ha)= 0.09			
Dep. Storage (mm)= 1.00			
Average Slope (%)= 25.82			
Length (m)= 0.013			
Mannings n = 0.013			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN	TIME RAIN	TIME RAIN	TIME RAIN
hrs mm/hr	hrs mm/hr	hrs mm/hr	hrs mm/hr
0.083 2.81 3.083 4.50	6.083 20.25 9.08 3.94	0.167 2.81 3.167 4.50	6.167 20.25 9.17 3.94
0.250 2.81 3.250 4.50	6.250 20.25 9.25 3.94	0.333 2.81 3.333 4.50	6.333 20.25 9.33 3.94
0.417 2.81 3.417 4.50	6.417 20.25 9.42 3.94	0.500 2.81 3.500 4.50	6.500 20.25 9.50 3.94
0.583 2.81 3.583 4.50	6.583 9.00 9.58 3.94	0.667 2.81 3.667 4.50	6.667 9.00 9.67 3.94
0.750 2.81 3.750 4.50	6.750 9.00 9.75 3.94	0.833 2.81 3.833 4.50	6.833 9.00 9.83 3.94
0.917 2.81 3.917 4.50	6.917 9.00 9.92 3.94	1.000 2.81 4.000 4.50	7.000 9.00 10.00 3.94
1.083 2.81 4.083 6.75	7.083 6.75 10.08 2.25	1.167 2.81 4.167 6.75	7.167 6.75 10.17 2.25
1.250 2.81 4.250 6.75	7.250 6.75 10.25 2.25	1.333 2.81 4.333 6.75	7.333 6.75 10.33 2.25

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (mm/hr)	R.V. (mm)
ID1= 1 (200111):	0.10	0.041	6.00	108.92
+ ID2= 2 (200121):	0.11	0.045	6.00	108.30
ID = 3 (0142):	0.21	0.085	6.00	108.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200112) ID= 1 DT= 5.0 min	Area (ha)= 0.10 Total Imp(%)= 79.00	IMPERVIOUS (%)= 0.08	PERVIOUS (i) (%)= 0.02
Surface Area (ha)= 0.08			
Dep. Storage (mm)= 1.00			
Average Slope (%)= 1.00			
Length (m)= 25.69			
Mannings n = 0.013			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN	TIME RAIN	TIME RAIN	TIME RAIN
hrs mm/hr	hrs mm/hr	hrs mm/hr	hrs mm/hr
0.083 2.81 3.083 4.50	6.083 20.25 9.08 3.94	0.167 2.81 3.167 4.50	6.167 20.25 9.17 3.94
0.250 2.81 3.250 4.50	6.250 20.25 9.25 3.94	0.333 2.81 3.333 4.50	6.333 20.25 9.33 3.94
0.417 2.81 3.417 4.50	6.417 20.25 9.42 3.94	0.500 2.81 3.500 4.50	6.500 20.25 9.50 3.94
0.583 2.81 3.583 4.50	6.583 9.00 9.58 3.94	0.667 2.81 3.667 4.50	6.667 9.00 9.67 3.94
0.750 2.81 3.750 4.50	6.750 9.00 9.75 3.94	0.833 2.81 3.833 4.50	6.833 9.00 9.83 3.94
0.917 2.81 3.917 4.50	6.917 9.00 9.92 3.94	1.000 2.81 4.000 4.50	7.000 9.00 10.00 3.94
1.083 2.81 4.083 6.75	7.083 6.75 10.08 2.25	1.167 2.81 4.167 6.75	7.167 6.75 10.17 2.25
1.250 2.81 4.250 6.75	7.250 6.75 10.25 2.25	1.333 2.81 4.333 6.75	7.333 6.75 10.33 2.25
1.417 2.81 4.417 6.75	7.417 6.75 10.42 2.25	1.500 2.81 4.500 6.75	7.500 6.75 10.50 2.25
1.583 2.81 4.583 9.00	7.583 6.75 10.58 2.25	1.667 2.81 4.667 9.00	7.667 6.75 10.67 2.25

1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min)= 0.97 (ii) 3.76 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.25

PEAK FLOW (cms)= 0.04 0.00 0.04 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 100.72
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200121) ID= 1 DT= 5.0 min	Area (ha)= 0.11 Total Imp(%)= 87.00	Dir. Conn.(%)= 87.00
Surface Area (ha)= 0.10	IMPERVIOUS (i) (%)= 0.01	PERVIOUS (%)= 0.01
Dep. Storage (mm)= 1.00		
Average Slope (%)= 1.00		
Length (m)= 27.08		
Mannings n = 0.013		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN	TIME RAIN	TIME RAIN	TIME RAIN
hrs mm/hr	hrs mm/hr	hrs mm/hr	hrs mm/hr
0.083 2.81 3.083 4.50	6.083 20.25 9.08 3.94	0.167 2.81 3.167 4.50	6.167 20.25 9.17 3.94
0.250 2.81 3.250 4.50	6.250 20.25 9.25 3.94	0.333 2.81 3.333 4.50	6.333 20.25 9.33 3.94
0.417 2.81 3.417 4.50	6.417 20.25 9.42 3.94	0.500 2.81 3.500 4.50	6.500 20.25 9.50 3.94
0.583 2.81 3.583 4.50	6.583 9.00 9.58 3.94	0.667 2.81 3.667 4.50	6.667 9.00 9.67 3.94
0.750 2.81 3.750 4.50	6.750 9.00 9.75 3.94	0.833 2.81 3.833 4.50	6.833 9.00 9.83 3.94
0.917 2.81 3.917 4.50	6.917 9.00 9.92 3.94	1.000 2.81 4.000 4.50	7.000 9.00 10.00 3.94
1.083 2.81 4.083 6.75	7.083 6.75 10.08 2.25	1.167 2.81 4.167 6.75	7.167 6.75 10.17 2.25
1.250 2.81 4.250 6.75	7.250 6.75 10.25 2.25	1.333 2.81 4.333 6.75	7.333 6.75 10.33 2.25
1.417 2.81 4.417 6.75	7.417 6.75 10.42 2.25	1.500 2.81 4.500 6.75	7.500 6.75 10.50 2.25
1.583 2.81 4.583 9.00	7.583 6.75 10.58 2.25	1.667 2.81 4.667 9.00	7.667 6.75 10.67 2.25
1.750 2.81 4.750 9.00	7.750 6.75 10.75 2.25	1.833 2.81 4.833 9.00	7.833 6.75 10.83 2.25
1.917 2.81 4.917 9.00	7.917 6.75 10.92 2.25	2.000 2.81 5.000 9.00	8.000 6.75 11.00 2.25

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200122) ID= 1 DT= 5.0 min	Area (ha)= 0.13 Total Imp(%)= 86.00	Dir. Conn.(%)= 86.00
Surface Area (ha)= 0.11	IMPERVIOUS (i) (%)= 0.02	PERVIOUS (%)= 0.04
Dep. Storage (mm)= 1.00		
Average Slope (%)= 1.00		
Length (m)= 29.44		
Mannings n = 0.013		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN	TIME RAIN	TIME RAIN	TIME RAIN
hrs mm/hr	hrs mm/hr	hrs mm/hr	hrs mm/hr
0.083 2.81 3.083 4.50	6.083 20.25 9.08 3.94	0.167 2.81 3.167 4.50	6.167 20.25 9.17 3.94
0.250 2.81 3.250 4.50	6.250 20.25 9.25 3.94	0.333 2.81 3.333 4.50	6.333 20.25 9.33 3.94
0.417 2.81 3.417 4.50	6.417 20.25 9.42 3.94	0.500 2.81 3.500 4.50	6.500 20.25 9.50 3.94
0.583 2.81 3.583 4.50	6.583 9.00 9.58 3.94	0.667 2.81 3.667 4.50	6.667 9.00 9.67 3.94
0.750 2.81 3.750 4.50	6.750 9.00 9.75 3.94	0.833 2.81 3.833 4.50	6.833 9.00 9.83 3.94
0.917 2.81 3.917 4.50	6.917 9.00 9.92 3.94	1.000 2.81 4.000 4.50	7.000 9.00 10.00 3.94
1.083 2.81 4.083 6.75	7.083 6.75 10.08 2.25	1.167 2.81 4.167 6.75	7.167 6.75 10.17 2.25
1.250 2.81 4.250 6.75	7.250 6.75 10.25 2.25	1.333 2.81 4.333 6.75	7.333 6.75 10.33 2.25
1.417 2.81 4.417 6.75	7.417 6.75 10.42 2.25	1.500 2.81 4.500 6.75	7.500 6.75 10.50 2.25
1.583 2.81 4.583 9.00	7.583 6.75 10.58 2.25	1.667 2.81 4.667 9.00	7.667 6.75 10.67 2.25
1.750 2.81 4.750 9.00	7.750 6.75 10.75 2.25	1.833 2.81 4.833 9.00	7.833 6.75 10.83 2.25
1.917 2.81 4.917 9.00	7.917 6.75 10.92 2.25	2.000 2.81 5.000 9.00	8.000 6.75 11.00 2.25
2.083 3.37 5.083 13.50	8.083 3.94 11.08 2.25	2.167 3.37 5.167 13.50	8.167 3.94 11.17 2.25
2.250 3.37 5.250 13.50	8.250 3.94 11.25 2.25	2.333 3.37 5.333 13.50	8.333 3.94 11.33 2.25

2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min)= 1.05 (ii) 4.23 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.24

PEAK FLOW (cms)= 0.05 0.01 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.053 (iii)
RUNOFF VOLUME (mm)= 111.50 87.00 108.06
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

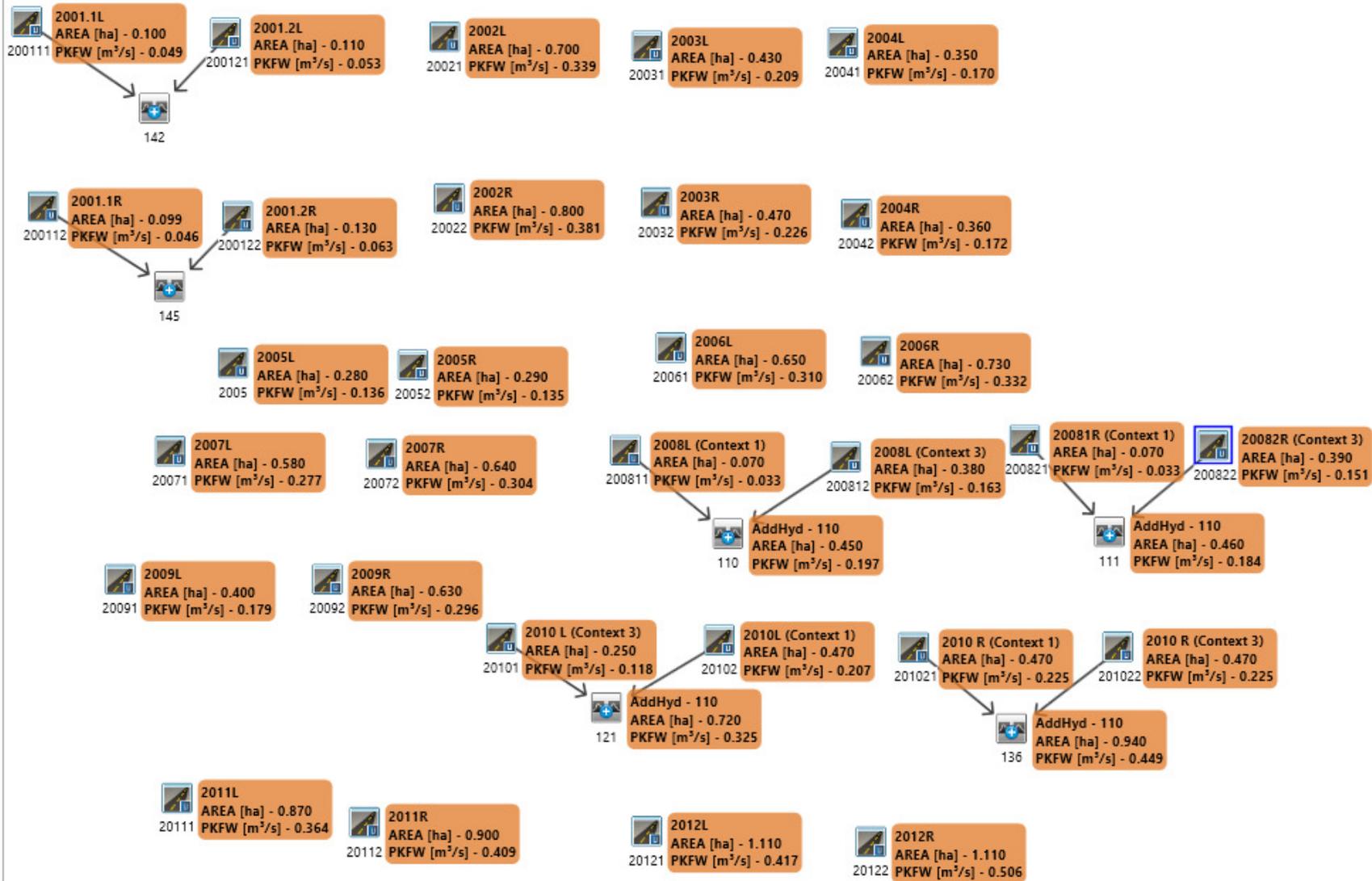
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (200112):	0.10	0.040	6.00	106.33
+ ID2= 2 (200122):	0.13	0.053	6.00	108.06
ID = 3 (0145):	0.23	0.092	6.00	107.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

100-yr Post-Developmet Roadway Drainage Area Flows (4-hr Chicao Storm)



V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A A L
V V I SS U U A A A L
V V I SSSSS UUUU A A LLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O O
O O T T H H Y Y M M O O O
000 T T H H Y Y M M 000
Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****
Input filename: C:\Program Files (x86)\visualOTHMWO 6.1\VO2\voindat
Output filename: C:\Users\caef070146\AppData\Local\Civica\H5\30564c49-8c07-4086-8157-8729fb3caba3\82eebc13
Summary filename: C:\Users\caef070146\AppData\Local\Civica\H5\30564c49-8c07-4086-8157-8729fb3caba3\82eebc13

DATE: 05-18-2022 TIME: 12:44:41
USER:
COMMENTS:

** SIMULATION : Run 01

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for 2-year, 4-hr Chicago, City of Barrile.

CALIB STANDHYD (20022) Area (ha)= 0.80 Dir. Conn.(%)= 85.00
Surface Area (ha)= 0.68 IMPERVIOUS 0.12 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 73.03 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for 2-year, 4-hr Chicago, City of Barrile.

Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 53.34 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for 2-year, 4-hr Chicago, City of Barrile.

Max. Eff. Inten. (mm/hr)= 83.11 41.24
Storage Coeff. (min)= 1.89 (ii) 5.34 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16
PEAK FLOW (cms)= 0.09 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33 (iii)
RUNOFF VOLUME (mm)= 35.98 19.22 34.30
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20032) Area (ha)= 0.47 Dir. Conn.(%)= 87.00
Surface Area (ha)= 0.41 IMPERVIOUS 0.06 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for 2-year, 4-hr Chicago, City of Barrile.

Max. Eff. Inten. (mm/hr)= 83.11 41.24
Storage Coeff. (min)= 1.94 (ii) 5.83 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.15
PEAK FLOW (cms)= 0.09 0.01 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33

0.833 5.30 | 1.833 9.01 | 2.833 3.46 | 3.83 2.26
0.917 7.98 | 1.917 6.97 | 2.917 3.17 | 3.92 2.15
1.000 7.98 | 2.000 6.97 | 3.000 3.17 | 4.00 2.15
Max. Eff. Inten. (mm/hr)= 83.11 41.24
Storage Coeff. (min)= 2.28 (ii) 6.43 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.14
TOTALS (iii)
PEAK FLOW (cms)= 0.16 0.01 0.164 (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 35.98 19.22 34.30
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20021) Area (ha)= 0.70 Dir. Conn.(%)= 89.00
Surface Area (ha)= 0.62 IMPERVIOUS 0.08 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 68.31 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for 2-year, 4-hr Chicago, City of Barrile.

CALIB STANDHYD (20031) Area (ha)= 0.43 Dir. Conn.(%)= 90.00
Surface Area (ha)= 0.39 IMPERVIOUS 0.04 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.30 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for 2-year, 4-hr Chicago, City of Barrile.

RUNOFF VOLUME (mm)= 35.98 19.22 33.80
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.91
***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20041) Area (ha)= 0.35 Dir. Conn.(%)= 89.00
Surface Area (ha)= 0.31 IMPERVIOUS 0.04 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.30 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for 2-year, 4-hr Chicago, City of Barrile.

Max. Eff. Inten. (mm/hr)= 83.11 41.24
Storage Coeff. (min)= 1.78 (ii) 5.38 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16
PEAK FLOW (cms)= 0.07 0.00 0.075 (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 35.98 19.22 34.30
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20042) Area (ha)= 0.36 Dir. Conn.(%)= 85.00
Surface Area (ha)= 0.31 IMPERVIOUS 0.05 PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.30 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for 2-year, 4-hr Chicago, City of Barrile.

0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max. Eff. Inten. (mm/hr) = 83.11 41.24
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 1.79 (ii) 5.94 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.32 0.15

PEAK FLOW (cms) = 0.07 0.00 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.42 0.075 (iii)
 RUNOFF VOLUME (mm) = 35.98 19.22 33.46
 TOTAL RAINFALL (mm) = 36.98 36.98 36.98
 RUNOFF COEFFICIENT = 0.97 0.52 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20052)	Area (ha) = 0.29
ID= 1 DT= 5.0 min		Total Imp (%) = 78.00

Surface Area (ha) =	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm) =	0.23	0.06
Average Slope (%) =	1.00	1.00
Length (m) =	43.97	40.00
Mannings n =	0.13	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max. Eff. Inten. (mm/hr) = 83.11 32.82
 over (min) = 5.00 15.00
 Storage Coeff. (min) = 1.68 (ii) 13.73 (ii)
 Unit Hyd. Tpeak (min) = 5.00 15.00
 Unit Hyd. peak (cms) = 0.32 0.08

PEAK FLOW (cms) = 0.05 0.00 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.50 0.054 (iii)
 RUNOFF VOLUME (mm) = 35.98 19.22 32.27
 TOTAL RAINFALL (mm) = 36.98 36.98 36.98
 RUNOFF COEFFICIENT = 0.97 0.52 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Max. Eff. Inten. (mm/hr) = 83.11 41.24
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 2.14 (ii) 6.29 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.31 0.15

PEAK FLOW (cms) = 0.13 0.01 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.42 0.134 (iii)
 RUNOFF VOLUME (mm) = 35.98 19.22 33.46
 TOTAL RAINFALL (mm) = 36.98 36.98 36.98
 RUNOFF COEFFICIENT = 0.97 0.52 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20072)	Area (ha) = 0.64
ID= 1 DT= 5.0 min		Total Imp (%) = 84.00

Surface Area (ha) =	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm) =	0.54	0.10
Average Slope (%) =	1.00	1.00
Length (m) =	65.32	40.00
Mannings n =	0.13	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max. Eff. Inten. (mm/hr) = 83.11 41.24
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 2.13 (ii) 6.41 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.31 0.14

PEAK FLOW (cms) = 0.12 0.01 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.42 0.131 (iii)
 RUNOFF VOLUME (mm) = 35.98 19.22 33.30
 TOTAL RAINFALL (mm) = 36.98 36.98 36.98
 RUNOFF COEFFICIENT = 0.97 0.52 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20071)	Area (ha) = 0.58
ID= 1 DT= 5.0 min		Total Imp (%) = 85.00

Surface Area (ha) =	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm) =	0.49	0.09
Average Slope (%) =	1.00	1.00
Length (m) =	62.18	40.00
Mannings n =	0.13	0.290

CALIB	STANDHYD (20062)	Area (ha) = 0.73
ID= 1 DT= 5.0 min		Total Imp (%) = 80.00

Surface Area (ha) =	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm) =	0.58	0.15
Average Slope (%) =	1.00	1.00
Length (m) =	69.76	40.00
Mannings n =	0.13	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max. Eff. Inten. (mm/hr) = 83.11 41.24
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 2.22 (ii) 6.99 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.30 0.14

PEAK FLOW (cms) = 0.13 0.01 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.42 0.144 (iii)
 RUNOFF VOLUME (mm) = 35.98 19.22 32.63
 TOTAL RAINFALL (mm) = 36.98 36.98 36.98
 RUNOFF COEFFICIENT = 0.97 0.52 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20061)	Area (ha) = 0.65
ID= 1 DT= 5.0 min		Total Imp (%) = 85.00

Surface Area (ha) =	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm) =	0.55	0.10
Average Slope (%) =	1.00	1.00
Length (m) =	65.83	40.00
Mannings n =	0.13	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max. Eff. Inten. (mm/hr) = 83.11 41.24
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 2.07 (ii) 6.22 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.31 0.15

PEAK FLOW (cms) = 0.11 0.01 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.42 0.120 (iii)
 RUNOFF VOLUME (mm) = 35.98 19.22 33.46
 TOTAL RAINFALL (mm) = 36.98 36.98 36.98
 RUNOFF COEFFICIENT = 0.97 0.52 0.90

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max. Eff. Inten. (mm/hr) = 83.11 41.24
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 2.07 (ii) 6.22 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.31 0.15

PEAK FLOW (cms) = 0.11 0.01 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.42 0.120 (iii)
 RUNOFF VOLUME (mm) = 35.98 19.22 33.46
 TOTAL RAINFALL (mm) = 36.98 36.98 36.98
 RUNOFF COEFFICIENT = 0.97 0.52 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20092)	Area (ha) = 0.63
ID= 1 DT= 5.0 min		Total Imp (%) = 81.00

Surface Area (ha) =	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm) =	0.51	0.12
Average Slope (%) =	1.00	1.00
Length (m) =	64.81	40.00
Mannings n =	0.13	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20091) ID=1 DT= 5.0 min Area (ha)= 0.40 Total Imp(%)= 77.00 Dir. Conn.(%)= 77.00

Surface Area (ha)= 0.31 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 51.64 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

Max. Eff. Inten. (mm/hr)= 83.11 32.82
over (min)= 5.00 15.00
Storage Coeff. (min)= 1.85 (ii) 13.90 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.08

PEAK FLOW (cms)= 0.07 0.01 *TOTALS* 0.074 (iii)
TIME TO PEAK (hrs)= 1.33 1.50
RUNOFF VOLUME (mm)= 35.98 19.22 32.11
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200812) ID=1 DT= 5.0 min Area (ha)= 0.38 Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00

Surface Area (ha)= 0.27 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

1 + 2 = 3 AREA QPEAK TPEAK R.V.
ID1= 1 (200811): 0.07 0.014 1.33 33.27
+ ID2= 2 (200812): 0.38 0.065 1.33 30.94

ID = 3 (0110): 0.45 0.079 1.33 31.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200822) ID=1 DT= 5.0 min Area (ha)= 0.39 Total Imp(%)= 59.00 Dir. Conn.(%)= 59.00

Surface Area (ha)= 0.23 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

Max. Eff. Inten. (mm/hr)= 83.11 32.82
over (min)= 5.00 15.00
Storage Coeff. (min)= 1.84 (ii) 13.88 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.08

PEAK FLOW (cms)= 0.05 0.01 *TOTALS* 0.058 (iii)
TIME TO PEAK (hrs)= 1.33 1.50
RUNOFF VOLUME (mm)= 35.98 19.22 29.09
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200821) ID=1 DT= 5.0 min Area (ha)= 0.07 Total Imp(%)= 83.00 Dir. Conn.(%)= 83.00

Surface Area (ha)= 0.05 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 21.60 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

0.500 3.31 1.500 24.75 2.500 4.28 3.50 2.55
0.583 4.05 1.583 13.01 2.583 3.82 3.58 2.39
0.667 4.05 1.667 13.01 2.667 3.82 3.67 2.39
0.750 5.30 1.750 9.01 2.750 3.46 3.75 2.26
0.833 5.30 1.833 9.01 2.833 3.46 3.83 2.26
0.917 7.98 1.917 6.97 2.917 3.17 3.92 2.15
1.000 7.98 2.000 6.97 3.000 3.17 4.00 2.15

Max. Eff. Inten. (mm/hr)= 83.11 32.82
over (min)= 5.00 15.00
Storage Coeff. (min)= 1.82 (ii) 13.87 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.08

PEAK FLOW (cms)= 0.06 0.01 *TOTALS* 0.065 (iii)
TIME TO PEAK (hrs)= 1.33 1.50
RUNOFF VOLUME (mm)= 35.98 19.22 30.94
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200811) ID=1 DT= 5.0 min Area (ha)= 0.07 Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

Surface Area (ha)= 0.06 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 21.60 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

Max. Eff. Inten. (mm/hr)= 83.11 41.24
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.10 (ii) 5.32 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.34 0.16

PEAK FLOW (cms)= 0.01 0.00 *TOTALS* 0.014 (iii)
TIME TO PEAK (hrs)= 1.33 1.42
RUNOFF VOLUME (mm)= 35.98 19.22 33.27
TOTAL RAINFALL (mm)= 36.98 36.97 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (0110) |

0.500 3.31 1.500 24.75 2.500 4.28 3.50 2.55
0.583 4.05 1.583 13.01 2.583 3.82 3.58 2.39
0.667 4.05 1.667 13.01 2.667 3.82 3.67 2.39
0.750 5.30 1.750 9.01 2.750 3.46 3.75 2.26
0.833 5.30 1.833 9.01 2.833 3.46 3.83 2.26
0.917 7.98 1.917 6.97 2.917 3.17 3.92 2.15
1.000 7.98 2.000 6.97 3.000 3.17 4.00 2.15

Max. Eff. Inten. (mm/hr)= 83.11 41.24
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.10 (ii) 5.50 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.34 0.16

PEAK FLOW (cms)= 0.01 0.00 *TOTALS* 0.014 (iii)
TIME TO PEAK (hrs)= 1.33 1.42
RUNOFF VOLUME (mm)= 35.98 19.22 33.10
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

1 + 2 = 3 AREA QPEAK TPEAK R.V.
ID1= 1 (200821): 0.07 0.014 1.33 33.10
+ ID2= 2 (200822): 0.39 0.058 1.33 29.09

ID = 3 (0111): 0.46 0.072 1.33 29.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (20102) ID=1 DT= 5.0 min Area (ha)= 0.47 Total Imp(%)= 74.00 Dir. Conn.(%)= 74.00

Surface Area (ha)= 0.35 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN

Max. Eff. Inten. (mm/hr)= 83.11 32.82
over (min)= 5.00 15.00
Storage Coeff. (min)= 1.94 (ii) 13.99 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

PEAK FLOW (cms)= 0.08 0.01 *TOTALS* 0.084 (iii)
TIME TO PEAK (hrs)= 1.33 1.50
RUNOFF VOLUME (mm)= 35.98 19.22 31.61
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20101) ID= 1 DT= 5.0 min	Area (ha)= 0.25 Total Imp(%)= 82.00	Dir. Conn.(%)= 82.00
--	--	----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.20	0.05
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	40.82	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max.Eff.Inten.(mm/hr)=	83.11	41.24
over (min)=	5.00	10.00
Storage Coeff. (min)=	1.61 (ii)	6.14 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.32	0.15
PEAK FLOW (cms)=	0.05	0.00
TIME TO PEAK (hrs)=	1.33	1.42
RUNOFF VOLUME (mm)=	35.98	19.22
TOTAL RAINFALL (mm)=	36.98	36.98
RUNOFF COEFFICIENT =	0.97	0.52

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0121) 1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (20101):	0.25	0.051	1.33	32.95
+ ID2 = 2 (20102):	0.47	0.084	1.33	31.61
ID = 3 (0121):	0.72	0.134	1.33	32.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (20111) ID= 1 DT= 5.0 min	Area (ha)= 0.87 Total Imp(%)= 67.00	Dir. Conn.(%)= 67.00
--	--	----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.58	0.29
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	76.16	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max.Eff.Inten.(mm/hr)=	83.11	41.24
over (min)=	5.00	10.00
Storage Coeff. (min)=	2.34 (ii)	14.38 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.30	0.08
PEAK FLOW (cms)=	0.13	0.02
TIME TO PEAK (hrs)=	1.33	1.50
RUNOFF VOLUME (mm)=	35.98	19.22
TOTAL RAINFALL (mm)=	36.98	36.98
RUNOFF COEFFICIENT =	0.97	0.52

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20112) ID= 1 DT= 5.0 min	Area (ha)= 0.90 Total Imp(%)= 80.00	Dir. Conn.(%)= 80.00
--	--	----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.72	0.18
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	77.46	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max.Eff.Inten.(mm/hr)=	83.11	41.24
over (min)=	5.00	10.00
Storage Coeff. (min)=	2.36 (ii)	7.13 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.30	0.14
PEAK FLOW (cms)=	0.16	0.02
TIME TO PEAK (hrs)=	1.33	1.42
RUNOFF VOLUME (mm)=	35.98	19.22
TOTAL RAINFALL (mm)=	36.98	36.98
RUNOFF COEFFICIENT =	0.97	0.52

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20122) ID= 1 DT= 5.0 min	Area (ha)= 1.11 Total Imp(%)= 81.00	Dir. Conn.(%)= 81.00
--	--	----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.90	0.21
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	86.02	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000	6.97	3.000	3.17	4.00	2.15

Max.Eff.Inten.(mm/hr)=	83.11	41.24
over (min)=	5.00	10.00
Storage Coeff. (min)=	1.61 (ii)	7.16 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.29	0.14
PEAK FLOW (cms)=	0.20	0.02
TIME TO PEAK (hrs)=	1.33	1.42
RUNOFF VOLUME (mm)=	35.98	19.22
TOTAL RAINFALL (mm)=	36.98	36.98
RUNOFF COEFFICIENT =	0.97	0.52

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (201021) ID= 1 DT= 5.0 min	Area (ha)= 0.47 Total Imp(%)= 85.00	Dir. Conn.(%)= 85.00
---	--	----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	1.00	0.47
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	55.98	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (201022) ID= 1 DT= 5.0 min	Area (ha)= 0.47 Total Imp(%)= 85.00	Dir. Conn.(%)= 85.00
---	--	----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.40	0.07
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	55.98	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.47	1.083	18.78	2.083	5.73	3.08	2.93
0.167	2.47	1.167	18.78	2.167	5.73	3.17	2.93
0.250	2.82	1.250	83.11	2.250	4.89	3.25	2.72
0.333	2.82	1.333	83.11	2.333	4.89	3.33	2.72
0.417	3.31	1.417	24.75	2.417	4.28	3.42	2.55
0.500	3.31	1.500	24.75	2.500	4.28	3.50	2.55
0.583	4.05	1.583	13.01	2.583	3.82	3.58	2.39
0.667	4.05	1.667	13.01	2.667	3.82	3.67	2.39
0.750	5.30	1.750	9.01	2.750	3.46	3.75	2.26
0.833	5.30	1.833	9.01	2.833	3.46	3.83	2.26
0.917	7.98	1.917	6.97	2.917	3.17	3.92	2.15
1.000	7.98	2.000					

1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (201021): 0.47 0.097 1.33 33.46
+ ID2= 2 (201022): 0.47 0.097 1.33 33.46
ID = 3 (0136): 0.94 0.194 1.33 33.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (2005) | Area (ha)= 0.28 Total Imp (%) = 90.00 Dir. Conn. (%) = 90.00
Surface Area (ha)= 0.28
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 43.20
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 2.47 1.083 18.78 2.083 5.73 3.08 2.93

Max.Eff.Inten.(mm/hr)= 83.11 41.24
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.66 (ii) 5.11 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.06 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42
RUNOFF VOLUME (mm)= 35.98 19.22 34.30
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^ = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20121) | Area (ha)= 1.11 Total Imp (%) = 56.00 Dir. Conn. (%) = 56.00
Surface Area (ha)= 0.62
Dep. Storage (mm)= 1.00
Average Slope (%)= 86.02
Length (m)= 86.02
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 2.47 1.083 18.78 2.083 5.73 3.08 2.93

Max.Eff.Inten.(mm/hr)= 83.11 41.24
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.26 (ii) 5.14 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.02 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42
RUNOFF VOLUME (mm)= 35.98 19.22 33.78
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^ = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142) | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (200111): 0.10 0.022 1.33 34.21
+ ID2= 2 (200121): 0.11 0.023 1.33 33.78
ID = 3 (0142): 0.21 0.045 1.33 33.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (200112) | Area (ha)= 0.10 Total Imp (%) = 79.00 Dir. Conn. (%) = 79.00
Surface Area (ha)= 0.10
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 25.69
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 2.47 1.083 18.78 2.083 5.73 3.08 2.93

Max.Eff.Inten.(mm/hr)= 83.11 41.24
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.32 (ii) 5.34 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.03 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42
RUNOFF VOLUME (mm)= 35.99 19.22 33.61
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^ = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.500 3.31 1.500 24.75 2.500 4.28 3.50 2.55
0.583 4.05 1.583 13.01 2.583 3.82 3.58 2.39
0.667 4.05 1.667 13.01 2.667 3.82 3.67 2.39
0.750 5.30 1.750 9.01 2.750 3.46 3.75 2.26
0.833 5.30 1.833 9.01 2.833 3.46 3.83 2.26
0.917 7.98 1.917 6.97 2.917 3.17 3.92 2.15
1.000 7.98 2.000 6.97 3.000 3.17 4.00 2.15

Max.Eff.Inten.(mm/hr)= 83.11 32.82
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.51 (ii) 14.56 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.29 0.08

PEAK FLOW (cms)= 0.14 0.03 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.50
RUNOFF VOLUME (mm)= 35.98 19.22 28.60
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^ = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (200111) | Area (ha)= 0.10 Total Imp (%) = 89.50 Dir. Conn. (%) = 89.50
Surface Area (ha)= 0.09
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 25.82
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.583 4.05 1.583 13.01 2.583 3.82 3.58 2.39

Max.Eff.Inten.(mm/hr)= 83.11 41.24
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.22 (ii) 4.75 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.02 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.33
RUNOFF VOLUME (mm)= 35.99 19.22 34.21
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^ = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (200121) | Area (ha)= 0.11
Surface Area (ha)= 0.11
Dep. Storage (mm)= 1.00
Average Slope (%)= 29.44
Length (m)= 29.44
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 2.47 1.083 18.78 2.083 5.73 3.08 2.93

Max.Eff.Inten.(mm/hr)= 83.11 41.24
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.22 (ii) 6.11 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.02 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42
RUNOFF VOLUME (mm)= 35.98 19.22 32.42
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^ = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (200122) | Area (ha)= 0.13 Total Imp (%) = 86.00 Dir. Conn. (%) = 86.00
Surface Area (ha)= 0.11
Dep. Storage (mm)= 1.00
Average Slope (%)= 29.44
Length (m)= 29.44
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 2.47 1.083 18.78 2.083 5.73 3.08 2.93

Max.Eff.Inten.(mm/hr)= 83.11 41.24
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.32 (ii) 5.34 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.03 0.00 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42
RUNOFF VOLUME (mm)= 35.99 19.22 33.61
TOTAL RAINFALL (mm)= 36.98 36.98 36.98
RUNOFF COEFFICIENT = 0.97 0.52 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^ = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (200112):	0.10	0.020	1.33	32.42
+ ID2 = 2 (200122):	0.13	0.027	1.33	33.61
ID = 3 (0145):	0.23	0.047	1.33	33.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U A A A A L
W V I SSSSS UUUU A L LLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2020 Smart City Water Inc
 All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\VisualOTHYMO 6.1\vo2\voin.dat
 Output filename: C:\Users\caef070146\AppData\Local\Civica\WHS\50564c49-8c07-4086-8157-8729fb3caba3\5ca142d0
 Summary filename: C:\Users\caef070146\AppData\Local\Civica\WHS\50564c49-8c07-4086-8157-8729fb3caba3\5ca142d0

DATE: 05-18-2022 TIME: 12:44:41

USER:

COMMENTS:

** SIMULATION : Run 02 **

READ STORM		Filename: C:\Users\caef070146\AppData\Local\Temp\11403a02-80e0-48b3-97a8-b203216ba841\3558f47a	
Ptotal= 50.52 mm		Comments: 5-year, 4 hr-Chicago, City of Barrie	
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.17	3.57	1.17	25.64
0.33	4.07	1.33	108.92
0.50	4.76	1.50	33.31
0.67	5.79	1.67	17.99
0.83	7.53	1.83	12.60
1.00	11.20	2.00	9.82

CALIB STANDHYD (20022)		Area (ha)= 0.80	Dir. Conn.(%)= 85.00
ID= 1 DT= 5.0 min		Total Imp(%)= 85.00	
Surface Area (ha)	IMPERVIOUS (0.68)	PERVIOUS (i) (0.12)	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	73.03	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

CALIB STANDHYD (20031)		Area (ha)= 0.43	Dir. Conn.(%)= 90.00
ID= 1 DT= 5.0 min		Total Imp(%)= 90.00	
Surface Area (ha)	IMPERVIOUS (0.39)	PERVIOUS (i) (0.04)	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	53.54	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max. Eff. Inten. (mm/hr)	108.92	64.33
over (min)	5.00	10.00
Storage Coeff. (min)	1.70 (ii)	4.79 (ii)
Unit Hyd. Tpeak (min)	5.00	5.00
Unit Hyd. peak (cms)	0.32	0.22
PEAK FLOW (cms)	0.12	0.01
TIME TO PEAK (hrs)	1.33	1.33
RUNOFF VOLUME (mm)	49.52	30.31
TOTAL RAINFALL (mm)	50.52	50.52
RUNOFF COEFFICIENT	0.98	0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20032)		Area (ha)= 0.47	Dir. Conn.(%)= 87.00
ID= 1 DT= 5.0 min		Total Imp(%)= 87.00	
Surface Area (ha)	IMPERVIOUS (0.41)	PERVIOUS (i) (0.06)	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	55.98	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20021)		Area (ha)= 0.70	Dir. Conn.(%)= 89.00
ID= 1 DT= 5.0 min		Total Imp(%)= 89.00	
Surface Area (ha)	IMPERVIOUS (0.62)	PERVIOUS (i) (0.08)	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	68.31	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max. Eff. Inten. (mm/hr)	108.92	64.33
over (min)	5.00	10.00
Storage Coeff. (min)	1.96 (ii)	5.19 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.31	0.16
PEAK FLOW (cms)	0.19	0.01
TIME TO PEAK (hrs)	1.33	1.42
RUNOFF VOLUME (mm)	49.52	30.31
TOTAL RAINFALL (mm)	50.52	50.52
RUNOFF COEFFICIENT	0.98	0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20041)		Area (ha)= 0.35	Dir. Conn.(%)= 89.00
ID= 1 DT= 5.0 min		Total Imp(%)= 89.00	
Surface Area (ha)	IMPERVIOUS (0.31)	PERVIOUS (i) (0.04)	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	48.30	40.00	
Mannings n	0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Max. Eff. Inten. (mm/hr)	108.92	64.33
over (min)	5.00	10.00
Storage Coeff. (min)	1.74 (ii)	5.23 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.32	0.16
PEAK FLOW (cms)	0.12	0.01
TIME TO PEAK (hrs)	1.33	1.42
RUNOFF VOLUME (mm)	49.52	30.31
TOTAL RAINFALL (mm)	50.52	50.52
RUNOFF COEFFICIENT	0.98	0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20042)		Area (ha)= 0.36	Dir. Conn.(%)= 85.00
ID= 1 DT= 5.0 min		Total Imp(%)= 85.00	
Surface Area (ha)	IMPERVIOUS (0.31)	PERVIOUS (i) (0.05)	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	48.99	40.00	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20042)		Area (ha)= 0.36	Dir. Conn.(%)= 85.00
ID= 1 DT= 5.0 min		Total Imp(%)= 85.00	
Surface Area (ha)	IMPERVIOUS (0.31)	PERVIOUS (i) (0.05)	
Dep. Storage (mm)	1.00	1.00	
Average Slope (%)	1.00	2.00	
Length (m)	48.99	40.00	

Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.15 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.168 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 45.86
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20091) Area (ha)= 0.40
 ID= 1 DT= 5.0 min Total Imp(%)= 77.00 Dir. Conn.(%)= 77.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 51.64 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		TIME RAIN		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.66 (ii) 6.26 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.09 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.104 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 45.09
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200812) Area (ha)= 0.38
 ID= 1 DT= 5.0 min Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 50.33 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110) AREA QPEAK TPEAK R.V.
 ID= 1 (200811): 0.07 0.020 1.33 46.42
 ID= 2 (200812): 0.38 0.087 1.33 43.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (200822) Area (ha)= 0.39
 ID= 1 DT= 5.0 min Total Imp(%)= 59.00 Dir. Conn.(%)= 59.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 50.99 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		TIME RAIN		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 15.00
 Storage Coeff. (min)= 1.65 (ii) 10.85 (ii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.32 0.09

PEAK FLOW (cms)= 0.07 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.079 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 41.62
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200821) Area (ha)= 0.07
 ID= 1 DT= 5.0 min Total Imp(%)= 83.00 Dir. Conn.(%)= 83.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 21.60 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		TIME RAIN		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 15.00
 Storage Coeff. (min)= 1.64 (ii) 10.84 (ii)
 Unit Hyd. Tpeak (min)= 5.00 15.00
 Unit Hyd. peak (cms)= 0.32 0.09

PEAK FLOW (cms)= 0.08 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.50 0.087 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 43.74
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200811) Area (ha)= 0.07
 ID= 1 DT= 5.0 min Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 21.60 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		TIME RAIN		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 0.98 (ii) 4.83 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.22

PEAK FLOW (cms)= 0.02 0.00 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.020 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 46.42
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

TIME RAIN		TIME RAIN		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 0.98 (ii) 4.94 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.22

PEAK FLOW (cms)= 0.02 0.00 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.020 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 46.23
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0111) AREA QPEAK TPEAK R.V.
 ID= 1 (200821): 0.07 0.020 1.33 46.23
 ID= 2 (200822): 0.39 0.079 1.33 41.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (20102) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 74.00 Dir. Conn.(%)= 74.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		TIME RAIN		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.74 (ii) 6.65 (ii)

Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.14

PEAK FLOW (cms)= 0.10 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.119 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 44.51
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20101) Area (ha)= 0.25
 ID= 1 DT= 5.0 min Total Imp(%)= 82.00 Dir. Conn.(%)= 82.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.20 0.05
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 40.82 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12
0.167	3.57	1.167	25.64	2.167	8.12
0.250	4.07	1.250	108.92	2.250	6.96
0.333	4.07	1.333	108.92	2.333	6.96
0.417	4.76	1.417	33.31	2.417	6.12
0.500	4.76	1.500	33.31	2.500	6.12
0.583	5.79	1.583	17.99	2.583	5.48
0.667	5.79	1.667	17.99	2.667	5.48
0.750	7.53	1.750	12.60	2.750	4.97
0.833	7.53	1.833	12.60	2.833	4.97
0.917	11.20	1.917	9.82	2.917	4.56
1.000	11.20	2.000	9.82	3.000	4.56

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.44 (ii) 1.33 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.06 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.068 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 46.05
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0121)

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID= 1 (20101):	0.25	0.068	1.33
+ ID= 2 (20102):	0.47	0.119	1.33
-----	-----	-----	-----
ID = 3 (0121):	0.72	0.187	1.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (20122) Area (ha)= 0.90
 ID= 1 DT= 5.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.90 0.21
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 86.02 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.31 0.14

PEAK FLOW (cms)= 0.22 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.237 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 45.67
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20122) Area (ha)= 1.11
 ID= 1 DT= 5.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.90 0.21
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 86.02 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12
0.167	3.57	1.167	25.64	2.167	8.12
0.250	4.07	1.250	108.92	2.250	6.96
0.333	4.07	1.333	108.92	2.333	6.96
0.417	4.76	1.417	33.31	2.417	6.12
0.500	4.76	1.500	33.31	2.500	6.12
0.583	5.79	1.583	17.99	2.583	5.48
0.667	5.79	1.667	17.99	2.667	5.48
0.750	7.53	1.750	12.60	2.750	4.97
0.833	7.53	1.833	12.60	2.833	4.97
0.917	11.20	1.917	9.82	2.917	4.56
1.000	11.20	2.000	9.82	3.000	4.56

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 2.26 (ii) 6.43 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.30 0.14

PEAK FLOW (cms)= 0.27 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.294 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 45.87
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (201021) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.40 0.07
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

STANDHYD (20111) Area (ha)= 0.87
 ID= 1 DT= 5.0 min Total Imp(%)= 67.00 Dir. Conn.(%)= 67.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.20 0.20
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 76.16 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12
0.167	3.57	1.167	25.64	2.167	8.12
0.250	4.07	1.250	108.92	2.250	6.96
0.333	4.07	1.333	108.92	2.333	6.96
0.417	4.76	1.417	33.31	2.417	6.12
0.500	4.76	1.500	33.31	2.500	6.12
0.583	5.79	1.583	17.99	2.583	5.48
0.667	5.79	1.667	17.99	2.667	5.48
0.750	7.53	1.750	12.60	2.750	4.97
0.833	7.53	1.833	12.60	2.833	4.97
0.917	11.20	1.917	9.82	2.917	4.56
1.000	11.20	2.000	9.82	3.000	4.56

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 2.10 (ii) 11.30 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.31 0.09

PEAK FLOW (cms)= 0.18 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.30 0.192 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 43.17
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20112) Area (ha)= 0.90
 ID= 1 DT= 5.0 min Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.72 0.18
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 77.46 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12
0.167	3.57	1.167	25.64	2.167	8.12
0.250	4.07	1.250	108.92	2.250	6.96
0.333	4.07	1.333	108.92	2.333	6.96
0.417	4.76	1.417	33.31	2.417	6.12
0.500	4.76	1.500	33.31	2.500	6.12
0.583	5.79	1.583	17.99	2.583	5.48
0.667	5.79	1.667	17.99	2.667	5.48
0.750	7.53	1.750	12.60	2.750	4.97
0.833	7.53	1.833	12.60	2.833	4.97
0.917	11.20	1.917	9.82	2.917	4.56
1.000	11.20	2.000	9.82	3.000	4.56

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 2.12 (ii) 6.40 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.12 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.129 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 46.63
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (201022) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.40 0.07
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12
0.167	3.57	1.167	25.64	2.167	8.12
0.250	4.07	1.250	108.92	2.250	6.96
0.333	4.07	1.333	108.92	2.333	6.96
0.417	4.76	1.417	33.31	2.417	6.12
0.500	4.76	1.500	33.31	2.500	6.12
0.583	5.79	1.583	17.99	2.583	5.48
0.667	5.79	1.667	17.99	2.667	5.48
0.750	7.53	1.750	12.60	2.750	4.97
0.833	7.53	1.833	12.60	2.833	4.97
0.917	11.20	1.917	9.82	2.917	4.56
1.000	11.20	2.000	9.82	3.000	4.56

Max.Eff.Inten.(mm/hr)= 108.92 64.33
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.74 (ii) 5.47 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.12 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.129 (iii)
 RUNOFF VOLUME (mm)= 49.52 30.31 46.63
 TOTAL RAINFALL (mm)= 50.52 50.52 50.52
 RUNOFF COEFFICIENT = 0.98 0.60 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (201021) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.40 0.07
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (201021):	0.47	0.129	1.33	46.63
+ ID2 = 2 (201022):	0.47	0.129	1.33	46.63
ID = 3 (0136):	0.94	0.259	1.33	46.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2005) ID= 1 DT= 5.0 min	Area (ha)= 0.28	Dir. Conn.(%)= 90.00
	Total Imp (%) = 90.00	
Surface Area (ha)= 0.25	IMPERVIOUS (i)	PERVIOUS (i)
Dep. Storage (mm)= 1.00	0.03	0.03
Average Slope (%)= 1.00	1.00	1.00
Length (m)= 43.20	40.00	40.00
Mannings n = 0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92	64.33
over (min)= 5.00	5.00
Storage Coeff. (min)= 1.49 (ii)	4.58 (ii)
Unit Hyd. Tpeak (min)= 5.00	5.00
Unit Hyd. peak (cms)= 0.33	0.23
PEAK FLOW (cms)= 0.08	0.00
TIME TO PEAK (hrs)= 1.33	1.33
RUNOFF VOLUME (mm)= 49.52	30.31
TOTAL RAINFALL (mm)= 50.52	50.52
RUNOFF COEFFICIENT = 0.98	0.60

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20121) ID= 1 DT= 5.0 min	Area (ha)= 1.11	Dir. Conn.(%)= 56.00
	Total Imp (%) = 56.00	
Surface Area (ha)= 0.43	IMPERVIOUS (i)	PERVIOUS (i)
Dep. Storage (mm)= 1.00	1.00	1.00
Average Slope (%)= 1.00	2.00	2.00
Length (m)= 86.02	40.00	40.00
Mannings n = 0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92	64.33
over (min)= 5.00	5.00
Storage Coeff. (min)= 1.13 (ii)	4.61 (ii)
Unit Hyd. Tpeak (min)= 5.00	5.00
Unit Hyd. peak (cms)= 0.34	0.22
PEAK FLOW (cms)= 0.03	0.00
TIME TO PEAK (hrs)= 1.33	1.33
RUNOFF VOLUME (mm)= 49.52	30.31
TOTAL RAINFALL (mm)= 50.52	50.52
RUNOFF COEFFICIENT = 0.98	0.60

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200121) ID= 1 DT= 5.0 min	Area (ha)= 0.11	Dir. Conn.(%)= 87.00
	Total Imp (%) = 87.00	
Surface Area (ha)= 0.10	IMPERVIOUS (i)	PERVIOUS (i)
Dep. Storage (mm)= 1.00	1.00	1.00
Average Slope (%)= 1.00	2.00	2.00
Length (m)= 27.08	40.00	40.00
Mannings n = 0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92	64.33
over (min)= 5.00	5.00
Storage Coeff. (min)= 1.13 (ii)	4.61 (ii)
Unit Hyd. Tpeak (min)= 5.00	5.00
Unit Hyd. peak (cms)= 0.34	0.22
PEAK FLOW (cms)= 0.03	0.00
TIME TO PEAK (hrs)= 1.33	1.33
RUNOFF VOLUME (mm)= 49.52	30.31
TOTAL RAINFALL (mm)= 50.52	50.52
RUNOFF COEFFICIENT = 0.98	0.60

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (200111):	0.10	0.029	1.33	47.49
+ ID2 = 2 (200121):	0.11	0.031	1.33	47.00
ID = 3 (0142):	0.21	0.060	1.33	47.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200112) ID= 1 DT= 5.0 min	Area (ha)= 0.10	Dir. Conn.(%)= 79.00
	Total Imp (%) = 79.00	
Surface Area (ha)= 0.08	IMPERVIOUS (i)	PERVIOUS (i)
Dep. Storage (mm)= 1.00	0.02	0.02
Average Slope (%)= 1.00	1.00	1.00
Length (m)= 25.69	40.00	40.00
Mannings n = 0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92	64.33
over (min)= 5.00	5.00
Storage Coeff. (min)= 1.19 (ii)	4.79 (ii)
Unit Hyd. Tpeak (min)= 5.00	5.00
Unit Hyd. peak (cms)= 0.33	0.22
PEAK FLOW (cms)= 0.03	0.00
TIME TO PEAK (hrs)= 1.33	1.33
RUNOFF VOLUME (mm)= 49.52	30.31
TOTAL RAINFALL (mm)= 50.52	50.52
RUNOFF COEFFICIENT = 0.98	0.60

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68
0.583	5.79	1.583	17.99	2.583	5.48	3.58	3.47
0.667	5.79	1.667	17.99	2.667	5.48	3.67	3.47
0.750	7.53	1.750	12.60	2.750	4.97	3.75	3.28
0.833	7.53	1.833	12.60	2.833	4.97	3.83	3.28
0.917	11.20	1.917	9.82	2.917	4.56	3.92	3.12
1.000	11.20	2.000	9.82	3.000	4.56	4.00	3.12

Max.Eff.Inten.(mm/hr)= 108.92	64.33
over (min)= 5.00	5.00
Storage Coeff. (min)= 1.26 (ii)	11.46 (ii)
Unit Hyd. Tpeak (min)= 5.00	5.00
Unit Hyd. peak (cms)= 0.30	0.09
PEAK FLOW (cms)= 0.19	0.05
TIME TO PEAK (hrs)= 1.33	1.50
RUNOFF VOLUME (mm)= 49.52	30.31
TOTAL RAINFALL (mm)= 50.52	50.52
RUNOFF COEFFICIENT = 0.98	0.60

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200111) ID= 1 DT= 5.0 min	Area (ha)= 0.10	Dir. Conn.(%)= 89.50
	Total Imp (%) = 89.50	
Surface Area (ha)= 0.09	IMPERVIOUS (i)	PERVIOUS (i)
Dep. Storage (mm)= 1.00	1.00	1.00
Average Slope (%)= 1.00	2.00	2.00
Length (m)= 25.82	40.00	40.00
Mannings n = 0.013	0.290	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.57	1.083	25.64	2.083	8.12	3.08	4.22
0.167	3.57	1.167	25.64	2.167	8.12	3.17	4.22
0.250	4.07	1.250	108.92	2.250	6.96	3.25	3.93
0.333	4.07	1.333	108.92	2.333	6.96	3.33	3.93
0.417	4.76	1.417	33.31	2.417	6.12	3.42	3.68
0.500	4.76	1.500	33.31	2.500	6.12	3.50	3.68

CN^e = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Length (m) = 73.03 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

ADD HYD (0145)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (200112):	0.10	0.026	1.33	45.46
+ ID2 = 2 (200122):	0.13	0.037	1.33	46.81
ID = 3 (0145):	0.23	0.063	1.33	46.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 6.1.2002)
 V V I SS U U A A L
 V V I SS U U A A L
 V V I SS U U U U A A L L L L L L
 W I I SSSSS U U U U U A A L L L L L L

000 TTTT TTTT H H Y Y M M M 000 TM
 O O T T H H Y Y M M M O O
 O O T T H H Y Y M M O O
 000 T T H Y Y M M 000

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2020 Smart City Water Inc
 All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\visual\OTHYMO 6.1\VO2\voind.dat
 Output filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\219ec28a
 Summary filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\219ec28a

DATE: 05-18-2022 TIME: 12:44:42

USER:

COMMENTS:

***** SIMULATION : Run 03 *****

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	4.32	1.083	30.27	2.083	9.72	3.083	3.08
0.167	4.32	1.167	30.27	2.167	9.72	3.167	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.250	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.333	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.417	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.500	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.583	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.667	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.750	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.833	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.917	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.000	3.77

Area (ha)	Impervious (%)	Pervious (i)
0.80	0.68	0.12
85.00	1.00	1.00
	1.00	2.00

RUNOFF COEFFICIENT = 0.98 0.64 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
 CN^e = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha)	Impervious (%)	Pervious (i)
0.43	0.39	0.04
90.00	1.00	1.00
	1.00	2.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	4.32	1.083	30.27	2.083	9.72	3.083	3.08
0.167	4.32	1.167	30.27	2.167	9.72	3.167	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.250	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.333	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.417	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.500	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.583	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.667	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.750	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.833	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.917	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.000	3.77

Max. Eff. Inten. (mm/hr) = 126.55 80.98
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.60 (ii) 4.51 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.32 0.23

PEAK FLOW (cms) = 0.14 0.01 *TOTALS* (iii)
 TIME TO PEAK (hrs) = 1.33 1.33
 RUNOFF VOLUME (mm) = 58.72 38.27 56.67
 TOTAL RAINFALL (mm) = 59.72 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
 CN^e = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha)	Impervious (%)	Pervious (i)
0.47	0.41	0.06
87.00	1.00	1.00
	1.00	5.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	4.32	1.083	30.27	2.083	9.72	3.083	3.08
0.167	4.32	1.167	30.27	2.167	9.72	3.167	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.250	4.74

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	4.32	1.083	30.27	2.083	9.72	3.083	3.08
0.167	4.32	1.167	30.27	2.167	9.72	3.167	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.250	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.333	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.417	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.500	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.583	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.667	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.750	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.833	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.917	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.000	3.77

Max. Eff. Inten. (mm/hr) = 126.55 80.98
 over (min) = 5.00 10.00
 Storage Coeff. (min) = 1.93 (ii) 5.43 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.31 0.16

PEAK FLOW (cms) = 0.24 0.02 *TOTALS* (iii)
 TIME TO PEAK (hrs) = 1.42 1.42
 RUNOFF VOLUME (mm) = 58.72 38.27 55.65
 TOTAL RAINFALL (mm) = 59.72 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
 CN^e = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha)	Impervious (%)	Pervious (i)
0.70	0.62	0.08
89.00	1.00	1.00
	2.00	2.00

Surface Area (ha) = 0.62 0.08
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 68.31 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	4.32	1.083	30.27	2.083	9.72	3.083	3.08
0.167	4.32	1.167	30.27	2.167	9.72	3.167	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.250	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.333	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.417	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.500	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.583	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.667	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.750	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.833	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.917	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.000	3.77

Max. Eff. Inten. (mm/hr) = 126.55 80.98
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.85 (ii) 4.89 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.32 0.22

PEAK FLOW (cms) = 0.22 0.02 *TOTALS* (iii)
 TIME TO PEAK (hrs) = 1.33 1.33
 RUNOFF VOLUME (mm) = 58.72 38.27 56.47
 TOTAL RAINFALL (mm) = 59.72 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.333	4.91	1.333	126.55	2.333	8.53	3.333	4.74
0.500	5.73	1.500	39.22	2.500	7.35	3.500	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.583	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.667	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.750	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.833	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.917	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.000	3.77

Max. Eff. Inten. (mm/hr) = 126.55 80.98
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.85 (ii) 4.92 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.32 0.22

PEAK FLOW (cms) = 0.14 0.01 *TOTALS* (iii)
 TIME TO PEAK (hrs) = 1.33 1.33
 RUNOFF VOLUME (mm) = 58.72 38.27 56.06
 TOTAL RAINFALL (mm) = 59.72 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
 CN^e = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha)	Impervious (%)	Pervious (i)
0.35	0.31	0.04
89.00	1.00	1.00
	1.00	2.00

Surface Area (ha) = 0.31 0.04
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 48.30 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	4.32	1.083	30.27	2.083	9.72	3.083	3.08
0.167	4.32	1.167	30.27	2.167	9.72	3.167	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.250	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.333	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.417	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.500	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.583	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.667	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.750	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.833	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.917	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.000	3.77

Max. Eff. Inten. (mm/hr) = 126.55 80.98
 over (min) = 5.

CALIB
STANDHYD (20042)
ID= 1 DT= 5.0 min

Area (ha)= 0.36
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.31 0.05
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max.Eff.Inten.(mm/hr)= 126.55 80.98
over (min) 5.00 10.00
Storage Coeff. (min)= 1.52 (ii) 5.02 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.11 0.01 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 1.33 (iii)
RUNOFF VOLUME (mm)= 58.72 38.27 55.65
TOTAL RAINFALL (mm)= 59.72 59.72 59.72
RUNOFF COEFFICIENT = 0.98 0.64 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20052)
ID= 1 DT= 5.0 min

Area (ha)= 0.29
Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.23 0.06
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 43.97 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max.Eff.Inten.(mm/hr)= 126.55 80.98
over (min) 5.00 10.00
Storage Coeff. (min)= 1.87 (ii) 5.91 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.20 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 58.72 38.27 54.63
TOTAL RAINFALL (mm)= 59.72 59.72 59.72
RUNOFF COEFFICIENT = 0.98 0.64 0.91

Storage Coeff. (min)= 1.42 (ii) 5.65 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15

TOTALS
(iii)
PEAK FLOW (cms)= 0.08 0.01 0.089 (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 58.72 38.27 54.21
TOTAL RAINFALL (mm)= 59.72 59.72 59.72
RUNOFF COEFFICIENT = 0.98 0.64 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20062)
ID= 1 DT= 5.0 min

Area (ha)= 0.73
Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.58 0.15
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 69.76 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max.Eff.Inten.(mm/hr)= 126.55 80.98
over (min) 5.00 10.00
Storage Coeff. (min)= 1.87 (ii) 5.91 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.20 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 58.72 38.27 54.63
TOTAL RAINFALL (mm)= 59.72 59.72 59.72
RUNOFF COEFFICIENT = 0.98 0.64 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20061)
ID= 1 DT= 5.0 min

Area (ha)= 0.65
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.55 0.10
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 65.18 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max.Eff.Inten.(mm/hr)= 126.55 80.98
over (min) 5.00 10.00
Storage Coeff. (min)= 1.75 (ii) 5.26 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.17 0.02 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 58.72 38.27 55.65
TOTAL RAINFALL (mm)= 59.72 59.72 59.72
RUNOFF COEFFICIENT = 0.98 0.64 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20071)
ID= 1 DT= 5.0 min

Area (ha)= 0.58
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.49 0.09
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.18 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max.Eff.Inten.(mm/hr)= 126.55 80.98
over (min) 5.00 10.00
Storage Coeff. (min)= 1.75 (ii) 5.26 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.17 0.02 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 58.72 38.27 55.65
TOTAL RAINFALL (mm)= 59.72 59.72 59.72
RUNOFF COEFFICIENT = 0.98 0.64 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20072)
ID= 1 DT= 5.0 min

Area (ha)= 0.64
Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.54 0.14
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 65.32 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max.Eff.Inten.(mm/hr)= 126.55 80.98
over (min) 5.00 10.00
Storage Coeff. (min)= 1.80 (ii) 5.42 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.19 0.02 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 58.72 38.27 55.45
TOTAL RAINFALL (mm)= 59.72 59.72 59.72
RUNOFF COEFFICIENT = 0.98 0.64 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35		

0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr) over (min)	126.55	80.98	10.00
Storage Coeff. (min)	1.79 (ii)	5.72 (ii)	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)	0.32	0.15	
PEAK FLOW (cms) (hrs)	0.18	0.02	*TOTALS* 0.198 (iii)
TIME TO PEAK (hrs)	1.33	1.42	1.33
RUNOFF VOLUME (mm)	58.72	38.27	54.83
TOTAL RAINFALL (mm)	59.72	59.72	59.72
RUNOFF COEFFICIENT	0.98	0.64	0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20091)	Area (ha)	0.40
ID= 1 DT= 5.0 min	Total Imp (%)	77.00
	Dir. Conn. (%)	77.00
Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	0.01	0.00
Average Slope (%)	1.00	1.00
Length (m)	51.64	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr) over (min)	126.55	80.98	10.00
Storage Coeff. (min)	1.56 (ii)	5.89 (ii)	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)	0.33	0.15	
PEAK FLOW (cms) (hrs)	0.11	0.02	*TOTALS* 0.122 (iii)
TIME TO PEAK (hrs)	1.33	1.42	1.33
RUNOFF VOLUME (mm)	58.72	38.27	54.01
TOTAL RAINFALL (mm)	59.72	59.72	59.72
RUNOFF COEFFICIENT	0.98	0.64	0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (200812)	Area (ha)	0.38
ID= 1 DT= 5.0 min	Total Imp (%)	70.00
	Dir. Conn. (%)	70.00
Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	0.06	0.01
Average Slope (%)	1.00	1.00
Length (m)	21.60	40.00
Mannings n	0.013	0.290

PEAK FLOW (cms) (hrs)	0.02	0.00	0.023 (iii)
TIME TO PEAK (hrs)	1.33	1.42	1.33
RUNOFF VOLUME (mm)	58.72	38.27	55.43
TOTAL RAINFALL (mm)	59.72	59.72	59.72
RUNOFF COEFFICIENT	0.98	0.64	0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (200811):	0.07	0.023	1.33	55.43
+ ID2 = 2 (200812):	0.38	0.107	1.33	52.57
ID = 3 (0110):	0.45	0.130	1.33	53.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (200822)	Area (ha)	0.39
ID= 1 DT= 5.0 min	Total Imp (%)	59.00
	Dir. Conn. (%)	59.00
Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	0.23	0.16
Average Slope (%)	1.00	1.00
Length (m)	50.99	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr) over (min)	126.55	80.98	10.00
Storage Coeff. (min)	1.55 (ii)	9.95 (ii)	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)	0.33	0.11	
PEAK FLOW (cms) (hrs)	0.08	0.02	*TOTALS* 0.100 (iii)
TIME TO PEAK (hrs)	1.33	1.42	1.33
RUNOFF VOLUME (mm)	58.72	38.27	50.32
TOTAL RAINFALL (mm)	59.72	59.72	59.72
RUNOFF COEFFICIENT	0.98	0.64	0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (200821)	Area (ha)	0.07
ID= 1 DT= 5.0 min	Total Imp (%)	83.00
	Dir. Conn. (%)	83.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	0.27	0.11
Average Slope (%)	1.00	1.00
Length (m)	50.33	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr) over (min)	126.55	80.98	10.00
Storage Coeff. (min)	1.54 (ii)	9.93 (ii)	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)	0.33	0.11	
PEAK FLOW (cms) (hrs)	0.09	0.02	*TOTALS* 0.107 (iii)
TIME TO PEAK (hrs)	1.33	1.42	1.33
RUNOFF VOLUME (mm)	58.72	38.27	52.57
TOTAL RAINFALL (mm)	59.72	59.72	59.72
RUNOFF COEFFICIENT	0.98	0.64	0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (200811)	Area (ha)	0.07
ID= 1 DT= 5.0 min	Total Imp (%)	84.00
	Dir. Conn. (%)	84.00
Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	0.06	0.01
Average Slope (%)	1.00	1.00
Length (m)	21.60	40.00
Mannings n	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr) over (min)	126.55	80.98	10.00
Storage Coeff. (min)	0.93 (ii)	5.00 (ii)	
Unit Hyd. Tpeak (min)	5.00	5.00	
Unit Hyd. peak (cms)	0.34	0.23	
PEAK FLOW (cms) (hrs)	0.02	0.00	*TOTALS* 0.023 (iii)
TIME TO PEAK (hrs)	1.33	1.33	1.33
RUNOFF VOLUME (mm)	58.72	38.27	55.25
TOTAL RAINFALL (mm)	59.72	59.72	59.72
RUNOFF COEFFICIENT	0.98	0.64	0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr) over (min)	126.55	80.98	10.00
Storage Coeff. (min)	0.93 (ii)	4.65 (ii)	
Unit Hyd. Tpeak (min)	5.00	5.00	
Unit Hyd. peak (cms)	0.34	0.22	
PEAK FLOW (cms) (hrs)	0.02	0.00	*TOTALS* 0.023 (iii)
TIME TO PEAK (hrs)	1.33	1.33	1.33
RUNOFF VOLUME (mm)	58.72	38.27	55.25
TOTAL RAINFALL (mm)	59.72	59.72	59.72
RUNOFF COEFFICIENT	0.98	0.64	0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0111)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (200821):	0.07	0.023	1.33	55.2

0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)= 126.55 80.98
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.64 (ii) 6.26 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.15

TOTALS
 PEAK FLOW (cms)= 0.12 0.02 0.140 (iii)
 TIME TO PEAK (hrs)= 1.33 1.42 1.33
 RUNOFF VOLUME (mm)= 58.72 38.27 53.40
 TOTAL RAINFALL (mm)= 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20111) Area (ha)= 0.25
 ID= 1 DT= 5.0 min Total Imp(%)= 82.00 Dir. Conn.(%)= 82.00

Surface Area (ha)= 0.20 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 40.82 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)= 126.55 80.98
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.36 (ii) 5.19 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.16

TOTALS
 PEAK FLOW (cms)= 0.07 0.01 0.079 (iii)
 TIME TO PEAK (hrs)= 1.33 1.42 1.33
 RUNOFF VOLUME (mm)= 58.72 38.27 55.03
 TOTAL RAINFALL (mm)= 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (01211) AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 1 + 2 = 3

0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)= 126.55 80.98
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.39 (ii) 6.03 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.31 0.15

TOTALS
 PEAK FLOW (cms)= 0.25 0.03 0.279 (iii)
 TIME TO PEAK (hrs)= 1.33 1.42 1.33
 RUNOFF VOLUME (mm)= 58.72 38.27 54.63
 TOTAL RAINFALL (mm)= 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20122) Area (ha)= 1.11
 ID= 1 DT= 5.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

Surface Area (ha)= 0.90 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 86.02 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)= 126.55 80.98
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 2.12 (ii) 6.06 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.31 0.15

TOTALS
 PEAK FLOW (cms)= 0.31 0.04 0.346 (iii)
 TIME TO PEAK (hrs)= 1.33 1.42 1.33
 RUNOFF VOLUME (mm)= 58.72 38.27 54.83
 TOTAL RAINFALL (mm)= 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (201021) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.40 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n = 0.013 0.290

ID1= 1 (20101):	0.25	0.079	1.33	55.03
+ ID2= 2 (20102):	0.47	0.140	1.33	53.40
=====	=====	=====	=====	=====
ID = 3 (01211):	0.72	0.220	1.33	53.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (20111) Area (ha)= 0.87
 ID= 1 DT= 5.0 min Total Imp(%)= 67.00 Dir. Conn.(%)= 67.00

Surface Area (ha)= 0.58 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 76.16 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)= 126.55 80.98
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.97 (ii) 10.37 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.31 0.09

TOTALS
 PEAK FLOW (cms)= 0.20 0.04 0.226 (iii)
 TIME TO PEAK (hrs)= 1.33 1.50 1.33
 RUNOFF VOLUME (mm)= 58.72 38.27 51.97
 TOTAL RAINFALL (mm)= 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20112) Area (ha)= 0.90
 ID= 1 DT= 5.0 min Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

Surface Area (ha)= 0.40 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 77.46 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)= 126.55 80.98
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.64 (ii) 5.15 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

TOTALS
 PEAK FLOW (cms)= 0.14 0.01 0.152 (iii)
 TIME TO PEAK (hrs)= 1.33 1.42 1.33
 RUNOFF VOLUME (mm)= 58.72 38.27 55.65
 TOTAL RAINFALL (mm)= 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (201022) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.40 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)= 126.55 80.98
 over (min) = 5.00 10.00
 Storage Coeff. (min)= 1.64 (ii) 5.15 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

TOTALS
 PEAK FLOW (cms)= 0.14 0.01 0.152 (iii)
 TIME TO PEAK (hrs)= 1.33 1.42 1.33
 RUNOFF VOLUME (mm)= 58.72 38.27 55.65
 TOTAL RAINFALL (mm)= 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (201021) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.40 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 55.98 40.00
 Mannings n =

PEAK FLOW (cms)=	0.14	0.01	0.152 (iii)
TIME TO PEAK (hrs)=	1.33	1.42	1.33
RUNOFF VOLUME (mm)=	58.72	38.27	55.65
TOTAL RAINFALL (mm)=	59.72	59.72	59.72
RUNOFF COEFFICIENT =	0.98	0.64	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (201021):	0.47	0.152	1.33	55.65
+ ID2= 2 (201022):	0.47	0.152	1.33	55.65

ID = 3 (0136):	0.94	0.303	1.33	55.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2005)	Area (ha)=	0.28
ID= 1 DT= 5.0 min	Total Imp(%)=	90.00
	Dir. Conn.(%)=	90.00
	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.03	0.01
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	43.20	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)=	126.55	80.98	
over (min)	5.00	5.00	
Storage Coeff. (min)=	1.41 (ii)	4.32 (ii)	
Unit Hyd. Tpeak (min)=	5.00	5.00	
Unit Hyd. peak (cms)=	0.33	0.23	
PEAK FLOW (cms)=	0.09	0.01	*TOTALS*
TIME TO PEAK (hrs)=	1.33	1.33	0.095 (iii)
RUNOFF VOLUME (mm)=	58.72	38.27	56.67
TOTAL RAINFALL (mm)=	59.72	59.72	59.72
RUNOFF COEFFICIENT =	0.98	0.64	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20121)	Area (ha)=	1.11
ID= 1 DT= 5.0 min	Total Imp(%)=	56.00
	Dir. Conn.(%)=	56.00

PEAK FLOW (cms)=	0.03	0.00	0.034 (iii)
TIME TO PEAK (hrs)=	1.33	1.33	1.33
RUNOFF VOLUME (mm)=	58.72	38.27	56.67
TOTAL RAINFALL (mm)=	59.72	59.72	59.72
RUNOFF COEFFICIENT =	0.98	0.64	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200121)	Area (ha)=	0.11
ID= 1 DT= 5.0 min	Total Imp(%)=	87.00
	Dir. Conn.(%)=	87.00

Surface Area (ha)=	0.10	0.01
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	27.08	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)=	126.55	80.98	
over (min)	5.00	5.00	
Storage Coeff. (min)=	1.06 (ii)	4.34 (ii)	
Unit Hyd. Tpeak (min)=	5.00	5.00	
Unit Hyd. peak (cms)=	0.34	0.23	
PEAK FLOW (cms)=	0.03	0.00	*TOTALS*
TIME TO PEAK (hrs)=	1.33	1.33	0.037 (iii)
RUNOFF VOLUME (mm)=	58.72	38.27	56.04
TOTAL RAINFALL (mm)=	59.72	59.72	59.72
RUNOFF COEFFICIENT =	0.98	0.64	0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (200111):	0.10	0.034	1.33	56.56
+ ID2= 2 (200121):	0.11	0.037	1.33	56.04

ID = 3 (0142):	0.21	0.071	1.33	56.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (200122)	Area (ha)=	0.10
ID= 1 DT= 5.0 min	Total Imp(%)=	79.00
	Dir. Conn.(%)=	79.00

Surface Area (ha)=	IMPERVIOUS 0.62	PERVIOUS (i) 0.49
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	86.02	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)=	126.55	80.98	
over (min)	5.00	5.00	
Storage Coeff. (min)=	2.12 (ii)	10.52 (ii)	
Unit Hyd. Tpeak (min)=	5.00	5.00	
Unit Hyd. peak (cms)=	0.31	0.09	
PEAK FLOW (cms)=	0.22	0.06	*TOTALS*
TIME TO PEAK (hrs)=	1.33	1.50	0.254 (iii)
RUNOFF VOLUME (mm)=	58.72	38.27	49.72
TOTAL RAINFALL (mm)=	59.72	59.72	59.72
RUNOFF COEFFICIENT =	0.98	0.64	0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200111)	Area (ha)=	0.10
ID= 1 DT= 5.0 min	Total Imp(%)=	89.50
	Dir. Conn.(%)=	89.50

Surface Area (ha)=	IMPERVIOUS 0.09	PERVIOUS (i) 0.01
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	25.82	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09
0.167	4.32	1.167	30.27	2.167	9.72	3.17	5.09
0.250	4.91	1.250	126.55	2.250	8.53	3.25	4.74
0.333	4.91	1.333	126.55	2.333	8.53	3.33	4.74
0.417	5.73	1.417	39.22	2.417	7.35	3.42	4.45
0.500	5.73	1.500	39.22	2.500	7.35	3.50	4.45
0.583	6.96	1.583	21.35	2.583	6.59	3.58	4.19
0.667	6.96	1.667	21.35	2.667	6.59	3.67	4.19
0.750	9.03	1.750	15.01	2.750	5.99	3.75	3.97
0.833	9.03	1.833	15.01	2.833	5.99	3.83	3.97
0.917	13.36	1.917	11.74	2.917	5.50	3.92	3.77
1.000	13.36	2.000	11.74	3.000	5.50	4.00	3.77

Max. Eff. Inten. (mm/hr)=	126.55	80.98	
over (min)	5.00	5.00	
Storage Coeff. (min)=	1.03 (ii)	4.01 (ii)	
Unit Hyd. Tpeak (min)=	5.00	5.00	
Unit Hyd. peak (cms)=	0.34	0.24	
PEAK FLOW (cms)=	0.03	0.00	*TOTALS*
TIME TO PEAK (hrs)=	1.03	1.00	0.031 (iii)
RUNOFF VOLUME (mm)=	58.72	38.27	54.40
TOTAL RAINFALL (mm)=	59.72	59.72	59.72
RUNOFF COEFFICIENT =	0.98	0.64	0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200122)	Area (ha)=	0.13
ID= 1 DT= 5.0 min	Total Imp(%)=	86.00
	Dir. Conn.(%)=	86.00

Surface Area (ha)=	IMPERVIOUS 0.11	PERVIOUS (i) 0.02
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	29.44	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.32	1.083	30.27	2.083	9.72	3.08	5.09

PEAK FLOW (cms)= 0.04 0.00 0.043 (iii)
 TIME TO PEAK (hrs)= 1.33 1.33
 RUNOFF VOLUME (mm)= 58.72 38.27 55.84
 TOTAL RAINFALL (mm)= 59.72 59.72
 RUNOFF COEFFICIENT = 0.98 0.64 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (200112):	0.10	0.031	1.33	54.40
+ ID2= 2 (200122):	0.13	0.043	1.33	55.84
ID = 3 (0145):	0.23	0.074	1.33	55.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 6.1.2002)
 V V I SS U U A A L
 V V I SS U U AAAAA L
 V V I SS U U A A L
 W I SSSSS UUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO
 Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2020 Smart City Water Inc
 All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual THYMO 6.1\VO2\vo.in.dat
 Output filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\b4bab8e4
 Summary filename: C:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\b4bab8e4

DATE: 05-18-2022 TIME: 12:44:43
 USER:

COMMENTS:

***** SIMULATION : Run 04 *****

READ STORM Filename: C:\Users\caef070146\AppData\Local\Temp\11403a02-80e0-48b3-97a8-b203216ba841\53217781
 Ptotal= 71.24 mm Comments: 25-year, 4 hr-Chicago, City of Barrie

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.17	5.22	1.17	36.37	2.17	11.74	3.17	6.15
0.33	5.94	1.33	148.15	2.33	10.09	3.33	5.74
0.50	6.93	1.50	47.06	2.50	8.89	3.50	5.38
0.67	8.42	1.67	25.72	2.67	7.96	3.67	5.08
0.83	10.91	1.83	18.11	2.83	7.24	3.83	4.80
1.00	16.13	2.00	14.17	3.00	6.65	4.00	4.57

Storage over (min)= 5.00 5.00
 over (min)= 1.74 (ii) 4.58 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.32 0.23

PEAK FLOW (cms)= 0.26 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.276 (iii)
 RUNOFF VOLUME (mm)= 70.24 48.54 67.85
 TOTAL RAINFALL (mm)= 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20031) Area (ha)= 0.43
 ID= 1 DT= 5.0 min Total Imp(%)= 90.00 Dir. Conn.(%)= 90.00

Surface Area (ha)= 0.39 IMPERVIOUS 0.04 PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 53.54 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 1.50 (ii) 4.24 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.33 0.24

PEAK FLOW (cms)= 0.16 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.171 (iii)
 RUNOFF VOLUME (mm)= 70.24 48.54 68.06
 TOTAL RAINFALL (mm)= 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20032) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00

Surface Area (ha)= 0.47 IMPERVIOUS 0.04 PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 53.58 40.00
 Mannings n = 0.013 0.290

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

CALIB STANDHYD (20022) Area (ha)= 0.80
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.68 IMPERVIOUS 0.12 PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 73.03 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.81 (ii) 5.08 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.28 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.304 (iii)
 RUNOFF VOLUME (mm)= 70.24 48.54 66.98
 TOTAL RAINFALL (mm)= 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20021) Area (ha)= 0.70
 ID= 1 DT= 5.0 min Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

Surface Area (ha)= 0.62 IMPERVIOUS 0.08 PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 68.31 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 1.54 (ii) 4.62 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.17 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.184 (iii)
 RUNOFF VOLUME (mm)= 70.24 48.54 67.41
 TOTAL RAINFALL (mm)= 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20041) Area (ha)= 0.35
 ID= 1 DT= 5.0 min Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

Surface Area (ha)= 0.31 IMPERVIOUS 0.04 PERVIOUS (i)
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 48.30 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
 over (min)= 5.00 5.00
 Storage Coeff. (min)= 1.41 (ii) 4.27 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.33 0.23

PEAK FLOW (cms)= 0.13 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.139 (iii)
 RUNOFF VOLUME (mm)= 70.24 48.54 67.84
 TOTAL RAINFALL (mm)= 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20042) Area (ha)= 0.36
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.31 0.05
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.13 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.42 (ii) 4.72 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.13 0.01 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.33 1.33
RUNOFF VOLUME (mm)= 70.24 48.54 66.97
TOTAL RAINFALL (mm)= 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.94

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20052) Area (ha)= 0.29
ID= 1 DT= 5.0 min Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.23 0.06
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.13 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.70 (ii) 4.99 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.21

PEAK FLOW (cms)= 0.23 0.03 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.33 1.33
RUNOFF VOLUME (mm)= 70.24 48.54 66.97
TOTAL RAINFALL (mm)= 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.94

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20072) Area (ha)= 0.64
ID= 1 DT= 5.0 min Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.54 0.10
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 65.32 40.00
Mannings n = 0.13 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.69 (ii) 5.09 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20092) Area (ha)= 0.63
ID= 1 DT= 5.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.51 0.12
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 64.81 40.00
Mannings n = 0.13 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.64 (ii) 4.90 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.22

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.33 (ii) 5.31 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.09 0.01 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 70.24 48.54 66.97
TOTAL RAINFALL (mm)= 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.92

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20062) Area (ha)= 0.73
ID= 1 DT= 5.0 min Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.58 0.15
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 69.76 40.00
Mannings n = 0.13 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.76 (ii) 5.55 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.24 0.03 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 70.24 48.54 66.97
TOTAL RAINFALL (mm)= 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.92

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20061) Area (ha)= 0.65
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.49 0.09
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.18 40.00
Mannings n = 0.13 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr)= 148.15 102.23
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.64 (ii) 4.90 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.20 0.02 *TOTALS* (iii)
TIME TO PEAK (hrs)= 1.33 1.33 1.33
RUNOFF VOLUME (mm)= 70.24 48.54 66.97
TOTAL RAINFALL (mm)= 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.94

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20071) Area (ha)= 0.58
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.68 (ii) 5.37 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.32
 PEAK FLOW (cms) = 0.21 0.03 *TOTALS* 0.234 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42
 RUNOFF VOLUME (mm) = 70.24 48.54 66.11
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (200812)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
1	5.0 min	0.40	77.00	77.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.31	0.09
Dep. Storage (mm) = 1.00	1.00
Average Slope (%) = 1.00	2.00
Length (m) = 51.64	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.47 (ii) 5.33 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.33
 PEAK FLOW (cms) = 0.13 0.02 *TOTALS* 0.145 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42
 RUNOFF VOLUME (mm) = 70.24 48.54 65.24
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (200812)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
1	5.0 min	0.40	77.00	77.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.31	0.09
Dep. Storage (mm) = 1.00	1.00
Average Slope (%) = 1.00	2.00
Length (m) = 51.64	40.00
Mannings n = 0.013	0.290

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (200822)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
1	5.0 min	0.39	59.00	59.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.23	0.16
Dep. Storage (mm) = 1.00	1.00
Average Slope (%) = 1.00	2.00
Length (m) = 50.99	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.46 (ii) 9.10 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.33
 PEAK FLOW (cms) = 0.09 0.03 *TOTALS* 0.120 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42
 RUNOFF VOLUME (mm) = 70.24 48.54 61.32
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (200812)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
1	5.0 min	0.38	70.00	70.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.27	0.11
Dep. Storage (mm) = 1.00	1.00
Average Slope (%) = 1.00	2.00
Length (m) = 50.33	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.45 (ii) 6.14 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.33
 PEAK FLOW (cms) = 0.11 0.02 *TOTALS* 0.131 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42
 RUNOFF VOLUME (mm) = 70.24 48.54 63.72
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (200811)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
1	5.0 min	0.07	84.00	84.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.01	0.01
Dep. Storage (mm) = 1.00	1.00
Average Slope (%) = 1.00	2.00
Length (m) = 21.60	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (200821)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
1	5.0 min	0.07	83.00	83.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.06	0.01
Dep. Storage (mm) = 1.00	1.00
Average Slope (%) = 1.00	2.00
Length (m) = 21.60	40.00
Mannings n = 0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 0.87 (ii) 4.37 (iii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.34
 PEAK FLOW (cms) = 0.02 0.00 *TOTALS* 0.027 (iii)
 TIME TO PEAK (hrs) = 1.33 1.33
 RUNOFF VOLUME (mm) = 70.24 48.54 66.52
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (200811):	0.07	0.027	1.33	66.74
+ ID2 = 2 (200821):	0.38	0.131	1.33	63.72
-----	-----	-----	-----	-----
ID = 3 (0110):	0.45	0.159	1.33	64.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (200822)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
1	5.0 min	0.47	74.00	74.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.25	0.10
Dep. Storage (mm) = 1.00	1.00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.54 (ii) 5.88 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.33 0.15

TOTALS
 PEAK FLOW (cms) = 0.14 0.03 0.167 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42 1.33
 RUNOFF VOLUME (mm) = 70.24 48.54 64.59
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20101)	Area	(ha) =	0.25	Dir. Conn. (%) =	82.00
ID=	1 DT=	5.0 min				
Surface Area	(ha)			0.20		
Dep. Storage	(mm)			1.00		
Average Slope	(%)			1.00		
Length	(m)			40.82		40.00
Mannings n				0.013		0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.85 (ii) 6.82 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.32 0.14

TOTALS
 PEAK FLOW (cms) = 0.24 0.06 0.292 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42 1.33
 RUNOFF VOLUME (mm) = 70.24 48.54 63.07
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20112)	Area	(ha) =	0.90	Dir. Conn. (%) =	80.00
ID=	1 DT=	5.0 min				
Surface Area	(ha)			0.72		0.18
Dep. Storage	(mm)			1.00		1.00
Average Slope	(%)			1.00		2.00
Length	(m)			77.46		40.00
Mannings n				0.013		0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.87 (ii) 5.66 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.32 0.15

TOTALS
 PEAK FLOW (cms) = 0.30 0.04 0.331 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42 1.33
 RUNOFF VOLUME (mm) = 70.24 48.54 65.89
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20122)	Area	(ha) =	1.11	Dir. Conn. (%) =	81.00
ID=	1 DT=	5.0 min				
Surface Area	(ha)			0.90		0.21
Dep. Storage	(mm)			1.00		1.00
Average Slope	(%)			1.00		2.00
Length	(m)			86.02		40.00
Mannings n				0.013		0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.99 (ii) 5.69 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.31 0.15

TOTALS
 PEAK FLOW (cms) = 0.37 0.05 0.410 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42 1.33
 RUNOFF VOLUME (mm) = 70.24 48.54 66.11
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(0121)	AREA	(ha) =	0.25	QPEAK	(cms)	TPEAK	(hrs)	R.V.	(mm)
1 +	2 =	3								
ID1 =	1 (20101):	0.25		0.096	1.33		66.32			
+ ID2 =	2 (20102):	0.47		0.167	1.33		64.59			
ID =	3 (0121):	0.72		0.263	1.33		65.19			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (20111)	Area	(ha) =	0.87	Dir. Conn. (%) =	67.00
ID=	1 DT=	5.0 min				
Surface Area	(ha)			1.00		1.00
Dep. Storage	(mm)			1.00		2.00
Average Slope	(%)			76.16		40.00
Length	(m)			0.013		0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.85 (ii) 6.82 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.32 0.14

TOTALS
 PEAK FLOW (cms) = 0.24 0.06 0.292 (iii)
 TIME TO PEAK (hrs) = 1.33 1.42 1.33
 RUNOFF VOLUME (mm) = 70.24 48.54 63.07
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (20112)	Area	(ha) =	0.90	Dir. Conn. (%) =	80.00
ID=	1 DT=	5.0 min				
Surface Area	(ha)			0.72		0.18
Dep. Storage	(mm)			1.00		1.00
Average Slope	(%)			1.00		2.00
Length	(m)			77.46		40.00
Mannings n				0.013		0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.22	1.083	36.37	2.083	11.74	3.08	6.15
0.167	5.22	1.167	36.37	2.167	11.74	3.17	6.15
0.250	5.94	1.250	148.15	2.250	10.09	3.25	5.74
0.333	5.94	1.333	148.15	2.333	10.09	3.33	5.74
0.417	6.93	1.417	47.06	2.417	8.89	3.42	5.38
0.500	6.93	1.500	47.06	2.500	8.89	3.50	5.38
0.583	8.42	1.583	25.72	2.583	7.96	3.58	5.08
0.667	8.42	1.667	25.72	2.667	7.96	3.67	5.08
0.750	10.91	1.750	18.11	2.750	7.24	3.75	4.80
0.833	10.91	1.833	18.11	2.833	7.24	3.83	4.80
0.917	16.13	1.917	14.17	2.917	6.65	3.92	4.57
1.000	16.13	2.000	14.17	3.000	6.65	4.00	4.57

Max. Eff. Inten. (mm/hr) = 148.15 over (min) = 5.00
 Storage Coeff. (min) = 1.54 (ii) 4.84 (ii)
 Unit Hyd. Tpeak (min) = 5.00 10.00
 Unit Hyd. peak (cms) = 0.33 0.22

TOTALS
 PEAK FLOW (cms) = 0.16 0.02 0.183 (iii)
 TIME TO PEAK (hrs) = 1.33 1.33 1.33
 RUNOFF VOLUME (mm) = 70.24 48.54 66.98
 TOTAL RAINFALL (mm) = 71.24 71.24 71.24
 RUNOFF COEFFICIENT = 0.99 0.68 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii)

0.917 16.13 | 1.917 14.17 | 2.917 6.65 | 3.92 4.57
1.000 16.13 | 2.000 14.17 | 3.000 6.65 | 4.00 4.57

Max. Eff. Inten. (mm/hr) = 148.15 102.23
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.54 (ii) 4.84 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.33 0.22
PEAK FLOW (cms) = 0.16 0.02 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.183 (iii)
RUNOFF VOLUME (mm) = 70.24 48.54 66.98
TOTAL RAINFALL (mm) = 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136)
1 + 2 = 3
ID1 = 1 (201021): AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID2 = 2 (201022): 0.47 0.183 1.33 66.98
ID = 3 (0136): 0.94 0.366 1.33 66.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (2005) Area (ha) = 0.28
ID = 1 DT = 5.0 min Total Imp (%) = 90.00 Dir. Conn. (%) = 90.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.25 0.03
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 43.20 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.22 1.083 36.37 2.083 11.74 3.08 6.15
0.167 5.22 1.167 36.37 2.167 11.74 3.17 6.15
0.250 5.94 1.250 48.54 2.250 10.09 3.25 5.74
0.333 5.94 1.333 48.54 2.333 10.09 3.33 5.74
0.417 6.93 1.417 47.06 2.417 8.89 3.42 5.38
0.500 6.93 1.500 47.06 2.500 8.89 3.50 5.38
0.583 8.42 1.583 25.72 2.583 7.96 3.58 5.08
0.667 8.42 1.667 25.72 2.667 7.96 3.67 5.08
0.750 10.91 1.750 18.11 2.750 7.24 3.75 4.80
0.833 10.91 1.833 18.11 2.833 7.24 3.83 4.80
0.917 16.13 1.917 14.17 2.917 6.65 3.92 4.57
1.000 16.13 2.000 14.17 3.000 6.65 4.00 4.57

Max. Eff. Inten. (mm/hr) = 148.15 102.23
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.32 (ii) 4.06 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.33 0.24
PEAK FLOW (cms) = 0.10 0.01 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.111 (iii)
RUNOFF VOLUME (mm) = 70.24 48.54 68.06
TOTAL RAINFALL (mm) = 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.917 16.13 | 1.917 14.17 | 2.917 6.65 | 3.92 4.57
1.000 16.13 | 2.000 14.17 | 3.000 6.65 | 4.00 4.57

Max. Eff. Inten. (mm/hr) = 148.15 102.23
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.32 (ii) 4.06 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.34 0.25
PEAK FLOW (cms) = 0.04 0.00 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.040 (iii)
RUNOFF VOLUME (mm) = 70.24 48.54 67.94
TOTAL RAINFALL (mm) = 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200121) Area (ha) = 0.11
ID = 1 DT = 5.0 min Total Imp (%) = 87.00 Dir. Conn. (%) = 87.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.10 0.01
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 27.08 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.22 1.083 36.37 2.083 11.74 3.08 6.15
0.167 5.22 1.167 36.37 2.167 11.74 3.17 6.15
0.250 5.94 1.250 48.54 2.250 10.09 3.25 5.74
0.333 5.94 1.333 48.54 2.333 10.09 3.33 5.74
0.417 6.93 1.417 47.06 2.417 8.89 3.42 5.38
0.500 6.93 1.500 47.06 2.500 8.89 3.50 5.38
0.583 8.42 1.583 25.72 2.583 7.96 3.58 5.08
0.667 8.42 1.667 25.72 2.667 7.96 3.67 5.08
0.750 10.91 1.750 18.11 2.750 7.24 3.75 4.80
0.833 10.91 1.833 18.11 2.833 7.24 3.83 4.80
0.917 16.13 1.917 14.17 2.917 6.65 3.92 4.57
1.000 16.13 2.000 14.17 3.000 6.65 4.00 4.57

Max. Eff. Inten. (mm/hr) = 148.15 102.23
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.00 (ii) 4.08 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.34 0.24
PEAK FLOW (cms) = 0.04 0.00 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.043 (iii)
RUNOFF VOLUME (mm) = 70.24 48.54 67.94
TOTAL RAINFALL (mm) = 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142)
1 + 2 = 3
ID1 = 1 (200111): AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID2 = 2 (200121): 0.11 0.043 1.33 67.40
ID = 3 (0142): 0.11 0.043 1.33 67.40

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20121) Area (ha) = 1.11
ID = 1 DT = 5.0 min Total Imp (%) = 56.00 Dir. Conn. (%) = 56.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.62 0.49
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 86.02 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.22 1.083 36.37 2.083 11.74 3.08 6.15
0.167 5.22 1.167 36.37 2.167 11.74 3.17 6.15
0.250 5.94 1.250 48.54 2.250 10.09 3.25 5.74
0.333 5.94 1.333 48.54 2.333 10.09 3.33 5.74
0.417 6.93 1.417 47.06 2.417 8.89 3.42 5.38
0.500 6.93 1.500 47.06 2.500 8.89 3.50 5.38
0.583 8.42 1.583 25.72 2.583 7.96 3.58 5.08
0.667 8.42 1.667 25.72 2.667 7.96 3.67 5.08
0.750 10.91 1.750 18.11 2.750 7.24 3.75 4.80
0.833 10.91 1.833 18.11 2.833 7.24 3.83 4.80
0.917 16.13 1.917 14.17 2.917 6.65 3.92 4.57
1.000 16.13 2.000 14.17 3.000 6.65 4.00 4.57

Max. Eff. Inten. (mm/hr) = 148.15 102.23
over (min) = 1.99 (ii) 9.64 (ii)
Storage Coeff. (min) = 5.00 10.00
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.31 0.11
PEAK FLOW (cms) = 0.25 0.09 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.42 0.330 (iii)
RUNOFF VOLUME (mm) = 70.24 48.54 60.69
TOTAL RAINFALL (mm) = 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200111) Area (ha) = 0.10
ID = 1 DT = 5.0 min Total Imp (%) = 89.50 Dir. Conn. (%) = 89.50

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.01 0.01
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 25.82 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.22 1.083 36.37 2.083 11.74 3.08 6.15
0.167 5.22 1.167 36.37 2.167 11.74 3.17 6.15
0.250 5.94 1.250 48.54 2.250 10.09 3.25 5.74
0.333 5.94 1.333 48.54 2.333 10.09 3.33 5.74
0.417 6.93 1.417 47.06 2.417 8.89 3.42 5.38
0.500 6.93 1.500 47.06 2.500 8.89 3.50 5.38
0.583 8.42 1.583 25.72 2.583 7.96 3.58 5.08
0.667 8.42 1.667 25.72 2.667 7.96 3.67 5.08
0.750 10.91 1.750 18.11 2.750 7.24 3.75 4.80
0.833 10.91 1.833 18.11 2.833 7.24 3.83 4.80

ID = 3 (0142): 0.21 0.083 1.33 67.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (200112) Area (ha) = 0.10
ID = 1 DT = 5.0 min Total Imp (%) = 79.00 Dir. Conn. (%) = 79.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.08 0.02
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 25.69 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.22 1.083 36.37 2.083 11.74 3.08 6.15
0.167 5.22 1.167 36.37 2.167 11.74 3.17 6.15
0.250 5.94 1.250 48.54 2.250 10.09 3.25 5.74
0.333 5.94 1.333 48.54 2.333 10.09 3.33 5.74
0.417 6.93 1.417 47.06 2.417 8.89 3.42 5.38
0.500 6.93 1.500 47.06 2.500 8.89 3.50 5.38
0.583 8.42 1.583 25.72 2.583 7.96 3.58 5.08
0.667 8.42 1.667 25.72 2.667 7.96 3.67 5.08
0.750 10.91 1.750 18.11 2.750 7.24 3.75 4.80
0.833 10.91 1.833 18.11 2.833 7.24 3.83 4.80
0.917 16.13 1.917 14.17 2.917 6.65 3.92 4.57
1.000 16.13 2.000 14.17 3.000 6.65 4.00 4.57

Max. Eff. Inten. (mm/hr) = 148.15 102.23
over (min) = 5.00 5.00
Storage Coeff. (min) = 0.97 (ii) 4.85 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.34 0.22
PEAK FLOW (cms) = 0.03 0.01 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.038 (iii)
RUNOFF VOLUME (mm) = 70.24 48.54 65.64
TOTAL RAINFALL (mm) = 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200122) Area (ha) = 0.13
ID = 1 DT = 5.0 min Total Imp (%) = 86.00 Dir. Conn. (%) = 86.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.11 0.02
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 29.44 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.22 1.083 36.37 2.083 11.74 3.08 6.15
0.167 5.22 1.167 36.37 2.167 11.74 3.17 6.15
0.250 5.94 1.250 48.54 2.250 10.09 3.25 5.74
0.333 5.94 1.333 48.54 2.333 10.09 3.33 5.74
0.417 6.93 1.417 47.06 2.417 8.89 3.42 5.38
0.500 6.93 1.500 47.06 2.500 8.89 3.50 5.38
0.583 8.42 1.583 25.72 2.583 7.96 3.58 5.08
0.667 8.42 1.667 25.72 2.667 7.96 3.67 5.08
0.750 10.91 1.750 18.11 2.750 7.24 3.75 4.80
0.833 10.91 1.833 18.11 2.833 7.24 3.83 4.80

0.917 16.13 | 1.917 14.17 | 2.917 6.65 | 3.92 4.57
1.000 16.13 | 2.000 14.17 | 3.000 6.65 | 4.00 4.57

Max. Eff. Inten. (mm/hr) = 148.15 102.23
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.05 (ii) 4.24 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.34 0.24
PEAK FLOW (cms) = 0.05 0.00 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.051 (iii)
RUNOFF VOLUME (mm) = 70.24 48.54 65.64
TOTAL RAINFALL (mm) = 71.24 71.24 71.24
RUNOFF COEFFICIENT = 0.99 0.68 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)
1 + 2 = 3
ID1 = 1 (200112): AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
0.10 0.238 1.33 65.64
+ ID2 = 2 (200122): 0.13 0.051 1.33 67.17
ID = 3 (0145): 0.23 0.089 1.33 66.51
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U A A L
V I SSSSS UUUU A A LLLL
OOO TTTT TTTT H H Y Y M M O O TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****
Input filename: c:\Program Files (x86)\visualOTHMO 6.1\VO2\vo.in.dat
Output filename: c:\Users\caef070146\AppData\Local\Temp\50564c49-8c07-4086-8157-8729Fb3caba3\b83c9b70
Summary filename: c:\Users\caef070146\AppData\Local\Temp\50564c49-8c07-4086-8157-8729Fb3caba3\b83c9b70

DATE: 05-18-2022 TIME: 12:44:44
USER:
COMMENTS:
***** SIMULATION : Run 05 *****

READ STORM Filename: c:\Users\caef070146\AppData\Local\Temp\11403a02-80e0-48b3-97a8-b203216ba841\05b28c32
Ptotal = 79.45 mm Comments: 50-year, 4 hr-Chicago, City of Barrie

TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
0.500 7.85 | 1.500 51.92 | 2.500 10.03 | 3.50 6.11
0.583 9.50 | 1.583 28.58 | 2.583 9.00 | 3.58 5.77
0.667 9.50 | 1.667 28.58 | 2.667 9.00 | 3.67 5.77
0.750 12.27 | 1.750 20.23 | 2.750 8.19 | 3.75 5.46
0.833 12.27 | 1.833 20.23 | 2.833 8.19 | 3.83 5.46
0.917 18.04 | 1.917 15.88 | 2.917 7.53 | 3.92 5.19
1.000 18.04 | 2.000 15.88 | 3.000 7.53 | 4.00 5.19

Max. Eff. Inten. (mm/hr) = 164.22 118.16
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.67 (ii) 4.41 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.32 0.23
PEAK FLOW (cms) = 0.28 0.02 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.308 (iii)
RUNOFF VOLUME (mm) = 78.45 56.03 75.98
TOTAL RAINFALL (mm) = 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20031) Area (ha) = 0.43
ID = 1 DT = 5.0 min Total Imp (%) = 90.00 Dir. Conn. (%) = 90.00
Surface Area (ha) = 0.39 IMPERVIOUS 0.04 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 53.54 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
0.083 5.93 | 1.083 40.22 | 2.083 13.20 | 3.08 6.98
0.167 5.93 | 1.167 40.22 | 2.167 13.20 | 3.17 6.98
0.250 6.74 | 1.250 164.22 | 2.250 11.37 | 3.25 6.51
0.333 6.74 | 1.333 164.22 | 2.333 11.37 | 3.33 6.51
0.417 7.85 | 1.417 51.92 | 2.417 10.03 | 3.42 6.11
0.500 7.85 | 1.500 51.92 | 2.500 10.03 | 3.50 6.11
0.583 9.50 | 1.583 28.58 | 2.583 9.00 | 3.58 5.77
0.667 9.50 | 1.667 28.58 | 2.667 9.00 | 3.67 5.77
0.750 12.27 | 1.750 20.23 | 2.750 8.19 | 3.75 5.46
0.833 12.27 | 1.833 20.23 | 2.833 8.19 | 3.83 5.46
0.917 18.04 | 1.917 15.88 | 2.917 7.53 | 3.92 5.19
1.000 18.04 | 2.000 15.88 | 3.000 7.53 | 4.00 5.19

Max. Eff. Inten. (mm/hr) = 164.22 118.16
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.44 (ii) 4.07 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.33 0.24
PEAK FLOW (cms) = 0.18 0.01 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.190 (iii)
RUNOFF VOLUME (mm) = 78.45 56.03 76.21
TOTAL RAINFALL (mm) = 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20032) Area (ha) = 0.47

0.17 5.93 | 1.17 40.22 | 2.17 13.20 | 3.17 6.98
0.33 6.74 | 1.33 164.22 | 2.33 11.37 | 3.33 6.51
0.50 7.85 | 1.50 51.92 | 2.50 10.03 | 3.50 6.11
0.67 9.50 | 1.67 28.58 | 2.67 9.00 | 3.67 5.77
0.83 12.27 | 1.83 20.23 | 2.83 8.19 | 3.83 5.46
1.00 18.04 | 2.00 15.88 | 3.00 7.53 | 4.00 5.19

CALIB STANDBYD (20022) Area (ha) = 0.80
ID = 1 DT = 5.0 min Total Imp (%) = 85.00 Dir. Conn. (%) = 85.00
Surface Area (ha) = 0.68 IMPERVIOUS 0.12 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 73.03 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
0.083 5.93 | 1.083 40.22 | 2.083 13.20 | 3.08 6.98
0.167 5.93 | 1.167 40.22 | 2.167 13.20 | 3.17 6.98
0.250 6.74 | 1.250 164.22 | 2.250 11.37 | 3.25 6.51
0.333 6.74 | 1.333 164.22 | 2.333 11.37 | 3.33 6.51
0.417 7.85 | 1.417 51.92 | 2.417 10.03 | 3.42 6.11
0.500 7.85 | 1.500 51.92 | 2.500 10.03 | 3.50 6.11
0.583 9.50 | 1.583 28.58 | 2.583 9.00 | 3.58 5.77
0.667 9.50 | 1.667 28.58 | 2.667 9.00 | 3.67 5.77
0.750 12.27 | 1.750 20.23 | 2.750 8.19 | 3.75 5.46
0.833 12.27 | 1.833 20.23 | 2.833 8.19 | 3.83 5.46
0.917 18.04 | 1.917 15.88 | 2.917 7.53 | 3.92 5.19
1.000 18.04 | 2.000 15.88 | 3.000 7.53 | 4.00 5.19

Max. Eff. Inten. (mm/hr) = 164.22 118.16
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.73 (ii) 4.90 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.32 0.22
PEAK FLOW (cms) = 0.31 0.04 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.346 (iii)
RUNOFF VOLUME (mm) = 78.45 56.03 75.98
TOTAL RAINFALL (mm) = 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20021) Area (ha) = 0.70
ID = 1 DT = 5.0 min Total Imp (%) = 89.00 Dir. Conn. (%) = 89.00
Surface Area (ha) = 0.62 IMPERVIOUS 0.08 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 68.31 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
0.083 5.93 | 1.083 40.22 | 2.083 13.20 | 3.08 6.98
0.167 5.93 | 1.167 40.22 | 2.167 13.20 | 3.17 6.98
0.250 6.74 | 1.250 164.22 | 2.250 11.37 | 3.25 6.51
0.333 6.74 | 1.333 164.22 | 2.333 11.37 | 3.33 6.51
0.417 7.85 | 1.417 51.92 | 2.417 10.03 | 3.42 6.11
0.500 7.85 | 1.500 51.92 | 2.500 10.03 | 3.50 6.11
0.583 9.50 | 1.583 28.58 | 2.583 9.00 | 3.58 5.77
0.667 9.50 | 1.667 28.58 | 2.667 9.00 | 3.67 5.77
0.750 12.27 | 1.750 20.23 | 2.750 8.19 | 3.75 5.46
0.833 12.27 | 1.833 20.23 | 2.833 8.19 | 3.83 5.46
0.917 18.04 | 1.917 15.88 | 2.917 7.53 | 3.92 5.19
1.000 18.04 | 2.000 15.88 | 3.000 7.53 | 4.00 5.19

ID = 1 DT = 5.0 min | Total Imp (%) = 87.00 Dir. Conn. (%) = 87.00
Surface Area (ha) = 0.41 IMPERVIOUS 0.06 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 55.98 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
0.083 5.93 | 1.083 40.22 | 2.083 13.20 | 3.08 6.98
0.167 5.93 | 1.167 40.22 | 2.167 13.20 | 3.17 6.98
0.250 6.74 | 1.250 164.22 | 2.250 11.37 | 3.25 6.51
0.333 6.74 | 1.333 164.22 | 2.333 11.37 | 3.33 6.51
0.417 7.85 | 1.417 51.92 | 2.417 10.03 | 3.42 6.11
0.500 7.85 | 1.500 51.92 | 2.500 10.03 | 3.50 6.11
0.583 9.50 | 1.583 28.58 | 2.583 9.00 | 3.58 5.77
0.667 9.50 | 1.667 28.58 | 2.667 9.00 | 3.67 5.77
0.750 12.27 | 1.750 20.23 | 2.750 8.19 | 3.75 5.46
0.833 12.27 | 1.833 20.23 | 2.833 8.19 | 3.83 5.46
0.917 18.04 | 1.917 15.88 | 2.917 7.53 | 3.92 5.19
1.000 18.04 | 2.000 15.88 | 3.000 7.53 | 4.00 5.19

Max. Eff. Inten. (mm/hr) = 164.22 118.16
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.48 (ii) 4.40 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.33 0.23
PEAK FLOW (cms) = 0.19 0.02 *TOTALS*
TIME TO PEAK (hrs) = 1.33 1.33 0.205 (iii)
RUNOFF VOLUME (mm) = 78.45 56.03 75.53
TOTAL RAINFALL (mm) = 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20041) Area (ha) = 0.35
ID = 1 DT = 5.0 min Total Imp (%) = 89.00 Dir. Conn. (%) = 89.00
Surface Area (ha) = 0.31 IMPERVIOUS 0.04 PERVIOUS (i)
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 48.30 40.00
Mannings n = 0.013 0.290
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
0.083 5.93 | 1.083 40.22 | 2.083 13.20 | 3.08 6.98
0.167 5.93 | 1.167 40.22 | 2.167 13.20 | 3.17 6.98
0.250 6.74 | 1.250 164.22 | 2.250 11.37 | 3.25 6.51
0.333 6.74 | 1.333 164.22 | 2.333 11.37 | 3.33 6.51
0.417 7.85 | 1.417 51.92 | 2.417 10.03 | 3.42 6.11
0.500 7.85 | 1.500 51.92 | 2.500 10.03 | 3.50 6.11
0.583 9.50 | 1.583 28.58 | 2.583 9.00 | 3.58 5.77
0.667 9.50 | 1.667 28.58 | 2.667 9.00 | 3.67 5.77
0.750 12.27 | 1.750 20.23 | 2.750 8.19 | 3.75 5.46
0.833 12.27 | 1.833 20.23 | 2.833 8.19 | 3.83 5.46
0.917 18.04 | 1.917 15.88 | 2.917 7.53 | 3.92 5.19
1.000 18.04 | 2.000 15.88 | 3.000 7.53 | 4.00 5.19

Max. Eff. Inten. (mm/hr) = 164.22 118.16
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.35 (ii) 4.10 (ii)
Unit Hyd. Tpeak (min) = 5.00 5.00

Unit Hyd. peak (cms)= 0.33 0.24
PEAK FLOW (cms)= 0.14 0.01 0.154 (iii)
TIME TO PEAK (hrs)= 1.33 1.33 1.33
RUNOFF VOLUME (mm)= 78.45 56.03 75.98
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20042) Area (ha)= 0.36
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.31 0.05
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max.Eff.Inten.(mm/hr)= 164.22 118.16
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.37 (ii) 4.53 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.23

PEAK FLOW (cms)= 0.14 0.02 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.156 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 75.08
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20052) Area (ha)= 0.29
ID= 1 DT= 5.0 min Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.23 0.06
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 43.97 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max.Eff.Inten.(mm/hr)= 164.22 118.16
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.63 (ii) 4.78 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.25 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.281 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 75.09
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20061) Area (ha)= 0.65
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.55 0.10
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 65.83 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max.Eff.Inten.(mm/hr)= 164.22 118.16
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.63 (ii) 4.78 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.25 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.281 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 75.09
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20072) Area (ha)= 0.64
ID= 1 DT= 5.0 min Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.54 0.10
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 65.32 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46

Max.Eff.Inten.(mm/hr)= 164.22 118.16
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.58 (ii) 4.74 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.22 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.251 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 75.09
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max.Eff.Inten.(mm/hr)= 164.22 118.16
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.28 (ii) 5.09 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.10 0.02 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.119 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 73.51
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20062) Area (ha)= 0.73
ID= 1 DT= 5.0 min Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.58 0.15
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 69.76 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max.Eff.Inten.(mm/hr)= 164.22 118.16
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.69 (ii) 5.32 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.27 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.301 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 74.96
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max.Eff.Inten.(mm/hr)= 164.22 118.16
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.62 (ii) 4.88 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.24 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.276 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 74.96
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20071) Area (ha)= 0.58
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.49 0.09
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.18 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333							

Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 64.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98		
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98		
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51		
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51		
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11		
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11		
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77		
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77		
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46		
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46		
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19		
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19		

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.61 (ii) 5.16 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16
PEAK FLOW (cms)= 0.23 0.03 0.261 (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 78.45 56.03 74.19
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20091)
ID= 1 DT= 5.0 min
Area (ha)= 0.40
Total Imp(%)= 77.00 Dir. Conn.(%)= 77.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 51.64 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98		
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98		
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51		
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51		
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11		
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11		
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77		
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77		
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46		
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46		
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19		
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19		

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.41 (ii) 5.31 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16
PEAK FLOW (cms)= 0.14 0.02 0.162 (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20081)
ID= 1 DT= 5.0 min
Area (ha)= 0.40
Total Imp(%)= 77.00 Dir. Conn.(%)= 77.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 51.64 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98		
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98		
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51		
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51		
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11		
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11		
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77		
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77		
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46		
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46		
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19		
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19		

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 0.84 (ii) 4.09 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.34 0.24
PEAK FLOW (cms)= 0.03 0.00 0.030 (iii)
TIME TO PEAK (hrs)= 1.33 1.33 1.33
RUNOFF VOLUME (mm)= 78.45 56.03 74.19
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20082)
ID= 1 DT= 5.0 min
Area (ha)= 0.39
Total Imp(%)= 59.00 Dir. Conn.(%)= 59.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98		
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98		
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51		
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51		
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11		
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11		
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77		
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77		
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46		
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46		
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19		
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19		

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 0.80 (ii) 8.61 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.12
PEAK FLOW (cms)= 0.10 0.04 0.136 (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RUNOFF VOLUME (mm)= 78.45 56.03 73.29
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20081)
ID= 1 DT= 5.0 min
Area (ha)= 0.38
Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 50.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98		
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98		
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51		
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51		
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11		
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11		
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77		
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77		
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46		
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46		
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19		
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19		

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.39 (ii) 5.90 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15
PEAK FLOW (cms)= 0.12 0.03 0.147 (iii)
TIME TO PEAK (hrs)= 1.33 1.42 1.33
RUNOFF VOLUME (mm)= 78.45 56.03 71.71
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20081)
ID= 1 DT= 5.0 min
Area (ha)= 0.07
Total Imp(%)= 84.00 Dir. Conn.(%)= 84.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 21.60 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98		
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98		
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51		
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51		
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11		
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11		
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77		
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77		
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46		
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46		
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19		
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19		

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 0.84 (ii) 4.19 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.34 0.24
PEAK FLOW (cms)= 0.03 0.00 0.030 (iii)
TIME TO PEAK (hrs)= 1.33 1.33 1.33
RUNOFF VOLUME (mm)= 78.45 56.03 74.61
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20082)
ID= 1 DT= 5.0 min
Area (ha)= 0.07
Total Imp(%)= 83.00 Dir. Conn.(%)= 83.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 21.60 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98		
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98		
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51		
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51		

Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22
0.167	5.93	1.167	40.22
0.250	6.74	1.250	164.22
0.333	6.74	1.333	164.22
0.417	7.85	1.417	51.92
0.500	7.85	1.500	51.92
0.583	9.50	1.583	28.58
0.667	9.50	1.667	28.58
0.750	12.27	1.750	20.23
0.833	12.27	1.833	20.23
0.917	18.04	1.917	15.88
1.000	18.04	2.000	15.88

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.48 (ii) 5.60 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15
PEAK FLOW (cms)= 0.16 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.187 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 72.61
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20101) Area (ha)= 0.25
ID= 1 DT= 5.0 min Total Imp(%)= 82.00 Dir. Conn.(%)= 82.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 40.52 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22
0.167	5.93	1.167	40.22
0.250	6.74	1.250	164.22
0.333	6.74	1.333	164.22
0.417	7.85	1.417	51.92
0.500	7.85	1.500	51.92
0.583	9.50	1.583	28.58
0.667	9.50	1.667	28.58
0.750	12.27	1.750	20.23
0.833	12.27	1.833	20.23
0.917	18.04	1.917	15.88
1.000	18.04	2.000	15.88

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.22 (ii) 4.67 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.22
PEAK FLOW (cms)= 0.09 0.01 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.107 (iii)

Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 77.46 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22
0.167	5.93	1.167	40.22
0.250	6.74	1.250	164.22
0.333	6.74	1.333	164.22
0.417	7.85	1.417	51.92
0.500	7.85	1.500	51.92
0.583	9.50	1.583	28.58
0.667	9.50	1.667	28.58
0.750	12.27	1.750	20.23
0.833	12.27	1.833	20.23
0.917	18.04	1.917	15.88
1.000	18.04	2.000	15.88

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.80 (ii) 5.43 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.16
PEAK FLOW (cms)= 0.33 0.05 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.370 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 73.97
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20122) Area (ha)= 1.11
ID= 1 DT= 5.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 86.02 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22
0.167	5.93	1.167	40.22
0.250	6.74	1.250	164.22
0.333	6.74	1.333	164.22
0.417	7.85	1.417	51.92
0.500	7.85	1.500	51.92
0.583	9.50	1.583	28.58
0.667	9.50	1.667	28.58
0.750	12.27	1.750	20.23
0.833	12.27	1.833	20.23
0.917	18.04	1.917	15.88
1.000	18.04	2.000	15.88

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.91 (ii) 5.46 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.16
PEAK FLOW (cms)= 0.41 0.05 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.458 (iii)

RUNOFF VOLUME (mm)= 78.45 56.03 74.40
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20101) Area (ha)= 0.25
ID= 1 DT= 5.0 min Total Imp(%)= 82.00 Dir. Conn.(%)= 82.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 76.16 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22
0.167	5.93	1.167	40.22
0.250	6.74	1.250	164.22
0.333	6.74	1.333	164.22
0.417	7.85	1.417	51.92
0.500	7.85	1.500	51.92
0.583	9.50	1.583	28.58
0.667	9.50	1.667	28.58
0.750	12.27	1.750	20.23
0.833	12.27	1.833	20.23
0.917	18.04	1.917	15.88
1.000	18.04	2.000	15.88

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.78 (ii) 6.53 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.14
PEAK FLOW (cms)= 0.27 0.07 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.328 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 71.05
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20112) Area (ha)= 0.90
ID= 1 DT= 5.0 min Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

RUNOFF VOLUME (mm)= 78.45 56.03 74.19
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20102) Area (ha)= 0.47
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22
0.167	5.93	1.167	40.22
0.250	6.74	1.250	164.22
0.333	6.74	1.333	164.22
0.417	7.85	1.417	51.92
0.500	7.85	1.500	51.92
0.583	9.50	1.583	28.58
0.667	9.50	1.667	28.58
0.750	12.27	1.750	20.23
0.833	12.27	1.833	20.23
0.917	18.04	1.917	15.88
1.000	18.04	2.000	15.88

Max. Eff. Inten. (mm/hr)= 164.22 118.16
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.48 (ii) 4.64 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.22
PEAK FLOW (cms)= 0.18 0.02 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.204 (iii)
RUNOFF VOLUME (mm)= 78.45 56.03 75.09
TOTAL RAINFALL (mm)= 79.45 79.45 79.45
RUNOFF COEFFICIENT = 0.99 0.71 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20102) Area (ha)= 0.47
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 55.98 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22
0.167	5.93	1.167	40.22
0.250	6.74	1.250	164.22
0.333	6.74	1.333	164.22
0.417	7.85	1.417	51.92
0.500	7.85	1.500	51.92
0.583	9.50	1.583	28.58
0.667	9.50	1.667	28.58
0.750	12.27	1.750	20.23
0.833	12.27	1.833	20.23
0.917	18.04	1.917	15.88
1.000	18.04	2.000	15.88

0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max. Eff. Inten. (mm/hr) = 164.22
over (min) = 5.00
Storage Coeff. (min) = 1.48 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.33

PEAK FLOW (cms) = 0.18
TIME TO PEAK (hrs) = 1.33
RUNOFF VOLUME (mm) = 78.45
TOTAL RAINFALL (mm) = 79.45
RUNOFF COEFFICIENT = 0.99

TOTALS
0.204 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136)				
1 + 2 = 3				
AREA	OPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (201021):	0.47	0.204	1.33	75.09
+ ID2= 2 (201022):	0.47	0.204	1.33	75.09

ID = 3 (0136):	0.94	0.408	1.33	75.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	= 0.28
STANDHYD (2005)	Total Imp (%)	= 90.00	Dir. Conn. (%) = 90.00
ID= 1 DT= 5.0 min			

Surface Area (ha) = 0.25
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 43.20
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max. Eff. Inten. (mm/hr) = 164.22
over (min) = 5.00
Storage Coeff. (min) = 1.27 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.33

PEAK FLOW (cms) = 0.11
TIME TO PEAK (hrs) = 1.33

TOTALS
0.124 (iii)
1.33

0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max. Eff. Inten. (mm/hr) = 164.22
over (min) = 5.00
Storage Coeff. (min) = 0.93 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.34

PEAK FLOW (cms) = 0.04
TIME TO PEAK (hrs) = 1.33
RUNOFF VOLUME (mm) = 78.45
TOTAL RAINFALL (mm) = 79.45
RUNOFF COEFFICIENT = 0.99

TOTALS
0.044 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	= 0.11
STANDHYD (200121)	Total Imp (%)	= 87.00	Dir. Conn. (%) = 87.00
ID= 1 DT= 5.0 min			

Surface Area (ha) = 0.10
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 27.08
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max. Eff. Inten. (mm/hr) = 164.22
over (min) = 5.00
Storage Coeff. (min) = 0.96 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.34

PEAK FLOW (cms) = 0.04
TIME TO PEAK (hrs) = 1.33
RUNOFF VOLUME (mm) = 78.45
TOTAL RAINFALL (mm) = 79.45
RUNOFF COEFFICIENT = 0.99

TOTALS
0.048 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RUNOFF VOLUME (mm)	= 78.45	56.03	76.21
TOTAL RAINFALL (mm)	= 79.45	79.45	79.45
RUNOFF COEFFICIENT	= 0.99	0.71	0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	= 1.11
STANDHYD (20121)	Total Imp (%)	= 56.00	Dir. Conn. (%) = 56.00
ID= 1 DT= 5.0 min			

Surface Area (ha) = 0.62
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 86.02
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max. Eff. Inten. (mm/hr) = 164.22
over (min) = 5.00
Storage Coeff. (min) = 1.91 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.28
TIME TO PEAK (hrs) = 1.33
RUNOFF VOLUME (mm) = 78.45
TOTAL RAINFALL (mm) = 79.45
RUNOFF COEFFICIENT = 0.99

TOTALS
0.373 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	= 0.10
STANDHYD (200111)	Total Imp (%)	= 89.50	Dir. Conn. (%) = 89.50
ID= 1 DT= 5.0 min			

Surface Area (ha) = 0.09
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 25.82
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98

Max. Eff. Inten. (mm/hr) = 164.22
over (min) = 5.00
Storage Coeff. (min) = 1.00
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.34

PEAK FLOW (cms) = 0.04
TIME TO PEAK (hrs) = 1.33
RUNOFF VOLUME (mm) = 78.45
TOTAL RAINFALL (mm) = 79.45
RUNOFF COEFFICIENT = 0.99

TOTALS
0.044 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	= 0.10
STANDHYD (200112)	Total Imp (%)	= 79.00	Dir. Conn. (%) = 79.00
ID= 1 DT= 5.0 min			

Surface Area (ha) = 0.08
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 25.69
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98
0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max. Eff. Inten. (mm/hr) = 164.22
over (min) = 5.00
Storage Coeff. (min) = 0.93 (ii)
Unit Hyd. Tpeak (min) = 5.00
Unit Hyd. peak (cms) = 0.34

PEAK FLOW (cms) = 0.04
TIME TO PEAK (hrs) = 1.33
RUNOFF VOLUME (mm) = 78.45
TOTAL RAINFALL (mm) = 79.45
RUNOFF COEFFICIENT = 0.99

TOTALS
0.044 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	= 0.13
STANDHYD (200122)	Total Imp (%)	= 86.00	Dir. Conn. (%) = 86.00
ID= 1 DT= 5.0 min			

Surface Area (ha) = 0.11
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 29.44
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.93	1.083	40.22	2.083	13.20	3.08	6.98

0.167	5.93	1.167	40.22	2.167	13.20	3.17	6.98
0.250	6.74	1.250	164.22	2.250	11.37	3.25	6.51
0.333	6.74	1.333	164.22	2.333	11.37	3.33	6.51
0.417	7.85	1.417	51.92	2.417	10.03	3.42	6.11
0.500	7.85	1.500	51.92	2.500	10.03	3.50	6.11
0.583	9.50	1.583	28.58	2.583	9.00	3.58	5.77
0.667	9.50	1.667	28.58	2.667	9.00	3.67	5.77
0.750	12.27	1.750	20.23	2.750	8.19	3.75	5.46
0.833	12.27	1.833	20.23	2.833	8.19	3.83	5.46
0.917	18.04	1.917	15.88	2.917	7.53	3.92	5.19
1.000	18.04	2.000	15.88	3.000	7.53	4.00	5.19

Max. Eff. Inten. (mm/hr) = 164.22
 over (min) = 5.00
 Storage Coeff. (min) = 1.01 (ii)
 Unit Hyd. Tpeak (min) = 5.00
 Unit Hyd. peak (cms) = 0.34
 PEAK FLOW (cms) = 0.05
 TIME TO PEAK (hrs) = 1.33
 RUNOFF VOLUME (mm) = 78.45
 TOTAL RAINFALL (mm) = 79.45
 RUNOFF COEFFICIENT = 0.99

TOTALS
 0.057 (iii)
 1.33
 75.29
 79.45
 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (200112):	0.10	0.042	1.33	73.70
+ ID2 = 2 (200122):	0.13	0.057	1.33	75.29
ID = 3 (0145):	0.23	0.099	1.33	74.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 6.1.2002)
 V V I SS U U A A L
 V V I SS U U A A A A L
 W I SSSS UUUU A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2020 Smart City Water Inc
 All rights reserved.

***** DETAILED OUTPUT *****

Input filename: c:\Program Files (x86)\Visual\OTTHYMO 6.1\VO2\voin.dat
 Output filename: c:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\68534fb6
 Summary filename: c:\Users\caef070146\AppData\Local\Civica\WH5\50564c49-8c07-4086-8157-8729fb3caba3\68534fb6

DATE: 05-18-2022 TIME: 12:44:44

USER:

COMMENTS:

** SIMULATION : Run 06 **

READ STORM
 Ptotal = 87.58 mm

Filename: c:\Users\caef070146\AppData\Local\Temp\11403a02-80e0-48b3-97a8-b203216ba841\c83eb39
 Comments: 100-year, 4 hr-Chicago, City of Barrie

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	6.41	1.17	45.22	2.17	14.50	3.17	7.56
0.33	7.29	1.33	180.15	2.33	12.44	3.33	7.04
0.50	8.52	1.50	58.54	2.50	10.94	3.50	6.60
0.67	10.36	1.67	31.96	2.67	9.80	3.67	6.22
0.83	13.45	1.83	22.45	2.83	8.90	3.83	5.89
1.00	19.96	2.00	17.52	3.00	8.16	4.00	5.59

CALIB
 STANDHYD (20022)
 ID= 1 DT= 5.0 min

Area (ha) = 0.80
 Total Imp (%) = 85.00 Dir. Conn. (%) = 85.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.68 0.12
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 73.03 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.67 (ii) 4.72 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.32 0.22
 PEAK FLOW (cms) = 0.34 0.04 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33
 RUNOFF VOLUME (mm) = 86.58 63.54 83.12
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20021)
 ID= 1 DT= 5.0 min

Area (ha) = 0.70
 Total Imp (%) = 89.00 Dir. Conn. (%) = 89.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.62 0.08
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 68.31 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.61 (ii) 4.25 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.32 0.24
 PEAK FLOW (cms) = 0.31 0.03 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33
 RUNOFF VOLUME (mm) = 86.58 63.54 84.04
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20031)
 ID= 1 DT= 5.0 min

Area (ha) = 0.43
 Total Imp (%) = 90.00 Dir. Conn. (%) = 90.00
 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.39 0.04
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 53.54 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.39 (ii) 3.92 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.33 0.24
 PEAK FLOW (cms) = 0.19 0.02 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33
 RUNOFF VOLUME (mm) = 86.58 63.54 84.27
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:

CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20032)
 ID= 1 DT= 5.0 min

Area (ha) = 0.47
 Total Imp (%) = 87.00 Dir. Conn. (%) = 87.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.41 0.06
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 55.98 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.43 (ii) 4.28 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.33 0.23 *TOTALS*
 PEAK FLOW (cms) = 0.20 0.02 0.226 (iii)
 TIME TO PEAK (hrs) = 1.33 1.33 1.33
 RUNOFF VOLUME (mm) = 86.58 63.54 83.57
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (20041)
 ID= 1 DT= 5.0 min

Area (ha) = 0.35
 Total Imp (%) = 89.00 Dir. Conn. (%) = 89.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.31 0.04
 Dep. Storage (mm) = 1.00 1.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 48.30 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22

0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)=	180.15	134.19
over (min)	5.00	5.00
Storage Coeff. (min)=	1.50 (ii)	5.95 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.33	0.24
PEAK FLOW (cms)=	0.16	0.01
TIME TO PEAK (hrs)=	1.33	1.33
RUNOFF VOLUME (mm)=	86.58	63.54
TOTAL RAINFALL (mm)=	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73

TOTALS	0.170 (iii)
PEAK FLOW (cms)=	0.16
TIME TO PEAK (hrs)=	1.33
RUNOFF VOLUME (mm)=	86.58
TOTAL RAINFALL (mm)=	87.58
RUNOFF COEFFICIENT =	0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20042)	Area (ha)= 0.36	Dir. Conn. (%) = 85.00
ID= 1 DT= 5.0 min	Total Imp (%) = 85.00	

Surface Area (ha)=	0.31	0.05
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	48.99	40.00
Mannings n =	0.013	0.290

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.31	0.05
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	48.99	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)=	180.15	134.19
over (min)	5.00	5.00
Storage Coeff. (min)=	1.32 (ii)	4.36 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.33	0.23
PEAK FLOW (cms)=	0.15	0.02
TIME TO PEAK (hrs)=	1.33	1.33
RUNOFF VOLUME (mm)=	86.58	63.54
TOTAL RAINFALL (mm)=	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73

TOTALS	0.172 (iii)
PEAK FLOW (cms)=	0.15
TIME TO PEAK (hrs)=	1.33
RUNOFF VOLUME (mm)=	86.58
TOTAL RAINFALL (mm)=	87.58
RUNOFF COEFFICIENT =	0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20052)	Area (ha)= 0.29	Dir. Conn. (%) = 78.00
ID= 1 DT= 5.0 min	Total Imp (%) = 78.00	

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.15	0.15
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	65.83	40.00
Mannings n =	0.013	0.290

TIME TO PEAK (hrs)=	1.33	1.42	1.33
RUNOFF VOLUME (mm)=	86.58	63.54	81.96
TOTAL RAINFALL (mm)=	87.58	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73	0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20061)	Area (ha)= 0.65	Dir. Conn. (%) = 85.00
ID= 1 DT= 5.0 min	Total Imp (%) = 85.00	

Surface Area (ha)=	0.15	0.15
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	65.83	40.00
Mannings n =	0.013	0.290

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.15	0.15
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	65.83	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)=	180.15	134.19
over (min)	5.00	5.00
Storage Coeff. (min)=	1.57 (ii)	4.62 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.33	0.22
PEAK FLOW (cms)=	0.28	0.03
TIME TO PEAK (hrs)=	1.33	1.33
RUNOFF VOLUME (mm)=	86.58	63.54
TOTAL RAINFALL (mm)=	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73

TOTALS	0.310 (iii)
PEAK FLOW (cms)=	0.28
TIME TO PEAK (hrs)=	1.33
RUNOFF VOLUME (mm)=	86.58
TOTAL RAINFALL (mm)=	87.58
RUNOFF COEFFICIENT =	0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20072)	Area (ha)= 0.64	Dir. Conn. (%) = 84.00
ID= 1 DT= 5.0 min	Total Imp (%) = 84.00	

Surface Area (ha)=	0.54	0.10
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	65.32	40.00
Mannings n =	0.013	0.290

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.54	0.10
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	65.32	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Surface Area (ha)=	0.23	0.06
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	43.97	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)=	180.15	134.19
over (min)	5.00	5.00
Storage Coeff. (min)=	1.23 (ii)	4.91 (ii)
Unit Hyd. Tpeak (min)=	5.00	5.00
Unit Hyd. peak (cms)=	0.33	0.22
PEAK FLOW (cms)=	0.11	0.02
TIME TO PEAK (hrs)=	1.33	1.33
RUNOFF VOLUME (mm)=	86.58	63.54
TOTAL RAINFALL (mm)=	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73

TOTALS	0.135 (iii)
PEAK FLOW (cms)=	0.11
TIME TO PEAK (hrs)=	1.33
RUNOFF VOLUME (mm)=	86.58
TOTAL RAINFALL (mm)=	87.58
RUNOFF COEFFICIENT =	0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (20062)	Area (ha)= 0.73	Dir. Conn. (%) = 80.00
ID= 1 DT= 5.0 min	Total Imp (%) = 80.00	

Surface Area (ha)=	0.58	0.15
Dep. Storage (mm)=	1.00	1.00
Average Slope (%)=	1.00	2.00
Length (m)=	69.76	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96				

CALIB
STANDHYD (20092)
ID= 1 DT= 5.0 min

Area (ha)= 0.63
Total Imp (%) = 81.00 Dir. Conn. (%) = 81.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.51 0.12
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 64.81 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56		
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56		
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04		
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04		
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60		
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60		
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22		
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22		
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89		
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89		
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59		
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59		

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.56 (ii) 4.97 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.26 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.296 (iii)
RUNOFF VOLUME (mm)= 86.58 63.54 82.19
TOTAL RAINFALL (mm)= 87.58 87.58 87.58
RUNOFF COEFFICIENT = 0.99 0.73 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^p = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20091)
ID= 1 DT= 5.0 min

Area (ha)= 0.40
Total Imp (%) = 77.00 Dir. Conn. (%) = 77.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.31 0.09
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 51.64 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56		
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56		
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04		
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04		
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60		
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60		
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22		
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22		
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89		
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89		
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59		
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59		

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.34 (ii) 5.68 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.13 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.163 (iii)
RUNOFF VOLUME (mm)= 86.58 63.54 79.65
TOTAL RAINFALL (mm)= 87.58 87.58 87.58
RUNOFF COEFFICIENT = 0.99 0.73 0.91

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.56 (ii) 5.12 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.15 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.179 (iii)
RUNOFF VOLUME (mm)= 86.58 63.54 81.27
TOTAL RAINFALL (mm)= 87.58 87.58 87.58
RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^p = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (200812)
ID= 1 DT= 5.0 min

Area (ha)= 0.38
Total Imp (%) = 70.00 Dir. Conn. (%) = 70.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.27 0.11
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 50.33 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56		
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56		
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04		
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04		
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60		
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60		
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22		
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22		
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89		
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89		
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59		
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59		

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.34 (ii) 5.68 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.13 0.03 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.163 (iii)
RUNOFF VOLUME (mm)= 86.58 63.54 79.65
TOTAL RAINFALL (mm)= 87.58 87.58 87.58
RUNOFF COEFFICIENT = 0.99 0.73 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^p = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (200811)
ID= 1 DT= 5.0 min

Area (ha)= 0.07
Total Imp (%) = 84.00 Dir. Conn. (%) = 84.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 1.00 1.00
Dep. Storage (mm)= 1.00 2.00
Average Slope (%) = 1.00 40.00
Length (m) = 21.60 40.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56		
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56		
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04		
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04		
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60		
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60		
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22		
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22		
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89		
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89		
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59		
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59		

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.35 (ii) 8.21 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.13

PEAK FLOW (cms)= 0.12 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.42 0.151 (iii)
RUNOFF VOLUME (mm)= 86.58 63.54 77.12
TOTAL RAINFALL (mm)= 87.58 87.58 87.58
RUNOFF COEFFICIENT = 0.99 0.73 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^p = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (200821)
ID= 1 DT= 5.0 min

Area (ha)= 0.07
Total Imp (%) = 83.00 Dir. Conn. (%) = 83.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.06 0.01
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 40.00
Length (m) = 21.60 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56		
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56		
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04		
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04		
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60		
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60		
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22		
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22		
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89		
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89		
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59		
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59		

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 5.00
Storage Coeff. (min)= 0.80 (ii) 4.04 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.24

PEAK FLOW (cms)= 0.03 0.00 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.33 0.033 (iii)
RUNOFF VOLUME (mm)= 86.58 63.54 82.63
TOTAL RAINFALL (mm)= 87.58 87.58 87.58
RUNOFF COEFFICIENT = 0.99 0.73 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^p = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110)
L + 2 = 3

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID= 1 (200811): 0.07 0.033 1.33 82.86
+ ID= 2 (200812): 0.38 0.163 1.33 79.65

ID = 3 (0110): 0.45 0.197 1.33 80.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (200822)
ID= 1 DT= 5.0 min

Area (ha)= 0.39
Total Imp (%) = 59.00 Dir. Conn. (%) = 59.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.23 0.16
Dep. Storage (mm)= 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 50.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56		
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56		
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04		
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04		
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60		
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60		
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22		
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22		
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89		
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89		
0.917	19.96	1.917	17.52	2.917	8.16	3.92			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (20102) Area (ha)= 0.47 Total Imp(%)= 74.00 Dir. Conn.(%)= 74.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.35 0.12 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 55.98 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals from 0.083 to 1.000 hours.

Max.Eff.Inten.(mm/hr)= 180.15 134.19 over (min)= 5.00 10.00 Storage Coeff.(min)= 1.43 (ii) 5.44 (iii) Unit Hyd. Tpeak (min)= 5.00 10.00 Unit Hyd. peak (cms)= 0.33 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20101) Area (ha)= 0.25 Total Imp(%)= 82.00 Dir. Conn.(%)= 82.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.20 0.05 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 40.82 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals from 0.083 to 1.000 hours.

Max.Eff.Inten.(mm/hr)= 180.15 134.19 over (min)= 5.00 10.00 Storage Coeff.(min)= 1.73 (ii) 5.23 (iii) Unit Hyd. Tpeak (min)= 5.00 10.00 Unit Hyd. peak (cms)= 0.32 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20112) Area (ha)= 0.90 Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.72 0.18 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 77.46 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals from 0.083 to 1.000 hours.

Max.Eff.Inten.(mm/hr)= 180.15 134.19 over (min)= 5.00 10.00 Storage Coeff.(min)= 1.73 (ii) 5.23 (iii) Unit Hyd. Tpeak (min)= 5.00 10.00 Unit Hyd. peak (cms)= 0.32 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20122) Area (ha)= 1.11 Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.90 0.21 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 86.02 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals from 0.083 to 1.000 hours.

Max.Eff.Inten.(mm/hr)= 180.15 134.19 over (min)= 5.00 5.00 Storage Coeff.(min)= 1.18 (ii) 4.50 (iii) Unit Hyd. Tpeak (min)= 5.00 5.00 Unit Hyd. peak (cms)= 0.33 0.23

PEAK FLOW (cms)= 0.10 0.02 *TOTALS* (iii) 0.118 TIME TO PEAK (hrs)= 1.33 1.33 1.33 RUNOFF VOLUME (mm)= 86.58 63.54 82.42 TOTAL RAINFALL (mm)= 87.58 87.58 87.58 RUNOFF COEFFICIENT = 0.99 0.73 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0121) table with columns: AREA, QPEAK, TPEAK, R.V. (mm). Rows show ID1=1, ID2=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (20111) Area (ha)= 0.87 Total Imp(%)= 67.00 Dir. Conn.(%)= 67.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.58 0.29 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 76.16 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals from 0.083 to 1.000 hours.

Max.Eff.Inten.(mm/hr)= 180.15 134.19 over (min)= 5.00 10.00 Storage Coeff.(min)= 1.71 (ii) 6.31 (iii) Unit Hyd. Tpeak (min)= 5.00 10.00 Unit Hyd. peak (cms)= 0.32 0.15

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Max.Eff.Inten.(mm/hr)= 180.15 134.19 over (min)= 5.00 10.00 Storage Coeff.(min)= 1.84 (ii) 5.26 (iii) Unit Hyd. Tpeak (min)= 5.00 10.00 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.45 0.06 *TOTALS* (iii) 0.506 TIME TO PEAK (hrs)= 1.33 1.42 1.33 RUNOFF VOLUME (mm)= 86.58 63.54 82.20 TOTAL RAINFALL (mm)= 87.58 87.58 87.58 RUNOFF COEFFICIENT = 0.99 0.73 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (201021) Area (ha)= 0.47 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.40 0.07 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 55.98 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals from 0.083 to 1.000 hours.

Max.Eff.Inten.(mm/hr)= 180.15 134.19 over (min)= 5.00 5.00 Storage Coeff.(min)= 1.43 (ii) 4.47 (iii) Unit Hyd. Tpeak (min)= 5.00 5.00 Unit Hyd. peak (cms)= 0.33 0.23

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (201022) Area (ha)= 0.47 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.40 0.07 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 55.98 40.00 Mannings n = 0.013 0.290

Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.43 (ii) 4.47 (ii)

Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.23
TOTALS
PEAK FLOW (cms)= 0.20 0.02 0.225 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (2005)
ID= 1 DT= 5.0 min
Area (ha)= 0.28
Total Imp(%)= 90.00 Dir. Conn.(%)= 90.00

Surface Area (ha)= 0.25 0.03
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 43.20 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 5.00
Storage Coeff. (min)= 0.30 (ii) 3.48 (ii)

Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.26
TOTALS
PEAK FLOW (cms)= 0.04 0.00 0.049 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (200121)
ID= 1 DT= 5.0 min
Area (ha)= 0.11
Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00

Surface Area (ha)= 0.10 0.01
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 27.08 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 5.00
Storage Coeff. (min)= 0.92 (ii) 3.77 (ii)

Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.25
TOTALS
PEAK FLOW (cms)= 0.05 0.01 0.053 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200122)
ID= 1 DT= 5.0 min
Area (ha)= 0.13
Total Imp(%)= 86.00 Dir. Conn.(%)= 86.00

Surface Area (ha)= 0.11 0.02
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 29.44 40.00

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 5.00
Storage Coeff. (min)= 1.22 (ii) 3.75 (ii)

Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.25
TOTALS
PEAK FLOW (cms)= 0.13 0.01 0.13 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (202121)
ID= 1 DT= 5.0 min
Area (ha)= 1.11
Total Imp(%)= 56.00 Dir. Conn.(%)= 56.00

Surface Area (ha)= 0.62 0.49
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 86.02 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.84 (ii) 8.70 (ii)

Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.12
TOTALS
PEAK FLOW (cms)= 0.31 0.12 0.417 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200111)
ID= 1 DT= 5.0 min
Area (ha)= 0.10
Total Imp(%)= 89.50 Dir. Conn.(%)= 89.50

Surface Area (ha)= 0.10 0.049
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 25.82 40.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (200112)
ID= 1 DT= 5.0 min
Area (ha)= 0.10
Total Imp(%)= 79.00 Dir. Conn.(%)= 79.00

Surface Area (ha)= 0.08 0.02
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 25.69 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

Max.Eff.Inten.(mm/hr)= 180.15 134.19
over (min)= 5.00 5.00
Storage Coeff. (min)= 0.89 (ii) 4.48 (ii)

Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.23
TOTALS
PEAK FLOW (cms)= 0.04 0.01 0.046 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (200122)
ID= 1 DT= 5.0 min
Area (ha)= 0.13
Total Imp(%)= 86.00 Dir. Conn.(%)= 86.00

Surface Area (ha)= 0.11 0.02
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 29.44 40.00

Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 0.97 (ii) 3.92 (ii)
 Unit Hyd. tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.34 0.24 *TOTALS*
 PEAK FLOW (cms) = 0.06 0.01 0.063 (iii)
 TIME TO PEAK (hrs) = 1.33 1.33 1.33
 RUNOFF VOLUME (mm) = 86.58 63.54 83.33
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

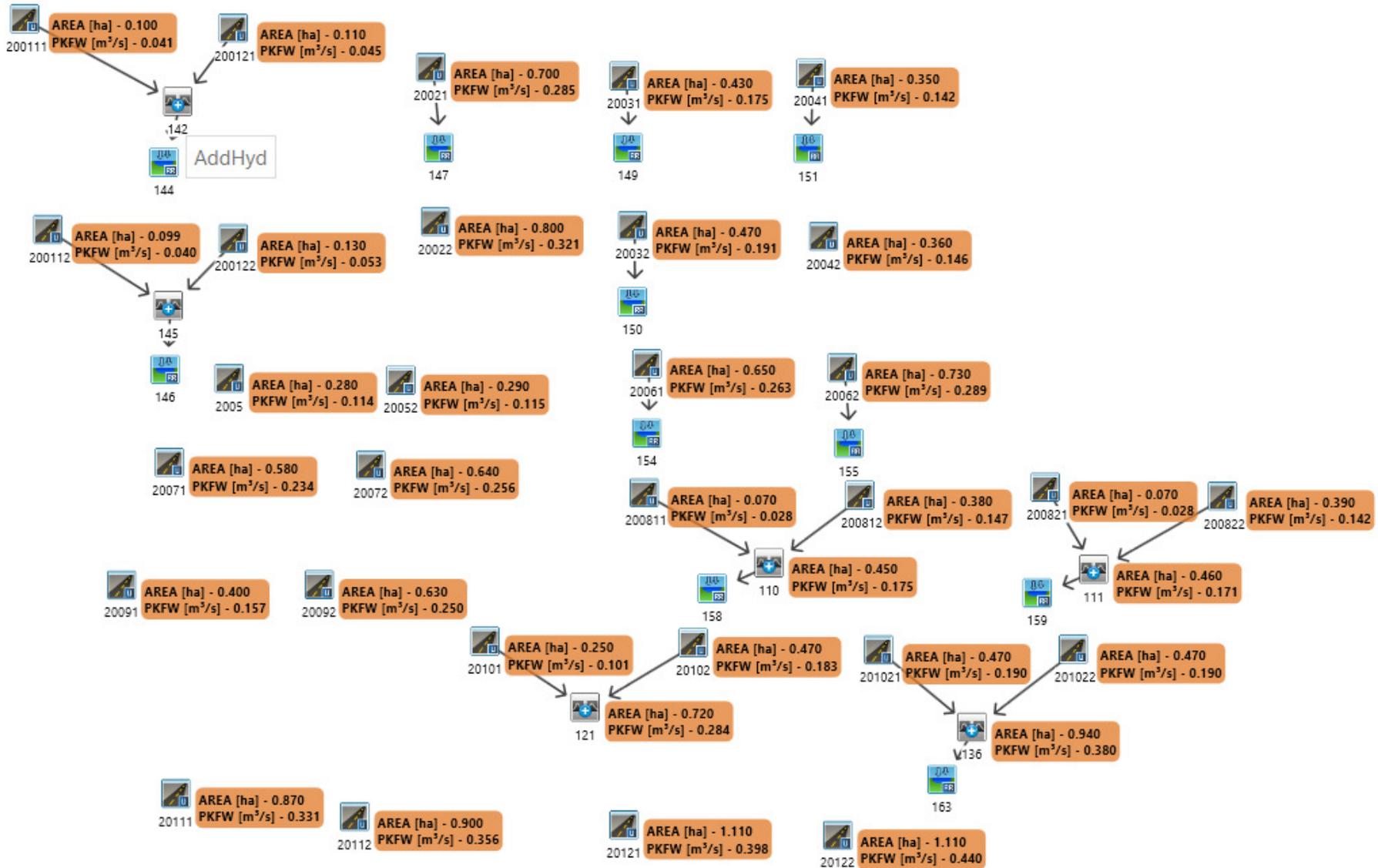
- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (200112):	0.10	0.046	1.33	81.73
+ ID2 = 2 (200122):	0.13	0.063	1.33	83.33
ID = 3 (0145):	0.23	0.109	1.33	82.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

100-Yr Post-Development Required Storage Volume Roadway Drainage Areas (12-hr SCSII)



V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A L
V V I SS U U A A L
W I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\visual OTTHMO 6.1\VO2\voim.dat
Output filename: C:\Users\caef070146\AppData\Local\Civica\VHS\30564c49-8c07-4086-8157-8729fb3caba3\ad5c9df4
Summary filename: C:\Users\caef070146\AppData\Local\Civica\VHS\30564c49-8c07-4086-8157-8729fb3caba3\ad5c9df4

DATE: 05-18-2022 TIME: 01:03:59

USER:

COMMENTS:

***** SIMULATION Run *****

READ STORM Filename: C:\Users\caef070146\AppData\Local\Temp\df3bea7-c83b-4af2-836d-f749a05f6013\13bf36c4
Ptotal=112.50 mm Comments: 100-yr, 12-hr SCS, City of Barrie

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.81	3.25	4.50	6.25	20.25	9.25	3.94
0.50	2.81	3.50	4.50	6.50	20.25	9.50	3.94
0.75	2.81	3.75	4.50	6.75	9.00	9.75	3.94
1.00	2.81	4.00	4.50	7.00	10.00	10.00	3.94
1.25	2.81	4.25	6.75	7.25	6.75	10.25	2.25
1.50	2.81	4.50	6.75	7.50	6.75	10.50	2.25
1.75	2.81	4.75	9.00	7.75	6.75	10.75	2.25
2.00	2.81	5.00	9.00	8.00	6.75	11.00	2.25
2.25	3.37	5.25	13.50	8.25	3.94	11.25	2.25
2.50	3.37	5.50	13.50	8.50	3.94	11.50	2.25
2.75	3.37	5.75	14.85	8.75	3.94	11.75	2.25
3.00	3.37	6.00	148.50	9.00	3.94	12.00	2.25

CALIB STANDBYD (20022) Area (ha)= 0.80
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.18 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 73.03 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94

1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 over (min)= 5.00
Storage Coeff. (min)= 1.42 (ii) 4.71 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.13 0.02 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 112.50 87.00 106.10
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20052) Area (ha)= 0.29
ID= 1 DT= 5.0 min Total Imp(%)= 78.00 Dir. Conn.(%)= 78.00

Surface Area (ha)= 0.23 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.97 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25

0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 over (min)= 5.00
Storage Coeff. (min)= 1.42 (ii) 4.71 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.16

PEAK FLOW (cms)= 0.28 0.04 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 107.82
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (20042) Area (ha)= 0.36
ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.31 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 1.00 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 48.99 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94

1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750							

2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	14.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50
over (min) = 5.00
Storage Coeff. (min)= 1.69 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.32

PEAK FLOW (cms)= 0.22
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 111.50
TOTAL RAINFALL (mm)= 112.50
RUNOFF COEFFICIENT = 0.99

TOTALS
0.256 (iii)
6.00
107.58
112.50
0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2007I)
ID= 1 DT= 5.0 min

Area (ha)= 0.58
Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

Surface Area (ha)= 0.49
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 62.18
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETEROGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25

PEAK FLOW (cms)= 0.21
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 111.50
TOTAL RAINFALL (mm)= 112.50
RUNOFF COEFFICIENT = 0.99

TOTALS
0.250 (iii)
6.00
106.84
112.50
0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2009I)
ID= 1 DT= 5.0 min

Area (ha)= 0.40
Total Imp(%)= 77.00 Dir. Conn.(%)= 77.00

Surface Area (ha)= 0.31
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 51.64
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETEROGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50
over (min) = 5.00
Storage Coeff. (min)= 1.47 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.33

PEAK FLOW (cms)= 0.13
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 111.50
TOTAL RAINFALL (mm)= 112.50
RUNOFF COEFFICIENT = 0.99

TOTALS
0.157 (iii)
6.00
105.86
112.50
0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25
-------	------	-------	--------	-------	------	-------	------

Max. Eff. Inten. (mm/hr)= 148.50
over (min) = 5.00
Storage Coeff. (min)= 1.64 (ii)
Unit Hyd. Tpeak (min)= 5.00
Unit Hyd. peak (cms)= 0.32

TOTALS
0.234 (iii)
6.00
107.82
112.50
0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2009I)
ID= 1 DT= 5.0 min

Area (ha)= 0.63
Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

Surface Area (ha)= 0.51
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 64.81
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HETEROGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917							

CALIB
STANDHYD (20101) Area (ha)= 0.25
ID= 1 DT= 5.0 min Total Imp (%) = 82.00 Dir. Conn. (%) = 82.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.20 0.05
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 40.82 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94				
0.167	2.81	4.50	6.167	20.25	9.17	3.94					
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94				
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94				
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94				
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94				
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94				
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94				
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94				
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94				
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94				
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94				
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25				
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25				
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25				
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25				
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25				
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25				
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25				
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25				
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25				
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25				
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25				
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25				
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25				
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25				
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25				
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25				
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25				
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25				
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25				
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25				
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25				
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25				
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25				
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25				

Max. Eff. Inten. (mm/hr) = 148.50 132.61
over (min) = 5.00 10.00
Storage Coeff. (min) = 1.27 (ii) 4.87 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.33 0.22

PEAK FLOW (cms) = 0.08 0.02 0.101 (iii)
TIME TO PEAK (hrs) = 6.00 6.00
RUNOFF VOLUME (mm) = 117.50 87.00
TOTAL RAINFALL (mm) = 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0121)
1 + 2 = 3
ID= 1 (20101): 0.25 0.101 6.00 107.08
+ ID= 2 (20102): 0.47 0.183 6.00 105.13
ID = 3 (0121): 0.72 0.284 6.00 105.81

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.72 0.18
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 77.46 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94				
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94				
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94				
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94				
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94				
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94				
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94				
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94				
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94				
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94				
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94				
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94				
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25				
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25				
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25				
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25				
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25				
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25				
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25				
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25				
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25				
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25				
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25				
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25				
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25				
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25				
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25				
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25				
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25				
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25				
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25				
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25				
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25				
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25				
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25				
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25				

Max. Eff. Inten. (mm/hr) = 148.50 132.61
over (min) = 5.00 10.00
Storage Coeff. (min) = 1.87 (ii) 5.65 (ii)
Unit Hyd. Tpeak (min) = 5.00 10.00
Unit Hyd. peak (cms) = 0.32 0.15

PEAK FLOW (cms) = 0.30 0.06 0.336 (iii)
TIME TO PEAK (hrs) = 6.00 6.00
RUNOFF VOLUME (mm) = 111.50 87.00 106.60
TOTAL RAINFALL (mm) = 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN# = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (20122) Area (ha) = 1.11
ID= 1 DT= 5.0 min Total Imp (%) = 81.00 Dir. Conn. (%) = 81.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.90 0.21
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 86.02 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94				
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94				
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94				

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (20111) Area (ha) = 0.87
ID= 1 DT= 5.0 min Total Imp (%) = 67.00 Dir. Conn. (%) = 67.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.58 0.29
Dep. Storage (mm) = 1.00 1.00
Average Slope (%) = 1.00 2.00
Length (m) = 76.16 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94				
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94				
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94				
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94				
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94				
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94				
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94				
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94				
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94				
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94				
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94				
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94				
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25				
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25				
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25				
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25				
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25				
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25				
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25				
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25				
1.750</											

0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.166	2.81	4.166	6.75	7.166	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max.Eff.Inten.(mm/hr)= 148.50 132.61
over (min) 5.00 5.00
Storage Coeff (min)= 1.32 (ii) 4.01 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.24

TOTALS
PEAK FLOW (cms)= 0.10 0.01 0.14 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 109.04
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (20121) Area (ha)= 1.11 Dir. Conn.(%)= 56.00
ID= 1 DT= 5.0 min Total Imp(%)= 56.00

Surface Area (ha)= 0.62 IMPERVIOUS 0.49
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 86.02 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94

1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max.Eff.Inten.(mm/hr)= 148.50 132.61
over (min) 5.00 5.00
Storage Coeff (min)= 0.97 (ii) 3.71 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.25

TOTALS
PEAK FLOW (cms)= 0.04 0.00 0.041 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 108.92
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200121) Area (ha)= 0.11 Dir. Conn.(%)= 87.00
ID= 1 DT= 5.0 min Total Imp(%)= 87.00

Surface Area (ha)= 0.10 IMPERVIOUS 0.01
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 27.08 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94

1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25

1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max.Eff.Inten.(mm/hr)= 148.50 132.61
over (min) 5.00 5.00
Storage Coeff (min)= 1.99 (ii) 8.88 (iii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.31 0.12

TOTALS
PEAK FLOW (cms)= 0.26 0.14 0.398 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 100.72
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200111) Area (ha)= 0.10 Dir. Conn.(%)= 89.50
ID= 1 DT= 5.0 min Total Imp(%)= 89.50

Surface Area (ha)= 0.09 IMPERVIOUS 0.01
Dep. Storage (mm)= 1.00 PERVIOUS (i) 1.00
Average Slope (%)= 1.00 2.00
Length (m)= 25.82 40.00
Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	

0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr) = 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.74 (ii) 4.59 (iii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.32 0.23

PEAK FLOW (cms) = 0.26 0.03 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.00 0.285 (iii)
RUNOFF VOLUME (mm) = 111.50 87.00 108.80
TOTAL RAINFALL (mm) = 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (200122)	Area (ha) = 0.13
ID= 1 DT= 5.0 min	Total Imp (%) = 86.00	Dir. Conn. (%) = 86.00

Surface Area (ha) =	1.00	PERVIOUS (i)
Dep. Storage (mm) =	1.00	
Average Slope (%) =	1.00	
Length (m) =	29.44	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	20.25	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	20.25	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25

Max. Eff. Inten. (mm/hr) = 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.74 (ii) 4.59 (iii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.32 0.23

PEAK FLOW (cms) = 0.26 0.03 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.00 0.285 (iii)
RUNOFF VOLUME (mm) = 111.50 87.00 108.80
TOTAL RAINFALL (mm) = 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0147)	OVERFLOW IS OFF	
ID= 2 --> OUT= 1		
DT= 5.0 min		
OUTFLOW (cms) = 0.0000	STORAGE (ha.m.) = 0.2820	STORAGE (ha.m.) = 0.0040

AREA (ha) = 0.700	QPEAK (cms) = 0.285	TPEAK (hrs) = 6.00	R.V. (mm) = 108.80	
INFLOW : ID= 2 (20021)	0.700	0.283	6.00	108.80
OUTFLOW : ID= 1 (0147)	0.700	0.283	6.00	108.80

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.58
TIME SHIFT OF PEAK FLOW (hrs) = 0.00
MAXIMUM STORAGE USED (ha.m.) = 0.0040

CALIB

1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr) = 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.05 (ii) 4.23 (iii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.34 0.24

PEAK FLOW (cms) = 0.05 0.01 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.00 0.053 (iii)
RUNOFF VOLUME (mm) = 111.50 87.00 108.06
TOTAL RAINFALL (mm) = 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA (ha) = 0.10	QPEAK (cms) = 0.040	TPEAK (hrs) = 6.00	R.V. (mm) = 106.33
ID= 1 (200112)	0.10	0.040	6.00	106.33
+ ID2= 2 (200122)	0.13	0.053	6.00	108.06
ID= 3 (0145)	0.23	0.092	6.00	107.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0146)	OVERFLOW IS OFF		
ID= 2 --> OUT= 1			
DT= 5.0 min			
OUTFLOW (cms) = 0.0000	STORAGE (ha.m.) = 0.0000	OUTFLOW (cms) = 0.0910	STORAGE (ha.m.) = 0.0027

AREA (ha) = 0.229	QPEAK (cms) = 0.092	TPEAK (hrs) = 6.00	R.V. (mm) = 107.31	
INFLOW : ID= 2 (0145)	0.229	0.087	6.00	107.31
OUTFLOW : ID= 1 (0146)	0.229	0.087	6.00	107.31

PEAK FLOW REDUCTION [Qout/Qin] (%) = 94.15
TIME SHIFT OF PEAK FLOW (hrs) = 0.00
MAXIMUM STORAGE USED (ha.m.) = 0.0027

CALIB	STANDHYD (20021)	Area (ha) = 0.70
ID= 1 DT= 5.0 min	Total Imp (%) = 89.00	Dir. Conn. (%) = 89.00

Surface Area (ha) =	0.62	PERVIOUS (i)
Dep. Storage (mm) =	1.00	
Average Slope (%) =	1.00	
Length (m) =	68.31	40.00
Mannings n =	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	20.25	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25

Max. Eff. Inten. (mm/hr) = 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min) = 1.50 (ii) 4.23 (iii)
Unit Hyd. Tpeak (min) = 5.00 5.00
Unit Hyd. peak (cms) = 0.33 0.24

PEAK FLOW (cms) = 0.16 0.02 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.00 0.175 (iii)
RUNOFF VOLUME (mm) = 111.50 87.00 109.05
TOTAL RAINFALL (mm) = 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.97

***** WARNING: STORAGE COEFF.

OUTFLOW: ID= 1 (0149) 0.430 0.133 6.00 109.02

PEAK FLOW REDUCTION [Qout/Qin](%)= 75.92
TIME SHIFT OF PEAK FLOW (min)= 0.00
MAXIMUM STORAGE USED (ha.m.)= 0.0094

CALIB STANDBYD (20032) ID= 1 DT= 5.0 min Area (ha)= 0.47 Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.41 0.06 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 55.98 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr

Max.Eff.Inten.(mm/hr)= 148.50 132.61 over (min)= 5.00 5.00 Storage Coeff. (min)= 1.54 (ii) 4.62 (ii) Unit Hyd. Tpeak (min)= 5.00 5.00 Unit Hyd. peak (cms)= 0.33 0.22

TOTALS (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0150) IN= 2--> OUT= 1 DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW STORAGE (cms) (ha.m.) 0.0000 0.0000 0.1540 0.0102

INFLOW : ID= 2 (20032) 0.470 0.191 6.00 108.31
OUTFLOW: ID= 1 (0150) 0.470 0.144 6.00 108.28

PEAK FLOW REDUCTION [Qout/Qin](%)= 75.58
TIME SHIFT OF PEAK FLOW (min)= 0.00
MAXIMUM STORAGE USED (ha.m.)= 0.0102

CALIB STANDBYD (20041) ID= 1 DT= 5.0 min Area (ha)= 0.35 Total Imp(%)= 89.00 Dir. Conn.(%)= 89.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.31 1.00 Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 40.00 Length (m)= 45.30 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr

Max.Eff.Inten.(mm/hr)= 148.50 132.61 over (min)= 5.00 5.00 Storage Coeff. (min)= 1.41 (ii) 4.25 (ii) Unit Hyd. Tpeak (min)= 5.00 5.00 Unit Hyd. peak (cms)= 0.33 0.23

TOTALS (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0151) IN= 2--> OUT= 1 DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW STORAGE (cms) (ha.m.) 0.0000 0.0000 0.1330 0.0051

INFLOW : ID= 2 (20041) 0.350 0.142 6.00 108.80
OUTFLOW: ID= 1 (0151) 0.350 0.128 6.00 108.79

PEAK FLOW REDUCTION [Qout/Qin](%)= 90.12
TIME SHIFT OF PEAK FLOW (min)= 0.00
MAXIMUM STORAGE USED (ha.m.)= 0.0051

CALIB STANDBYD (20061) ID= 1 DT= 5.0 min Area (ha)= 0.65 Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.55 0.10 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 65.83 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr

Max.Eff.Inten.(mm/hr)= 148.50 132.61 over (min)= 5.00 5.00 Storage Coeff. (min)= 1.70 (ii) 4.99 (ii) Unit Hyd. Tpeak (min)= 5.00 5.00

Unit Hyd. peak (cms)= 0.32 0.22 *TOTALS* (iii) PEAK FLOW (cms)= 0.23 0.03 0.263 (iii) TIME TO PEAK (hrs)= 6.00 6.00 6.00 RUNOFF VOLUME (mm)= 111.50 87.00 107.82 TOTAL RAINFALL (mm)= 112.50 112.50 112.50 RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR(0154) IN= 2--> OUT= 1 DT= 5.0 min

OVERFLOW IS OFF

OUTFLOW STORAGE (cms) (ha.m.) 0.0000 0.0000 0.2410 0.0099

INFLOW : ID= 2 (20061) 0.650 0.263 6.00 107.82
OUTFLOW: ID= 1 (0154) 0.650 0.230 6.00 107.81

PEAK FLOW REDUCTION [Qout/Qin](%)= 87.65
TIME SHIFT OF PEAK FLOW (min)= 0.00
MAXIMUM STORAGE USED (ha.m.)= 0.0099

CALIB STANDBYD (20062) ID= 1 DT= 5.0 min Area (ha)= 0.73 Total Imp(%)= 80.00 Dir. Conn.(%)= 80.00

IMPERVIOUS PERVIOUS (i) Surface Area (ha)= 0.58 0.15 Dep. Storage (mm)= 1.00 1.00 Average Slope (%)= 1.00 2.00 Length (m)= 69.76 40.00 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr

Max.Eff.Inten.(mm/hr)= 148.50 132.61 over (min)= 5.00 5.00 Storage Coeff. (min)= 1.70 (ii) 4.99 (ii) Unit Hyd. Tpeak (min)= 5.00 5.00

2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)=	148.50	132.61
over (min)	5.00	10.00
Storage Coeff. (min)=	0.87 (ii)	4.26 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)=	0.32	0.16
PEAK FLOW (cms)=	0.24	0.05
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	111.50	87.00
TOTAL RAINFALL (mm)=	112.50	112.50
RUNOFF COEFFICIENT =	0.99	0.77

TOTALS
 0.288 (iii)
 6.00
 106.60
 112.50
 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0155)				
IN= 2--> OUT= 1				
DT= 5.0 min				
OVERFLOW IS OFF				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.2430	0.0135	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
0.730	0.289	6.00	106.60	
OUTFLOW: ID= 1 (0155)	0.730	0.227	6.00	106.59

PEAK FLOW REDUCTION [Qout/Qin] (%) = 78.52
 TIME SHIFT OF PEAK FLOW (min) = 0
 MAXIMUM STORAGE USED (ha.m.) = 0.0135

CALIB (STANDHYD (200812))			
ID= 1 DT= 5.0 min			
Area (ha)	=	0.38	
Total Imp (%)	=	70.00	Dir. Conn. (%) = 70.00

Surface Area (ha)	=	0.27	IMPERVIOUS (i)	0.11
Dep. Storage (mm)	=	1.00	PERVIOUS (i)	1.00
Average Slope (%)	=	1.00		2.00
Length (m)	=	50.33		40.00
Mannings n	=	0.013		0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	10.00	9.75	3.94
1.167	2.81	4.167	6.75	7.167	10.17	2.25	2.25
1.250	2.81	4.250	6.75	7.250	10.25	2.25	2.25
1.333	2.81	4.333	6.75	7.333	10.33	2.25	2.25
1.417	2.81	4.417	6.75	7.417	10.42	2.25	2.25
1.500	2.81	4.500	6.75	7.500	10.50	2.25	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.38	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25

2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)=	148.50	132.61
over (min)	5.00	10.00
Storage Coeff. (min)=	0.87 (ii)	4.26 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)=	0.34	0.23
PEAK FLOW (cms)=	0.02	0.00
TIME TO PEAK (hrs)=	5.92	6.00
RUNOFF VOLUME (mm)=	111.50	87.00
TOTAL RAINFALL (mm)=	112.50	112.50
RUNOFF COEFFICIENT =	0.99	0.77

TOTALS
 0.028 (iii)
 6.00
 107.57
 112.50
 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0110)				
1 + 2 = 3				
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (200811):	0.07	0.028	6.00	107.57
+ ID2= 2 (200812):	0.38	0.147	6.00	104.14
ID = 3 (0110):	0.45	0.175	6.00	104.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0158)				
IN= 2--> OUT= 1				
DT= 5.0 min				
OVERFLOW IS OFF				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.0750	0.0162	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
0.450	0.175	6.00	104.68	
OUTFLOW: ID= 1 (0158)	0.450	0.074	6.08	104.57

PEAK FLOW REDUCTION [Qout/Qin] (%) = 42.25
 TIME SHIFT OF PEAK FLOW (min) = 5.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0162

CALIB (STANDHYD (200822))			
ID= 1 DT= 5.0 min			
Area (ha)	=	0.39	
Total Imp (%)	=	59.00	Dir. Conn. (%) = 59.00

Surface Area (ha)	=	0.23	IMPERVIOUS (i)	0.16
Dep. Storage (mm)	=	1.00	PERVIOUS (i)	1.00
Average Slope (%)	=	1.00		2.00
Length (m)	=	50.99		40.00
Mannings n	=	0.013		0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94

2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)=	148.50	132.61
over (min)	5.00	10.00
Storage Coeff. (min)=	1.44 (ii)	6.14 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)=	0.33	0.15
PEAK FLOW (cms)=	0.11	0.04
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	111.50	87.00
TOTAL RAINFALL (mm)=	112.50	112.50
RUNOFF COEFFICIENT =	0.99	0.77

TOTALS
 0.147 (iii)
 6.00
 104.14
 112.50
 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (STANDHYD (200811))			
ID= 1 DT= 5.0 min			
Area (ha)	=	0.07	
Total Imp (%)	=	84.00	Dir. Conn. (%) = 84.00

Surface Area (ha)	=	0.06	IMPERVIOUS (i)	0.01
Dep. Storage (mm)	=	1.00	PERVIOUS (i)	1.00
Average Slope (%)	=	1.00		2.00
Length (m)	=	21.60		40.00
Mannings n	=	0.013		0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3		

1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min)= 0.87 (ii) 4.36 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.34 0.23

PEAK FLOW (cms)= 0.02 0.00 *TOTALS*
TIME TO PEAK (hrs)= 5.92 6.00 0.028 (iii)
RUNOFF VOLUME (mm)= 111.50 87.00 107.32
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0111)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (200821):	0.07	0.028	6.00	107.32
+ ID2= 2 (200822):	0.39	0.142	6.00	101.44
=====				
ID = 3 (0111):	0.46	0.171	6.00	102.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0159)				
IN= 2 --> OUT= 1				
DT= 5.0 min				
OVERFLOW IS OFF				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.0850	0.0141	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (0111)	0.460	0.171	6.00	102.34
OUTFLOW : ID= 1 (0159)	0.460	0.083	6.08	102.26

PEAK FLOW REDUCTION [Qout/Qin] (%) = 48.79
TIME SHIFT OF PEAK FLOW (min) = 5.00
MAXIMUM STORAGE USED (ha.m.) = 0.0141

CALIB				
STANDHYD (201021)				
ID= 1 DT= 5.0 min				
Area (ha)	= 0.47			
Total Imp (%)	= 85.00 Dir. Conn. (%) = 85.00			
IMPERVIOUS PERVIOUS (i)				
Surface Area (ha)	= 0.40 0.07			
Dep. Storage (mm)	= 1.00 1.00			
Average Slope (%)	= 1.00 2.00			
Length (m)	= 55.98 40.00			
Mannings n	= 0.013 0.290			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN				
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr				
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94				
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94				
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94				
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94				
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94				
0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94				
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94				
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94				
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94				
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94				
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94				
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94				
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25				
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25				
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25				
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25				
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25				
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25				
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25				
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25				
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25				
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25				
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25				
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25				
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25				
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25				
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25				
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25				
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25				
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25				
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25				
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25				
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25				
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25				
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25				
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25				

Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min)= 1.54 (ii) 4.83 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.16 0.03 0.190 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 107.82
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (201022)				
ID= 1 DT= 5.0 min				
Area (ha)	= 0.47			
Total Imp (%)	= 85.00 Dir. Conn. (%) = 85.00			
IMPERVIOUS PERVIOUS (i)				
Surface Area (ha)	= 0.40 0.07			
Dep. Storage (mm)	= 1.00 1.00			
Average Slope (%)	= 1.00 2.00			
Length (m)	= 55.98 40.00			
Mannings n	= 0.013 0.290			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN				
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr				
0.083	2.81	3.083	4.50	6.083	20.25	9.08	3.94				
0.167	2.81	3.167	4.50	6.167	20.25	9.17	3.94				
0.250	2.81	3.250	4.50	6.250	20.25	9.25	3.94				
0.333	2.81	3.333	4.50	6.333	20.25	9.33	3.94				
0.417	2.81	3.417	4.50	6.417	20.25	9.42	3.94				

0.500	2.81	3.500	4.50	6.500	20.25	9.50	3.94
0.583	2.81	3.583	4.50	6.583	9.00	9.58	3.94
0.667	2.81	3.667	4.50	6.667	9.00	9.67	3.94
0.750	2.81	3.750	4.50	6.750	9.00	9.75	3.94
0.833	2.81	3.833	4.50	6.833	9.00	9.83	3.94
0.917	2.81	3.917	4.50	6.917	9.00	9.92	3.94
1.000	2.81	4.000	4.50	7.000	9.00	10.00	3.94
1.083	2.81	4.083	6.75	7.083	6.75	10.08	2.25
1.167	2.81	4.167	6.75	7.167	6.75	10.17	2.25
1.250	2.81	4.250	6.75	7.250	6.75	10.25	2.25
1.333	2.81	4.333	6.75	7.333	6.75	10.33	2.25
1.417	2.81	4.417	6.75	7.417	6.75	10.42	2.25
1.500	2.81	4.500	6.75	7.500	6.75	10.50	2.25
1.583	2.81	4.583	9.00	7.583	6.75	10.58	2.25
1.667	2.81	4.667	9.00	7.667	6.75	10.67	2.25
1.750	2.81	4.750	9.00	7.750	6.75	10.75	2.25
1.833	2.81	4.833	9.00	7.833	6.75	10.83	2.25
1.917	2.81	4.917	9.00	7.917	6.75	10.92	2.25
2.000	2.81	5.000	9.00	8.000	6.75	11.00	2.25
2.083	3.37	5.083	13.50	8.083	3.94	11.08	2.25
2.167	3.37	5.167	13.50	8.167	3.94	11.17	2.25
2.250	3.37	5.250	13.50	8.250	3.94	11.25	2.25
2.333	3.37	5.333	13.50	8.333	3.94	11.33	2.25
2.417	3.37	5.417	13.50	8.417	3.94	11.42	2.25
2.500	3.37	5.500	13.50	8.500	3.94	11.50	2.25
2.583	3.37	5.583	54.00	8.583	3.94	11.58	2.25
2.667	3.37	5.667	54.00	8.667	3.94	11.67	2.25
2.750	3.37	5.750	54.00	8.750	3.94	11.75	2.25
2.833	3.37	5.833	148.50	8.833	3.94	11.83	2.25
2.917	3.37	5.917	148.50	8.917	3.94	11.92	2.25
3.000	3.37	6.000	148.50	9.000	3.94	12.00	2.25

Max. Eff. Inten. (mm/hr)= 148.50 132.61
over (min) = 5.00 5.00
Storage Coeff. (min)= 1.54 (ii) 4.83 (ii)
Unit Hyd. Tpeak (min)= 5.00 5.00
Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.16 0.03 0.190 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 111.50 87.00 107.82
TOTAL RAINFALL (mm)= 112.50 112.50 112.50
RUNOFF COEFFICIENT = 0.99 0.77 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

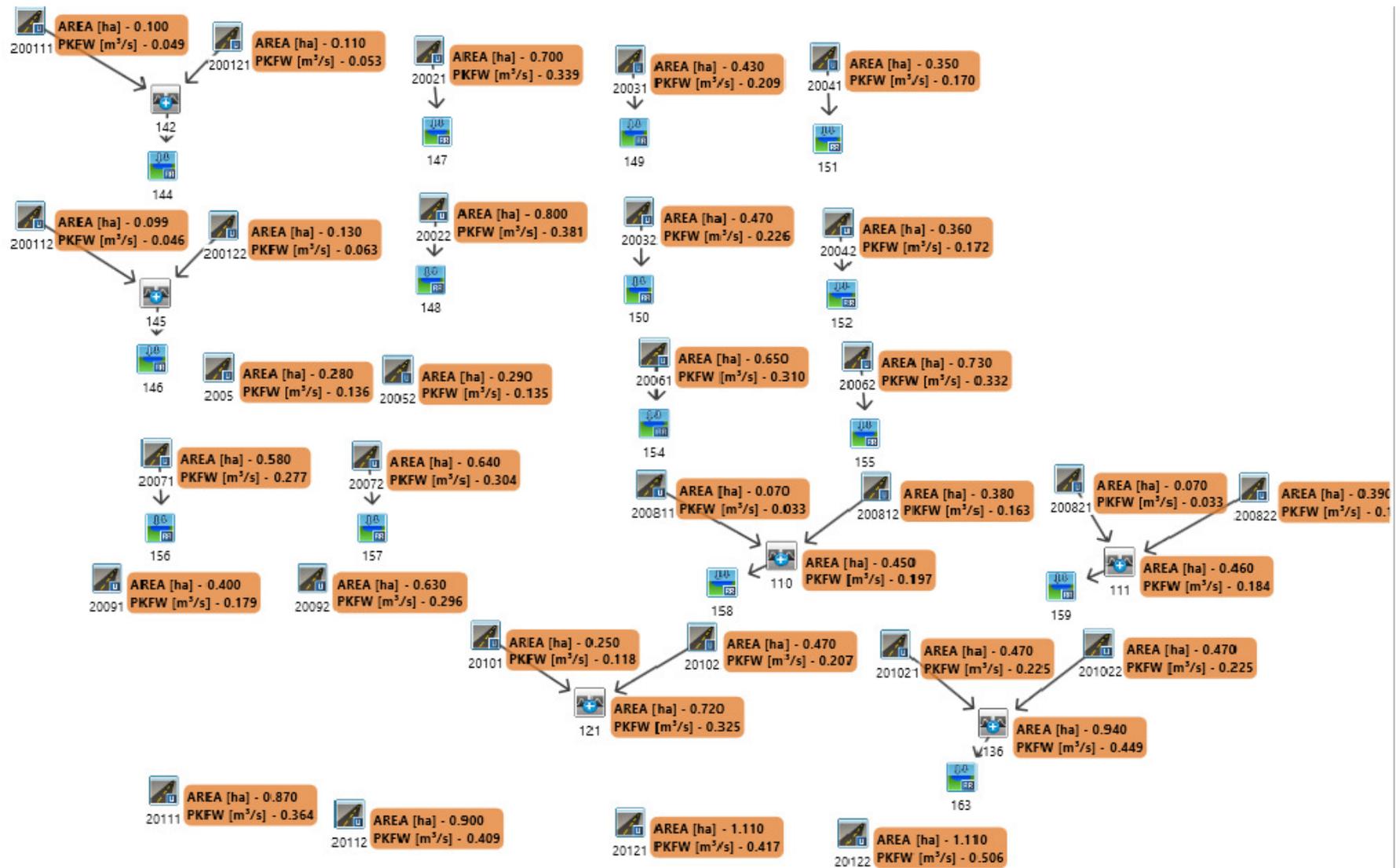
ADD HYD (0136)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (201021):	0.47	0.190	6.00	107.82
+ ID2= 2 (201022):	0.47	0.190	6.00	107.82
=====				
ID = 3 (0136):	0.94	0.380	6.00	107.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0163)				
IN= 2 --> OUT= 1				
DT= 5.0 min				
OVERFLOW IS OFF				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.3400	0.0159	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (0136)	0.940	0.380	6.00	107.82
OUTFLOW : ID= 1 (0163)	0.940	0.322	6.00	107.81

PEAK FLOW REDUCTION [Qout/Qin] (%) = 84.82
TIME SHIFT OF PEAK FLOW (min) = 0.00
MAXIMUM STORAGE USED (ha.m.) = 0.0159

100-Yr Post-Development Required Storage Volume Roadway Drainage Areas (4-hr Chicago Storm)



```

V V I SSSSS U U A L (v 6.1.2002)
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O O
O O T T H H Y Y M M O O O
OOO T T H H Y Y M M O O O
Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2020 Smart City Water Inc
All rights reserved.

```

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\VisualOTHMMO 6.1\VO2\vojn.dat
Output filename: C:\Users\caef070146\AppData\Local\Civica\H5\50564c49-8c07-4086-8157-8729fb3caba3\8c053552
Summary filename: C:\Users\caef070146\AppData\Local\Civica\H5\50564c49-8c07-4086-8157-8729fb3caba3\8c053552

DATE: 05-18-2022 TIME: 01:00:02

USER:

COMMENTS:

***** SIMULATION : Run 06 *****

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	6.41	1.17	45.22	2.17	14.50	3.17	7.56
0.33	7.29	1.33	180.15	2.33	12.44	3.33	7.04
0.50	8.52	1.50	58.54	2.50	10.94	3.50	6.60
0.67	10.36	1.67	31.96	2.67	9.80	3.67	6.22
0.83	13.45	1.83	22.45	2.83	8.90	3.83	5.89
1.00	19.96	2.00	17.52	3.00	8.16	4.00	5.59

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.23	0.06
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	43.97	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Dep. Storage	(mm)=	1.00	1.00
Dep. Storage	(mm)=	1.00	2.00
Average Slope	(%)=	51.64	40.00
Length	(m)=	0.013	0.290
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)=	180.15	134.19
Max. Eff. Inten. (mm/hr)=	180.15	134.19
over (min)	5.00	10.00
Storage Coeff. (min)	1.36 (ii)	5.12 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.33	0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.35	0.12
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	55.98	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)=	180.15	134.19
Max. Eff. Inten. (mm/hr)=	180.15	134.19
over (min)	5.00	10.00
Storage Coeff. (min)	1.43 (ii)	5.44 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.33	0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.51	0.12
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	64.81	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.23	0.06
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	43.97	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Dep. Storage	(mm)=	1.00	1.00
Dep. Storage	(mm)=	1.00	2.00
Average Slope	(%)=	51.64	40.00
Length	(m)=	0.013	0.290
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)=	180.15	134.19
Max. Eff. Inten. (mm/hr)=	180.15	134.19
over (min)	5.00	10.00
Storage Coeff. (min)	1.36 (ii)	5.12 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.33	0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN^{*} = 89.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.20	0.05
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	40.82	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167					

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min) 5.00 5.00
 Storage Coeff. (min)= 0.90 (ii) 3.48 (iii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.26

TOTALS
 PEAK FLOW (cms)= 0.04 0.06 0.049 (iii)
 TIME TO PEAK (hrs)= 1.33 1.33
 RUNOFF VOLUME (mm)= 86.58 63.54 84.14
 TOTAL RAINFALL (mm)= 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0142)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (200111):	0.10	0.049	1.33	84.14
+ ID2 = 2 (200121):	0.11	0.053	1.33	83.56

ID = 3 (0142):	0.21	0.102	1.33	83.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0144)	OVERFLOW IS OFF
IN = 2--> OUT = 1	
DT = 5.0 min	
	OUTFLOW STORAGE OUTFLOW STORAGE
	(cms) (ha.m.) (cms) (ha.m.)
	0.0000 0.0000 0.0980 0.0025
	AREA QPEAK TPEAK R.V.
	(ha) (cms) (hrs) (mm)
INFLOW: ID = 2 (0142)	0.210 0.102 1.33 83.84
OUTFLOW: ID = 1 (0144)	0.210 0.088 1.33 83.83

PEAK FLOW REDUCTION [Qout/Qin] (%) = 86.25
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0025

CALIB STANDBYD (200122)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
ID = 1 DT = 5.0 min	0.13	86.00	86.00

Surface Area (ha) = 0.11
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.00
 Length (m) = 29.44
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min) 5.00 5.00
 Storage Coeff. (min)= 0.97 (ii) 3.92 (iii)

RESERVOIR (0146)	OVERFLOW IS OFF
IN = 2--> OUT = 1	
DT = 5.0 min	
	OUTFLOW STORAGE OUTFLOW STORAGE
	(cms) (ha.m.) (cms) (ha.m.)
	0.0000 0.0000 0.1070 0.0024
	AREA QPEAK TPEAK R.V.
	(ha) (cms) (hrs) (mm)
INFLOW: ID = 2 (0145)	0.229 0.109 1.33 82.63
OUTFLOW: ID = 1 (0146)	0.229 0.097 1.33 82.63

PEAK FLOW REDUCTION [Qout/Qin] (%) = 89.01
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0024

CALIB STANDBYD (20021)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
ID = 1 DT = 5.0 min	0.70	89.00	89.00

Surface Area (ha) = 0.62
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.00
 Length (m) = 68.31
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min) 5.00 5.00
 Storage Coeff. (min)= 1.61 (ii) 4.25 (iii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.32 0.24

PEAK FLOW (cms)= 0.31 0.03 0.339 (iii)
 TIME TO PEAK (hrs)= 1.33 1.33
 RUNOFF VOLUME (mm)= 86.58 63.54 84.04
 TOTAL RAINFALL (mm)= 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0147)	OVERFLOW IS OFF
IN = 2--> OUT = 1	
DT = 5.0 min	
	OUTFLOW STORAGE OUTFLOW STORAGE
	(cms) (ha.m.) (cms) (ha.m.)
	0.0000 0.0000 0.3660 0.0083
	AREA QPEAK TPEAK R.V.
	(ha) (cms) (hrs) (mm)
INFLOW: ID = 2 (20021)	0.700 0.339 1.33 84.04
OUTFLOW: ID = 1 (0147)	0.700 0.299 1.33 84.04

PEAK FLOW REDUCTION [Qout/Qin] (%) = 88.14
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0074

Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.24
 PEAK FLOW (cms)= 0.06 0.01 0.063 (iii)
 TIME TO PEAK (hrs)= 1.33 1.33
 RUNOFF VOLUME (mm)= 86.58 63.54 83.33
 TOTAL RAINFALL (mm)= 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (200112)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
ID = 1 DT = 5.0 min	0.10	79.00	79.00

Surface Area (ha) = 0.08
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.00
 Length (m) = 25.69
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min) 5.00 5.00
 Storage Coeff. (min)= 0.89 (ii) 4.48 (iii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.23

PEAK FLOW (cms)= 0.04 0.01 0.046 (iii)
 TIME TO PEAK (hrs)= 1.33 1.33
 RUNOFF VOLUME (mm)= 86.58 63.54 81.73
 TOTAL RAINFALL (mm)= 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0145)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (200112):	0.10	0.046	1.33	81.73
+ ID2 = 2 (200122):	0.13	0.063	1.33	83.33

ID = 3 (0145):	0.23	0.109	1.33	82.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0146) OVERFLOW IS OFF

CALIB STANDBYD (20022)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
ID = 1 DT = 5.0 min	0.80	85.00	85.00

Surface Area (ha) = 0.68
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.00
 Length (m) = 73.03
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min) 5.00 5.00
 Storage Coeff. (min)= 1.67 (ii) 4.72 (iii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.32 0.22

PEAK FLOW (cms)= 0.34 0.04 0.381 (iii)
 TIME TO PEAK (hrs)= 1.33 1.33
 RUNOFF VOLUME (mm)= 86.58 63.54 83.12
 TOTAL RAINFALL (mm)= 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0148)	OVERFLOW IS OFF
IN = 2--> OUT = 1	
DT = 5.0 min	
	OUTFLOW STORAGE OUTFLOW STORAGE
	(cms) (ha.m.) (cms) (ha.m.)
	0.0000 0.0000 0.3660 0.0084
	AREA QPEAK TPEAK R.V.
	(ha) (cms) (hrs) (mm)
INFLOW: ID = 2 (20022)	0.800 0.381 1.33 83.12
OUTFLOW: ID = 1 (0148)	0.800 0.333 1.33 83.12

PEAK FLOW REDUCTION [Qout/Qin] (%) = 87.23
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0084

CALIB STANDBYD (20031)	Area (ha)	Total Imp (%)	Dir. Conn. (%)
ID = 1 DT = 5.0 min	0.43	90.00	90.00

Surface Area (ha) = 0.39
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.00
 Length (m) = 53.54
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.43 (ii) 4.28 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.33 0.24

PEAK FLOW (cms) = 0.19 0.02 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33 0.209 (iii)
 RUNOFF VOLUME (mm) = 86.58 63.54 84.27
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0149) OVERFLOW IS OFF
 IN= 2--> OUT= 1
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1590	0.0080

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
0.430	0.209	1.33	84.27
0.450	0.139	1.42	84.25

PEAK FLOW REDUCTION [Qout/Qin] (%) = 66.51
 TIME SHIFT OF PEAK FLOW (min) = 5.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0080

CALIB STANDHYD (20032)
 ID= 1 DT= 5.0 min

Area (ha)	Total Imp (%)	Dir. Conn. (%)
0.47	87.00	87.00

IMPERVIOUS (%)	PERVIOUS (i)
0.41	0.06
1.00	1.00
1.00	2.00
55.98	40.00
0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.32 (ii) 4.36 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.33 0.23

PEAK FLOW (cms) = 0.15 0.02 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33 0.172 (iii)
 RUNOFF VOLUME (mm) = 86.58 63.54 83.11
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0151) OVERFLOW IS OFF
 IN= 2--> OUT= 1
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1490	0.0051

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
0.350	0.170	1.33	84.04
0.350	0.131	1.33	84.03

PEAK FLOW REDUCTION [Qout/Qin] (%) = 77.28
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0051

CALIB STANDHYD (20042)
 ID= 1 DT= 5.0 min

Area (ha)	Total Imp (%)	Dir. Conn. (%)
0.36	85.00	85.00

IMPERVIOUS (%)	PERVIOUS (i)
0.31	0.05
1.00	1.00
1.00	2.00
48.99	40.00
0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.32 (ii) 4.36 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.33 0.23

PEAK FLOW (cms) = 0.15 0.02 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33 0.172 (iii)
 RUNOFF VOLUME (mm) = 86.58 63.54 83.11
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0152) OVERFLOW IS OFF
 IN= 2--> OUT= 1
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1670	0.0038

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
0.650	0.310	1.33	83.12
0.650	0.238	1.33	83.11

PEAK FLOW REDUCTION [Qout/Qin] (%) = 76.64
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0031

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.43 (ii) 4.28 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.33 0.23

PEAK FLOW (cms) = 0.20 0.02 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33 0.210 (iii)
 RUNOFF VOLUME (mm) = 86.58 63.54 83.57
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0150) OVERFLOW IS OFF
 IN= 2--> OUT= 1
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1820	0.0079

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
0.470	0.226	1.33	83.57
0.470	0.157	1.33	83.57

PEAK FLOW REDUCTION [Qout/Qin] (%) = 69.53
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0079

CALIB STANDHYD (20041)
 ID= 1 DT= 5.0 min

Area (ha)	Total Imp (%)	Dir. Conn. (%)
0.35	89.00	89.00

IMPERVIOUS (%)	PERVIOUS (i)
0.31	0.04
1.00	1.00
1.00	2.00
48.30	40.00
0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.30 (ii) 3.95 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.33 0.24

PEAK FLOW (cms) = 0.16 0.01 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33 0.170 (iii)
 RUNOFF VOLUME (mm) = 86.58 63.54 84.04
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0152) OVERFLOW IS OFF
 IN= 2--> OUT= 1
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.360	0.172	1.33	83.11
0.360	0.152	1.33	83.11

PEAK FLOW REDUCTION [Qout/Qin] (%) = 88.31
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0038

CALIB STANDHYD (20061)
 ID= 1 DT= 5.0 min

Area (ha)	Total Imp (%)	Dir. Conn. (%)
0.65	85.00	85.00

IMPERVIOUS (%)	PERVIOUS (i)
0.55	0.10
1.00	1.00
1.00	2.00
65.83	40.00
0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr) = 180.15 134.19
 over (min) = 5.00 5.00
 Storage Coeff. (min) = 1.57 (ii) 4.62 (ii)
 Unit Hyd. Tpeak (min) = 5.00 5.00
 Unit Hyd. peak (cms) = 0.33 0.22

PEAK FLOW (cms) = 0.28 0.03 *TOTALS*
 TIME TO PEAK (hrs) = 1.33 1.33 0.310 (iii)
 RUNOFF VOLUME (mm) = 86.58 63.54 83.12
 TOTAL RAINFALL (mm) = 87.58 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN^{*} = 89.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0154) OVERFLOW IS OFF
 IN= 2--> OUT= 1
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.2690	0.0091

AREA (ha)

Surface Area (ha)= 0.58 0.15
 Dep. Storage (mm)= 1.00 1.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 69.76 40.00
 Mannings n = 0.013 0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.63 (ii) 5.13 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.32 0.16

PEAK FLOW (cms)= 0.29 0.04 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.332 (iii)
 RUNOFF VOLUME (mm)= 86.58 63.54 81.96
 TOTAL RAINFALL (mm)= 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0155)		OVERFLOW IS OFF			
IN= 2--> OUT= 1		DT= 5.0 min			
		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.2600	0.0114
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (20062)		0.730	0.332	1.33	81.96
OUTFLOW : ID= 1 (0155)		0.730	0.232	1.42	81.96
		PEAK FLOW REDUCTION [qout/qin] (%) = 69.75			
		TIME SHIFT OF PEAK FLOW (min) = 5.00			
		MAXIMUM STORAGE USED (ha.m.) = 0.0114			

CALIB STANDBYD (20071)		Area (ha) = 0.58		Total Imp (%) = 85.00		Dir. Conn. (%) = 85.00	
ID= 1 DT= 5.0 min							
		IMPERVIOUS		PERVIOUS (i)			
Surface Area (ha)=	0.49						
Dep. Storage (mm)=	1.00						
Average Slope (%)=	1.00						
Length (m)=	62.18						
Mannings n =	0.013						

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

PEAK FLOW (cms)= 0.27 0.04 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.304 (iii)
 RUNOFF VOLUME (mm)= 86.58 63.54 82.89
 TOTAL RAINFALL (mm)= 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0157)		OVERFLOW IS OFF			
IN= 2--> OUT= 1		DT= 5.0 min			
		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.2600	0.0092
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (20072)		0.640	0.304	1.33	82.89
OUTFLOW : ID= 1 (0157)		0.640	0.229	1.33	82.88
		PEAK FLOW REDUCTION [qout/qin] (%) = 75.20			
		TIME SHIFT OF PEAK FLOW (min) = 0.00			
		MAXIMUM STORAGE USED (ha.m.) = 0.0092			

CALIB STANDBYD (200812)		Area (ha) = 0.38		Total Imp (%) = 70.00		Dir. Conn. (%) = 70.00	
ID= 1 DT= 5.0 min							
		IMPERVIOUS		PERVIOUS (i)			
Surface Area (ha)=	0.27						
Dep. Storage (mm)=	1.00						
Average Slope (%)=	1.00						
Length (m)=	50.33						
Mannings n =	0.013						

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.34 (ii) 5.68 (ii)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. peak (cms)= 0.33 0.15

PEAK FLOW (cms)= 0.33 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.42 0.163 (iii)
 RUNOFF VOLUME (mm)= 86.58 63.54 79.65
 TOTAL RAINFALL (mm)= 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.52 (ii) 4.56 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.33 0.23

PEAK FLOW (cms)= 0.25 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.033 (iii)
 RUNOFF VOLUME (mm)= 86.58 63.54 83.12
 TOTAL RAINFALL (mm)= 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN[#] = 89.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0156)		OVERFLOW IS OFF			
IN= 2--> OUT= 1		DT= 5.0 min			
		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.2800	0.0051
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (20071)		0.580	0.277	1.33	83.12
OUTFLOW : ID= 1 (0156)		0.580	0.258	1.33	83.12
		PEAK FLOW REDUCTION [qout/qin] (%) = 93.20			
		TIME SHIFT OF PEAK FLOW (min) = 0.00			
		MAXIMUM STORAGE USED (ha.m.) = 0.0050			

CALIB STANDBYD (20072)		Area (ha) = 0.64		Total Imp (%) = 84.00		Dir. Conn. (%) = 84.00	
ID= 1 DT= 5.0 min							
		IMPERVIOUS		PERVIOUS (i)			
Surface Area (ha)=	0.49						
Dep. Storage (mm)=	1.00						
Average Slope (%)=	1.00						
Length (m)=	65.32						
Mannings n =	0.013						

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max. Eff. Inten. (mm/hr)= 180.15 134.19
 over (min)= 5.00 10.00
 Storage Coeff. (min)= 1.56 (ii) 4.70 (ii)
 Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.33 0.22

PEAK FLOW (cms)= 0.27 0.01 *TOTALS*
 TIME TO PEAK (hrs)= 1.33 1.33 0.033 (iii)
 RUNOFF VOLUME (mm)= 86.58 63.54 82.86
 TOTAL RAINFALL (mm)= 87.58 87.58
 RUNOFF COEFFICIENT = 0.99 0.73 0.95

CALIB STANDBYD (200811)		Area (ha) = 0.07		Total Imp (%) = 84.00		Dir. Conn. (%) = 84.00	
ID= 1 DT= 5.0 min							
		IMPERVIOUS		PERVIOUS (i)			
Surface Area (ha)=	0.06						
Dep. Storage (mm)=	1.00						
Average Slope (%)=	1.00						
Length (m)=	21.60						
Mannings n =	0.013						

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME RAIN		TIME RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583			

STANDHYD (200822) Area (ha)= 0.39
 ID= 1 DT= 5.0 min Total Imp(%)= 59.00 Dir. Conn.(%)= 59.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.16	0.16
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	50.99	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max.Eff.Inten.(mm/hr)=	180.15	134.19	
over (min)	5.00	5.00	
Storage Coeff. (min)=	1.35 (ii)	8.21 (ii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.33	0.13	
TOTALS			
PEAK FLOW (cms)=	0.12	0.04	0.151 (iii)
TIME TO PEAK (hrs)=	1.33	1.42	1.33
RUNOFF VOLUME (mm)=	86.58	63.54	77.12
TOTAL RAINFALL (mm)=	87.58	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73	0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 I_a = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (200821) Area (ha)= 0.07
 ID= 1 DT= 5.0 min Total Imp(%)= 83.00 Dir. Conn.(%)= 83.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.06	0.01
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	21.60	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max.Eff.Inten.(mm/hr)=	180.15	134.19	
over (min)	5.00	5.00	
Storage Coeff. (min)=	0.80 (ii)	4.04 (ii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.33	0.23	
TOTALS			
PEAK FLOW (cms)=	0.20	0.02	0.225 (iii)
TIME TO PEAK (hrs)=	1.33	1.33	1.33
RUNOFF VOLUME (mm)=	86.58	63.54	83.12
TOTAL RAINFALL (mm)=	87.58	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 I_a = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (201021) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.40	0.07
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	55.98	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max.Eff.Inten.(mm/hr)=	180.15	134.19	
over (min)	5.00	5.00	
Storage Coeff. (min)=	1.43 (ii)	4.47 (ii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.33	0.23	
TOTALS			
PEAK FLOW (cms)=	0.20	0.02	0.225 (iii)
TIME TO PEAK (hrs)=	1.33	1.33	1.33
RUNOFF VOLUME (mm)=	86.58	63.54	83.12
TOTAL RAINFALL (mm)=	87.58	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 I_a = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0136)				
1 +	2 =	3	AREA (ha)	R.V. (mm)
ID1= 1 (201021):	0.47	0.225	1.33	83.12
+ ID2= 2 (201022):	0.47	0.225	1.33	83.12
ID = 3 (0136):	0.94	0.449	1.33	83.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0163)			
IN 2-->	OUT 1	DT= 5.0 min	OVERFLOW IS OFF
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1010	0.0090

Unit Hyd. Tpeak (min)= 5.00 5.00
 Unit Hyd. peak (cms)= 0.34 0.24

PEAK FLOW (cms)=	0.03	0.00	*TOTALS*
TIME TO PEAK (hrs)=	1.33	1.33	0.033 (iii)
RUNOFF VOLUME (mm)=	86.58	63.54	82.63
TOTAL RAINFALL (mm)=	87.58	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73	0.94

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 I_a = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0111)				
1 +	2 =	3	AREA (ha)	R.V. (mm)
ID1= 1 (200821):	0.07	0.033	1.33	82.63
+ ID2= 2 (200822):	0.39	0.151	1.33	77.12
ID = 3 (0111):	0.46	0.184	1.33	77.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0159)			
IN 2-->	OUT 1	DT= 5.0 min	OVERFLOW IS OFF
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1010	0.0090
INFLOW : ID= 2 (0111)	0.460	0.184	1.33
OUTFLOW: ID= 1 (0159)	0.460	0.099	1.42

PEAK FLOW REDUCTION [qout/qin] (%) = 53.86
 TIME SHIFT OF PEAK FLOW (min) = 3.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0090

CALIB STANDHYD (201021) Area (ha)= 0.47
 ID= 1 DT= 5.0 min Total Imp(%)= 85.00 Dir. Conn.(%)= 85.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.40	0.07
Dep. Storage	(mm)=	1.00	1.00
Average Slope	(%)=	1.00	2.00
Length	(m)=	55.98	40.00
Mannings n	=	0.013	0.290

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.41	1.083	45.22	2.083	14.50	3.08	7.56
0.167	6.41	1.167	45.22	2.167	14.50	3.17	7.56
0.250	7.29	1.250	180.15	2.250	12.44	3.25	7.04
0.333	7.29	1.333	180.15	2.333	12.44	3.33	7.04
0.417	8.52	1.417	58.54	2.417	10.94	3.42	6.60
0.500	8.52	1.500	58.54	2.500	10.94	3.50	6.60
0.583	10.36	1.583	31.96	2.583	9.80	3.58	6.22
0.667	10.36	1.667	31.96	2.667	9.80	3.67	6.22
0.750	13.45	1.750	22.45	2.750	8.90	3.75	5.89
0.833	13.45	1.833	22.45	2.833	8.90	3.83	5.89
0.917	19.96	1.917	17.52	2.917	8.16	3.92	5.59
1.000	19.96	2.000	17.52	3.000	8.16	4.00	5.59

Max.Eff.Inten.(mm/hr)=	180.15	134.19	
over (min)	5.00	5.00	
Storage Coeff. (min)=	1.43 (ii)	4.47 (ii)	
Unit Hyd. Tpeak (min)=	5.00	10.00	
Unit Hyd. peak (cms)=	0.33	0.23	
TOTALS			
PEAK FLOW (cms)=	0.20	0.02	0.225 (iii)
TIME TO PEAK (hrs)=	1.33	1.33	1.33
RUNOFF VOLUME (mm)=	86.58	63.54	83.12
TOTAL RAINFALL (mm)=	87.58	87.58	87.58
RUNOFF COEFFICIENT =	0.99	0.73	0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN² = 89.0 I_a = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.0000 0.0000 | 0.3800 0.0140

RESERVOIR (0163)				
IN 2-->	OUT 1	DT= 5.0 min	OVERFLOW IS OFF	
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.1010	0.0140	
INFLOW : ID= 2 (0136)	0.940	0.449	1.33	
OUTFLOW: ID= 1 (0163)	0.940	0.334	1.33	

PEAK FLOW REDUCTION [qout/qin] (%) = 74.38
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0140

FINISH

APPENDIX

D

CulvertMaster Results



Culvert Calculator Report

25th Innisfil Side Road (101) <duplicate>

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.61 m	Headwater Depth/Height	0.88
Computed Headwater Elev:	224.61 m	Discharge	3.7780 m ³ /s
Inlet Control HW Elev.	224.49 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.61 m	Control Type	Outlet Control

Grades			
Upstream Invert	223.00 m	Downstream Invert	222.78 m
Length	29.00 m	Constructed Slope	0.007586 m/m

Hydraulic Profile			
Profile	M2	Depth, Downstream	0.95 m
Slope Type	Mild	Normal Depth	1.09 m
Flow Regime	Subcritical	Critical Depth	0.95 m
Velocity Downstream	2.73 m/s	Critical Slope	0.011742 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	1.83 m
Section Size	1800 mm	Rise	1.83 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.61 m	Upstream Velocity Head	0.28 m
Ke	0.90	Entrance Loss	0.25 m

Inlet Control Properties			
Inlet Control HW Elev.	224.49 m	Flow Control	Unsubmerged
Inlet Type	Projecting	Area Full	2.6 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (101) Requirement size

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.52 m	Headwater Depth/Height	1.00
Computed Headwater Elev:	224.52 m	Discharge	8.5303 m ³ /s
Inlet Control HW Elev.	224.40 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.52 m	Control Type	Entrance Control

Grades			
Upstream Invert	223.00 m	Downstream Invert	222.78 m
Length	29.00 m	Constructed Slope	0.007586 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.65 m
Slope Type	Steep	Normal Depth	0.59 m
Flow Regime	Supercritical	Critical Depth	0.82 m
Velocity Downstream	3.61 m/s	Critical Slope	0.002902 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	3.66 m
Section Size	3660 x 1520 mm	Rise	1.52 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.52 m	Upstream Velocity Head	0.41 m
Ke	0.70	Entrance Loss	0.29 m

Inlet Control Properties			
Inlet Control HW Elev.	224.40 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	5.6 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (102-1)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	227.97 m	Headwater Depth/Height	0.94
Computed Headwater Elev:	227.97 m	Discharge	0.1786 m ³ /s
Inlet Control HW Elev.	227.91 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	227.97 m	Control Type	Outlet Control

Grades			
Upstream Invert	227.47 m	Downstream Invert	227.43 m
Length	12.00 m	Constructed Slope	0.003333 m/m

Hydraulic Profile			
Profile	M2	Depth, Downstream	0.28 m
Slope Type	Mild	Normal Depth	N/A m
Flow Regime	Subcritical	Critical Depth	0.28 m
Velocity Downstream	1.49 m/s	Critical Slope	0.017839 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	0.53 m
Section Size	525 mm	Rise	0.53 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	227.97 m	Upstream Velocity Head	0.05 m
Ke	0.90	Entrance Loss	0.04 m

Inlet Control Properties			
Inlet Control HW Elev.	227.91 m	Flow Control	Unsubmerged
Inlet Type	Projecting	Area Full	0.2 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (102) <duplicate>

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.07 m	Headwater Depth/Height	1.69
Computed Headwater Elev:	224.50 m	Discharge	0.5020 m ³ /s
Inlet Control HW Elev.	224.34 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.50 m	Control Type	Outlet Control

Grades			
Upstream Invert	223.47 m	Downstream Invert	223.43 m
Length	12.00 m	Constructed Slope	0.003333 m/m

Hydraulic Profile			
Profile	CompositeM2PressureProfile	Depth, Downstream	0.46 m
Slope Type	Mild	Normal Depth	N/A m
Flow Regime	Subcritical	Critical Depth	0.46 m
Velocity Downstream	2.11 m/s	Critical Slope	0.024521 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	0.61 m
Section Size	600 mm	Rise	0.61 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.50 m	Upstream Velocity Head	0.15 m
Ke	0.90	Entrance Loss	0.14 m

Inlet Control Properties			
Inlet Control HW Elev.	224.34 m	Flow Control	Transition
Inlet Type	Projecting	Area Full	0.3 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (102) Required culvert size

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.67 m	Headwater Depth/Height	0.98
Computed Headwater Elev:	224.67 m	Discharge	4.9864 m ³ /s
Inlet Control HW Elev.	224.58 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.67 m	Control Type	Entrance Control

Grades			
Upstream Invert	223.47 m	Downstream Invert	223.43 m
Length	12.00 m	Constructed Slope	0.003333 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.63 m
Slope Type	Steep	Normal Depth	0.63 m
Flow Regime	Supercritical	Critical Depth	0.65 m
Velocity Downstream	2.59 m/s	Critical Slope	0.003072 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	3.05 m
Section Size	3050 x 1220 mm	Rise	1.22 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.67 m	Upstream Velocity Head	0.32 m
Ke	0.70	Entrance Loss	0.23 m

Inlet Control Properties			
Inlet Control HW Elev.	224.58 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	3.7 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (103) <duplicate> <duplicate>

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.18 m	Headwater Depth/Height	1.13
Computed Headwater Elev:	224.18 m	Discharge	3.6897 m ³ /s
Inlet Control HW Elev.	224.07 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.18 m	Control Type	Entrance Control

Grades			
Upstream Invert	222.80 m	Downstream Invert	222.52 m
Length	15.68 m	Constructed Slope	0.017857 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.53 m
Slope Type	Steep	Normal Depth	0.44 m
Flow Regime	Supercritical	Critical Depth	0.75 m
Velocity Downstream	3.82 m/s	Critical Slope	0.004048 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	1.83 m
Section Size	1830 x 1220 mm	Rise	1.22 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.18 m	Upstream Velocity Head	0.37 m
Ke	0.70	Entrance Loss	0.26 m

Inlet Control Properties			
Inlet Control HW Elev.	224.07 m	Flow Control	N/A
Inlet Type	0° wingwall flares	Area Full	2.2 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (103) <Required culvert size

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.18 m	Headwater Depth/Height	1.13
Computed Headwater Elev:	224.18 m	Discharge	7.3793 m ³ /s
Inlet Control HW Elev.	224.07 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.18 m	Control Type	Entrance Control

Grades			
Upstream Invert	222.80 m	Downstream Invert	222.52 m
Length	15.68 m	Constructed Slope	0.017857 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.51 m
Slope Type	Steep	Normal Depth	0.41 m
Flow Regime	Supercritical	Critical Depth	0.75 m
Velocity Downstream	3.92 m/s	Critical Slope	0.002884 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	3.66 m
Section Size	3660 x 1220 mm	Rise	1.22 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.18 m	Upstream Velocity Head	0.37 m
Ke	0.70	Entrance Loss	0.26 m

Inlet Control Properties			
Inlet Control HW Elev.	224.07 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	4.5 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (104) <duplicate> <duplicate> <duplicate>

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.84 m	Headwater Depth/Height	1.01
Computed Headwater Elev:	224.84 m	Discharge	0.6624 m ³ /s
Inlet Control HW Elev.	224.78 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.84 m	Control Type	Outlet Control

Grades			
Upstream Invert	223.99 m	Downstream Invert	223.80 m
Length	14.00 m	Constructed Slope	0.013571 m/m

Hydraulic Profile			
Profile	M2	Depth, Downstream	0.49 m
Slope Type	Mild	Normal Depth	0.52 m
Flow Regime	Subcritical	Critical Depth	0.49 m
Velocity Downstream	1.99 m/s	Critical Slope	0.016233 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	0.84 m
Section Size	825 mm	Rise	0.84 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.84 m	Upstream Velocity Head	0.18 m
Ke	0.90	Entrance Loss	0.16 m

Inlet Control Properties			
Inlet Control HW Elev.	224.78 m	Flow Control	N/A
Inlet Type	Projecting	Area Full	0.6 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (104) required culvert size

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	225.10 m	Headwater Depth/Height	0.91
Computed Headwater Elev:	225.10 m	Discharge	4.4361 m ³ /s
Inlet Control HW Elev.	225.00 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	225.10 m	Control Type	Entrance Control

Grades			
Upstream Invert	223.99 m	Downstream Invert	223.80 m
Length	14.00 m	Constructed Slope	0.013571 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.44 m
Slope Type	Steep	Normal Depth	0.37 m
Flow Regime	Supercritical	Critical Depth	0.60 m
Velocity Downstream	3.34 m/s	Critical Slope	0.003059 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	3.05 m
Section Size	3050 x 1220 mm	Rise	1.22 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	225.10 m	Upstream Velocity Head	0.30 m
Ke	0.70	Entrance Loss	0.21 m

Inlet Control Properties			
Inlet Control HW Elev.	225.00 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	3.7 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (105-1)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.50 m	Headwater Depth/Height	0.98
Computed Headwater Elev:	224.50 m	Discharge	1.5474 m ³ /s
Inlet Control HW Elev.	224.37 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.50 m	Control Type	Entrance Control

Grades			
Upstream Invert	223.30 m	Downstream Invert	222.93 m
Length	12.30 m	Constructed Slope	0.030081 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.55 m
Slope Type	Steep	Normal Depth	0.54 m
Flow Regime	Supercritical	Critical Depth	0.68 m
Velocity Downstream	3.04 m/s	Critical Slope	0.013915 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	1.22 m
Section Size	1200 mm	Rise	1.22 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.50 m	Upstream Velocity Head	0.28 m
Ke	0.90	Entrance Loss	0.25 m

Inlet Control Properties			
Inlet Control HW Elev.	224.37 m	Flow Control	Unsubmerged
Inlet Type	Projecting	Area Full	1.2 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (105-1) Required culvert

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.52 m	Headwater Depth/Height	1.00
Computed Headwater Elev:	224.52 m	Discharge	3.5781 m ³ /s
Inlet Control HW Elev.	224.41 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.52 m	Control Type	Entrance Control

Grades			
Upstream Invert	223.30 m	Downstream Invert	222.93 m
Length	12.30 m	Constructed Slope	0.030081 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.42 m
Slope Type	Steep	Normal Depth	0.32 m
Flow Regime	Supercritical	Critical Depth	0.66 m
Velocity Downstream	4.02 m/s	Critical Slope	0.003617 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.13 m
Section Size	2130 x 1220 mm	Rise	1.22 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.52 m	Upstream Velocity Head	0.33 m
Ke	0.70	Entrance Loss	0.23 m

Inlet Control Properties			
Inlet Control HW Elev.	224.41 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	2.6 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (105-2)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.50 m	Headwater Depth/Height	1.98
Computed Headwater Elev:	224.50 m	Discharge	1.3846 m ³ /s
Inlet Control HW Elev.	224.43 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.50 m	Control Type	Outlet Control

Grades			
Upstream Invert	222.84 m	Downstream Invert	222.75 m
Length	12.57 m	Constructed Slope	0.007160 m/m

Hydraulic Profile			
Profile	CompositeM2PressureProfile	Depth, Downstream	0.70 m
Slope Type	Mild	Normal Depth	N/A m
Flow Regime	Subcritical	Critical Depth	0.70 m
Velocity Downstream	2.80 m/s	Critical Slope	0.028027 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	0.84 m
Section Size	825 mm	Rise	0.84 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.50 m	Upstream Velocity Head	0.32 m
Ke	0.90	Entrance Loss	0.29 m

Inlet Control Properties			
Inlet Control HW Elev.	224.43 m	Flow Control	Submerged
Inlet Type	Projecting	Area Full	0.6 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (105-2) required Culvert Size

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	224.50 m	Headwater Depth/Height	1.82
Computed Headwater Elev:	224.50 m	Discharge	2.8602 m ³ /s
Inlet Control HW Elev.	224.50 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	224.37 m	Control Type	Inlet Control

Grades			
Upstream Invert	222.84 m	Downstream Invert	222.75 m
Length	12.57 m	Constructed Slope	0.007160 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.76 m
Slope Type	Steep	Normal Depth	0.75 m
Flow Regime	Supercritical	Critical Depth	0.82 m
Velocity Downstream	3.09 m/s	Critical Slope	0.005531 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	1.22 m
Section Size	1220 x 910 mm	Rise	0.91 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	224.37 m	Upstream Velocity Head	0.41 m
Ke	0.70	Entrance Loss	0.29 m

Inlet Control Properties			
Inlet Control HW Elev.	224.50 m	Flow Control	Submerged
Inlet Type	0° wingwall flares	Area Full	1.1 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (106)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	226.85 m	Headwater Depth/Height	1.15
Computed Headwater Elev:	226.85 m	Discharge	0.4443 m ³ /s
Inlet Control HW Elev.	226.79 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	226.85 m	Control Type	Entrance Control

Grades			
Upstream Invert	226.15 m	Downstream Invert	225.96 m
Length	12.60 m	Constructed Slope	0.015079 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.29 m
Slope Type	Steep	Normal Depth	0.28 m
Flow Regime	Supercritical	Critical Depth	0.38 m
Velocity Downstream	2.51 m/s	Critical Slope	0.006721 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	0.61 m
Section Size	610 x 610 mm	Rise	0.61 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	226.85 m	Upstream Velocity Head	0.19 m
Ke	0.70	Entrance Loss	0.13 m

Inlet Control Properties			
Inlet Control HW Elev.	226.79 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	0.4 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (106) <Required culvert

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	226.85 m	Headwater Depth/Height	1.15
Computed Headwater Elev:	226.85 m	Discharge	0.8886 m ³ /s
Inlet Control HW Elev.	226.79 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	226.85 m	Control Type	Entrance Control

Grades			
Upstream Invert	226.15 m	Downstream Invert	225.96 m
Length	12.60 m	Constructed Slope	0.015079 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.29 m
Slope Type	Steep	Normal Depth	0.28 m
Flow Regime	Supercritical	Critical Depth	0.38 m
Velocity Downstream	2.51 m/s	Critical Slope	0.006721 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	0.61 m
Section Size	610 x 610 mm	Rise	0.61 m
Number Sections	2		

Outlet Control Properties			
Outlet Control HW Elev.	226.85 m	Upstream Velocity Head	0.19 m
Ke	0.70	Entrance Loss	0.13 m

Inlet Control Properties			
Inlet Control HW Elev.	226.79 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	0.7 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (107)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	230.10 m	Headwater Depth/Height	0.95
Computed Headwater Elev:	230.10 m	Discharge	0.5762 m ³ /s
Inlet Control HW Elev.	230.00 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	230.10 m	Control Type	Entrance Control

Grades			
Upstream Invert	229.30 m	Downstream Invert	228.72 m
Length	12.82 m	Constructed Slope	0.045242 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.33 m
Slope Type	Steep	Normal Depth	0.33 m
Flow Regime	Supercritical	Critical Depth	0.45 m
Velocity Downstream	2.81 m/s	Critical Slope	0.015524 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	0.84 m
Section Size	825 mm	Rise	0.84 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	230.10 m	Upstream Velocity Head	0.18 m
Ke	0.90	Entrance Loss	0.16 m

Inlet Control Properties			
Inlet Control HW Elev.	230.00 m	Flow Control	Unsubmerged
Inlet Type	Projecting	Area Full	0.6 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (107) required culvert

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	230.20 m	Headwater Depth/Height	0.98
Computed Headwater Elev:	230.20 m	Discharge	1.6194 m ³ /s
Inlet Control HW Elev.	230.11 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	230.20 m	Control Type	Entrance Control

Grades			
Upstream Invert	229.30 m	Downstream Invert	228.72 m
Length	12.82 m	Constructed Slope	0.045242 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.26 m
Slope Type	Steep	Normal Depth	0.21 m
Flow Regime	Supercritical	Critical Depth	0.49 m
Velocity Downstream	4.06 m/s	Critical Slope	0.004070 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	1.52 m
Section Size	1520 x 910 mm	Rise	0.91 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	230.20 m	Upstream Velocity Head	0.24 m
Ke	0.70	Entrance Loss	0.17 m

Inlet Control Properties			
Inlet Control HW Elev.	230.11 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	1.4 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (108-1)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	236.41 m	Headwater Depth/Height	1.15
Computed Headwater Elev:	236.41 m	Discharge	0.6665 m ³ /s
Inlet Control HW Elev.	236.35 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	236.41 m	Control Type	Entrance Control

Grades			
Upstream Invert	235.71 m	Downstream Invert	235.43 m
Length	12.05 m	Constructed Slope	0.023237 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.25 m
Slope Type	Steep	Normal Depth	0.22 m
Flow Regime	Supercritical	Critical Depth	0.38 m
Velocity Downstream	2.96 m/s	Critical Slope	0.005120 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	0.91 m
Section Size	910 x 610 mm	Rise	0.61 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	236.41 m	Upstream Velocity Head	0.19 m
Ke	0.70	Entrance Loss	0.13 m

Inlet Control Properties			
Inlet Control HW Elev.	236.35 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	0.6 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (108-1) Required Culvert

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	236.61 m	Headwater Depth/Height	0.98
Computed Headwater Elev:	236.61 m	Discharge	2.2671 m ³ /s
Inlet Control HW Elev.	236.53 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	236.61 m	Control Type	Entrance Control

Grades			
Upstream Invert	235.71 m	Downstream Invert	235.43 m
Length	12.05 m	Constructed Slope	0.023237 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.31 m
Slope Type	Steep	Normal Depth	0.26 m
Flow Regime	Supercritical	Critical Depth	0.49 m
Velocity Downstream	3.39 m/s	Critical Slope	0.003478 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.13 m
Section Size	2130 x 910 mm	Rise	0.91 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	236.61 m	Upstream Velocity Head	0.24 m
Ke	0.70	Entrance Loss	0.17 m

Inlet Control Properties			
Inlet Control HW Elev.	236.53 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	2.0 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report

25th Innisfil Side Road (108-2) <duplicate>

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	236.82 m	Headwater Depth/Height	0.98
Computed Headwater Elev:	236.82 m	Discharge	0.7538 m ³ /s
Inlet Control HW Elev.	236.72 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	236.82 m	Control Type	Entrance Control

Grades			
Upstream Invert	235.92 m	Downstream Invert	235.53 m
Length	12.42 m	Constructed Slope	0.031401 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	0.41 m
Slope Type	Steep	Normal Depth	0.41 m
Flow Regime	Supercritical	Critical Depth	0.51 m
Velocity Downstream	2.63 m/s	Critical Slope	0.015316 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	Aluminum	Span	0.91 m
Section Size	900 mm	Rise	0.91 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	236.82 m	Upstream Velocity Head	0.21 m
Ke	0.90	Entrance Loss	0.19 m

Inlet Control Properties			
Inlet Control HW Elev.	236.72 m	Flow Control	Unsubmerged
Inlet Type	Projecting	Area Full	0.7 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report 25th Innisfil Side Road (109)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	236.82 m	Headwater Depth/Height	7.06
Computed Headwater Elev:	236.82 m	Discharge	0.6233 m ³ /s
Inlet Control HW Elev.	236.44 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	236.82 m	Control Type	Outlet Control

Grades			
Upstream Invert	233.59 m	Downstream Invert	233.11 m
Length	12.42 m	Constructed Slope	0.038647 m/m

Hydraulic Profile			
Profile	CompositeM2PressureProfile	Depth, Downstream	0.45 m
Slope Type	Mild	Normal Depth	N/A m
Flow Regime	Subcritical	Critical Depth	0.45 m
Velocity Downstream	3.81 m/s	Critical Slope	0.135736 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	0.46 m
Section Size	450 mm	Rise	0.46 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	236.82 m	Upstream Velocity Head	0.73 m
Ke	0.90	Entrance Loss	0.66 m

Inlet Control Properties			
Inlet Control HW Elev.	236.44 m	Flow Control	Submerged
Inlet Type	Projecting	Area Full	0.2 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (109) Required culvert

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	234.19 m	Headwater Depth/Height	5.30
Computed Headwater Elev:	236.82 m	Discharge	1.1694 m ³ /s
Inlet Control HW Elev.	236.82 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	236.68 m	Control Type	Inlet Control

Grades			
Upstream Invert	233.59 m	Downstream Invert	233.11 m
Length	12.42 m	Constructed Slope	0.038647 m/m

Hydraulic Profile			
Profile	CompositeM2PressureProfile	Depth, Downstream	0.60 m
Slope Type	Mild	Normal Depth	N/A m
Flow Regime	Subcritical	Critical Depth	0.60 m
Velocity Downstream	4.03 m/s	Critical Slope	0.101488 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	0.61 m
Section Size	600 mm	Rise	0.61 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	236.68 m	Upstream Velocity Head	0.82 m
Ke	0.90	Entrance Loss	0.74 m

Inlet Control Properties			
Inlet Control HW Elev.	236.82 m	Flow Control	Submerged
Inlet Type	Projecting	Area Full	0.3 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (110)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	230.80 m	Headwater Depth/Height	1.00
Computed Headwater Elev:	230.80 m	Discharge	42.8156 m ³ /s
Inlet Control HW Elev.	230.55 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	230.80 m	Control Type	Entrance Control

Grades			
Upstream Invert	227.60 m	Downstream Invert	227.55 m
Length	12.20 m	Constructed Slope	0.004098 m/m

Hydraulic Profile			
Profile	S2	Depth, Downstream	1.61 m
Slope Type	Steep	Normal Depth	1.46 m
Flow Regime	Supercritical	Critical Depth	1.73 m
Velocity Downstream	4.43 m/s	Critical Slope	0.002531 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	6.01 m
Section Size	6010 x 3200mm	Rise	3.20 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	230.80 m	Upstream Velocity Head	0.86 m
Ke	0.70	Entrance Loss	0.61 m

Inlet Control Properties			
Inlet Control HW Elev.	230.55 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	19.2 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		

Culvert Calculator Report 25th Innisfil Side Road (111)

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	245.93 m	Headwater Depth/Height	0.98
Computed Headwater Elev:	245.93 m	Discharge	0.1054 m ³ /s
Inlet Control HW Elev.	245.83 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	245.93 m	Control Type	Outlet Control

Grades			
Upstream Invert	245.48 m	Downstream Invert	245.47 m
Length	22.00 m	Constructed Slope	0.000455 m/m

Hydraulic Profile			
Profile	M2	Depth, Downstream	0.22 m
Slope Type	Mild	Normal Depth	N/A m
Flow Regime	Subcritical	Critical Depth	0.22 m
Velocity Downstream	1.31 m/s	Critical Slope	0.018173 m/m

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	0.46 m
Section Size	450 mm	Rise	0.46 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	245.93 m	Upstream Velocity Head	0.02 m
Ke	0.90	Entrance Loss	0.02 m

Inlet Control Properties			
Inlet Control HW Elev.	245.83 m	Flow Control	Unsubmerged
Inlet Type	Projecting	Area Full	0.2 m ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

25th Innisfil Side Road (111) Required Culvert

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	246.48 m	Headwater Depth/Height	1.09
Computed Headwater Elev:	246.48 m	Discharge	2.7777 m ³ /s
Inlet Control HW Elev.	246.43 m	Tailwater Elevation	0.00 m
Outlet Control HW Elev.	246.48 m	Control Type	Outlet Control

Grades			
Upstream Invert	245.48 m	Downstream Invert	245.47 m
Length	22.00 m	Constructed Slope	0.000455 m/m

Hydraulic Profile			
Profile	M2	Depth, Downstream	0.56 m
Slope Type	Mild	Normal Depth	N/A m
Flow Regime	Subcritical	Critical Depth	0.56 m
Velocity Downstream	2.34 m/s	Critical Slope	0.003527 m/m

Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	2.13 m
Section Size	2130 x 910 mm	Rise	0.91 m
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	246.48 m	Upstream Velocity Head	0.18 m
Ke	0.70	Entrance Loss	0.12 m

Inlet Control Properties			
Inlet Control HW Elev.	246.43 m	Flow Control	Unsubmerged
Inlet Type	0° wingwall flares	Area Full	2.0 m ²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	3
C	0.04230	Equation Form	1
Y	0.82000		